DEVELOPMENT OF OPERATION POLICY FOR TAWA DAM



NATIONAL INSTITUTE OF HYDROLOGY JAL VIGYAN BHAWAN ROORKEE - 247 667

ABSTRACT

Reservoirs are one of the most important elements of water resource systems. They are used for spatial and temporal redistribution of water in accordance with requirement. Reservoir storage is necessary to use the highly variable water resources of a river basin for beneficial purposes such as municipal and industrial water supply, irrigation, hydroelectric power generation, and navigation. Under uncertain streamflows and multiobjective demands, water allocation process becomes more complex. The evaluation of current operating rules of existing reservoirs is very important to meet the changing demands according to the public needs and objectives. An operating plan or release is a set of rules for determining the quantity of water to be stored or released from a reservoir or a system of several reservoirs under various conditions. The operating rules provide guidance to the water managers who make the actual release decisions.

Simulation models are used to represent a system and to predict the behaviour of the system under given set of conditions. Simulation models are limited to predicting system performance for a user specified set of variable values. On the other hand, an optimization procedure may involve iterative executions of a simulation model, with the iterations being automated to various degrees.

In this report, rule curves based policy has been adopted for the conservation regulation of Tawa dam. Rule curves are developed for three levels such as upper rule level, middle rule level (critical for irrigation), lower rule level (critical for water supply and upstream use). The generalised simulation model developed by the Water Resources System Group at N.I.H. is used to simulate the system operation and to refine the rule curves. A number of simulation runs have been taken by adjusting the rule curves and the final rule levels have been arrived to satisfy the target demands to the maximum extent possible. The current operating policy of the reservoir is also simulated and the results are compared with the recommended rule curves based policy.

CONTENTS

		Page no
LIST OF FIG	GURES & TABLES	(ii)
CHAP 1	INTRODUCTION	1
CHAP 2	DESCRIPTION OF THE STUDY AREA	4
2.1	Tawa River Basin	4
2.2	Physical Characteristics of the Tawa Dam	4
2.3	Target Demands from the Tawa Dam	6
CHAP 3	GENERALISED MULTIRESERVOIR SIMULATION MODEL	7
CHAP 4	DEVELOPMENT OF OPERATION POLICY FOR TAWA	9
	RESERVOIR	
4.1	Current Operating Policy of the Reservoir	9
4.2	Aspects Considered for Policy Development	9
4.3	Data Used for the Study	9
4.4	The Solution Strategy Adopted	10
4.5	Monthly Dependable Inflow	11
4.6	Derivation of Initial Trial Rule Curves	11
4.7	Simulation of Monthly Operation of the Reservoir	12
4.8	Results of Simulation Runs for the Tawa Reservoir	14
4.9	Recommended Conservation Operation Procedure for the	16
	Tawa Reservoir	
CHAP 5	CONCLUSIONS	18
	REFERENCES	19
	SALIENT FEATURES OF TAWA RESERVOIR	20
	TABLES AND FIGURES	22

LIST OF FIGURES AND TABLES

		Page No.
Salier	nt Features of Tawa Reservoir	
TABL	ES	
Tab 1	Monthly Inflow (M Cum) Series at the Tawa Reservoir for the Period (1948-71)	22
Tab 2	Elevation-Area-Capacity Table for Tawa Reservoir (Based on 1990 Survey)	23
Tab 3	Normal Monthly Evaporation Depths (m) for Tawa Reservoir	23
Tab 4	Target Monthly Demands from Tawa Reservoir	24
Tab 5	Monthly Yield Estimation for Tawa Reservoir	24
Tab 6	Initial Rule Curve Levels (m) for Tawa Reservoir	25
Tab 7	Recommended Rule Curve Levels (m) for Tawa Reservoir	25
Tab 8	Simulation of Tawa Reservoir with Current Operation Policy	26
Tab 9	Simulation of Tawa Reservoir with Recommended Operation Policy	30
FIGU	RES	
Fig 1	Map of Tawa Basin showing Reservoir Location and Main Canal System	s 34
Fig 2	Elevation-Area Curve for Tawa Reservoir	35
Fig 3	Elevation-Capacity Curve for Tawa Reservoir	36
Fig 4	Recommended Rule Curves for Tawa Reservoir	37

CHAPTER 1 INTRODUCTION

Water has been recognised as a basic and crucial natural resource for economic development. With continuous increase in the demand for water in all regions, the need for planning for water resources development and management has become more important. The investments in water resources development are influenced by economic, social and political considerations along with the engineering aspects. Reservoirs are one of the most important elements of water resource systems. They are used for spatial and temporal redistribution of water according to prevalent demands. Reservoir storage is necessary to use the highly variable water resource of a river basin for beneficial purposes such as municipal and industrial water supply, irrigation, hydroelectric power generation, and navigation. Under uncertain streamflows and multiobjective demands, water allocation process becomes more complex.

Since more than 80% of the annual rainfall in India occurs in the four monsoon months from June to September, it is general tendency to fill up the reservoirs during the monsoon months and then to use this stored water for the remaining months of the water year. On the other hand, most of the flood situations also arise in the monsoon months and reservoirs should be kept empty so that they can effectively control any flood situation and prevent the downstream area from flooding. An optimum operation policy is derived keeping in view both these conflicting purposes to attain the maximum possible benefits from the reservoir. In India more than 3000 major and medium dams have already been completed and multipurpose reservoir projects are quite common among them.

The evaluation of current operating rules of existing reservoirs is very important to meet the changing demands according to the public needs and objectives. An operating plan or release is a set of rules for determining the quantity of water to be stored or released from a reservoir or system of several reservoirs under various conditions. Typically, a regulation plan includes a set of quantitative criteria within which significant flexibility exists for operator judgement. The operating rules provide guidance to the water managers who make the actual release decisions. Reservoir system analysis models are developed to support

- * in sizing of reservoir storage capacities and establishing operating policies during preconstruction planning of new projects.
- * in evaluation of the operating plans of existing reservoir systems.
- * in administration of water allocation systems involving water rights and agreements between water suppliers and users.
- * in operational planning for developing management strategies for the next year or season.
- * in real time operations.

The reservoir system models are of two kinds: simulation and optimization. A simulation model, which is a representation of a system, is used to predict the behaviour of the system under a given set of conditions. Alternative executions of a simulation model are made to analyse the performance of the system under varying conditions, such as for alternative operating policies. Optimization models are used to compute a set of decision variable values that minimize or maximize an objective function subject to constraints. Simulation models are limited to predicting system performance for a user specified set of variable values whereas optimization models automatically search for an optimum solution. All optimization models simulate the system. An optimization procedure may involve iterative executions of a simulation model, with the iterations being automated to various degrees.

Tawa reservoir is a multipurpose reservoir located in the Hoshangabad District, Madhya Pradesh mainly to serve irrigation, water supply and upstream use. The upstream use demands are accounted in the months of July, August and September. There is no control on the water supply for upstream use demands from the dam site. The power generation of this reservoir is incidental. The monthly inflow series (in million cubic meter) at the Tawa reservoir for the period 1948 to 1971 is considered for the study. The Elevation-Area-Capacity table based on the 1990 survey has been used for the operation study.

In this report, rule curves based policy has been recommended for the conservation regulation of the Tawa dam. This reservoir is not operated for flood protection. Power transformation technique is applied to arrive different reliable inflow series using the

available inflow data series. These reliable inflow series are used later in the trial rule curve derivation module of the SRA package to develop trial rule curve levels. Three rule curves are developed: upper rule level, middle rule level (critical for irrigation), lower rule level (critical for water supply and upstream use). The generalised simulation model developed by Water Resources System Group at NIH is used to simulate the system operation and to refine the rule curves. A number of simulation runs have been taken by adjusting the rule curves and the final recommended rule levels have been arrived by using the reliability analysis. In the simulation runs the upper and middle rule curves are adjusted to avoid the unnecessary spill so that the failures can be reduced. The rule curves are adjusted to avoid the critical failures (supply less than 0.75 times of the demand) at the maximum possible and thus the partial supply is maintained for longer period to avoid heavy damage of the crop. The current operating policy is also simulated. The results of both policies are compared. The final recommended rule curves policy for the reservoir gives better performance than the presently followed policy. Three rule curves have been recommended for different purposes for the conservation regulation of the reservoir and the operation procedure for the reservoir has been explained.

This report consists of five chapters. Chapter 2 contains the brief description of the basin and the general details of the reservoir. It also presents the different demands from the reservoir. Chapter 3 gives brief description about the capability of SRA (Software for Reservoir Analysis) package. It also explains briefly about the multireservoir simulation module. Chapter 4 details the data used for the study, the solution strategy of the study and the recommended procedure for the operation of the system. The conclusions drawn from the study are given in Chapter 5. The data used for the study and the recommended rule curves have been presented in tabular and graphical form. The detailed simulation tables for the current operating policy and the recommended rule curves based policy have also been presented.

CHAPTER 2

DESCRIPTION OF THE STUDY AREA

2.1 TAWA RIVER BASIN

The Tawa river is one of the major tributaries of the Narmada river. Rising on the south western end of the Mahadeo hills near the Mankhoi village (Latitude 22°7', Longitude 78°29') in the Chhindwara district, it cuts its course through the hills of the Satpura Range, flowing first westwards for about 64.4 kms up to the Golai reserve forest (Latitude 22°1', Longitude 77°49') and then northwards for about the same distance upto the G.I.P. Railway Bridge (Latitude 22°36½', Longitude 77°59') where it emerges from the Satpura hills into the alluvial plains of the Narmada valley. Emerging from plains, it turns north-westwards and, widening its bed to 1.61 km, runs for about 32.2 kms to meet the Narmada. About 6.44 kms upstream of the railway bridge, the Tawa is joined by its biggest tributary, the Denwa, which rises on the northern slopes of the Mahadeo hills. The Tawa basin showing the reservoir location and the main canal systems is presented in Fig. 1.

2.2 PHYSICAL CHARACTERISTICS OF THE TAWA DAM

The Tawa dam site across the Tawa river lies in the Hoshangabad District, Madhya Pradesh. It is situated 4.83 kms southwest of Bagra Tawa Railway Station (Latitude 22°35½, Longitude 77°59½) and about 1.61 km north of Ranipura (Latitude 22°34½, Longitude 78°59¾). It is about 2.01 kms upstream of the railway bridge on the Tawa river. The hills on either side of the dam site rise to about 381 to 457.2 m while the river bed is about 307.85 m above MSL. The hilly region is overgrown with a fairly dense jungle while the alluvial plain in the north is a fertile wheat producing area.

Tawa masonry dam is a gravity structure with maximum height of 57.912 m above deepest foundation level. The masonry dam is 417.576 m long consisting of 237.744 m of spillway in the river portion. Road way of 6.706 m between the kerbs is provided for full length of the masonry dam. The spillway can pass a maximum moderated flood discharge of 21225 cumecs with 13.487 m depth of flow over the crest at RL 343.105 m. The spillway is controlled by 13 radial gates of size 15.240 m × 12.192 m. The concrete spillway piers are 3.048 m thick. The spillway is of Ogee type with Hydraulic jump type stilling basin with floor

level at RL 303.276 m.

The masonry dam is flanked on either sides by earth dam 690.372 m long on left side and 521.208 m long on the right side in addition to 375.760 m of dykes in the two saddle portion. The maximum height of earth dam above the stripped level is 33.528 m. The top of the earth dam is at RL 359.664 m providing a free board of 2.972 m above the maximum reservoir level.

The catchment area at the dam site is 5982.90 sq kms. The gross, live and dead storage capacities of the reservoir are 2079.698 Mm³, 1955.854 Mm³ and 123.844 Mm³ respectively. The maximum water level, full reservoir level, spillway crest level and dead storage level are 356.692 m, 355.397 m, 343.205 m and 334.243 m respectively. The average annual rainfall for this catchment area is 1546.13 mm. The maximum and minimum rainfall values are 2506.22 mm and 873.76 mm respectively. The average annual runoff is 3706.616 Mm³. The maximum and minimum runoffs are 9400.373 Mm³ and 1927.934 Mm³ respectively. The peak of the design flood hydrograph is 30800 cumecs while the maximum observed flood is 24300 cumecs.

The project has been constructed to meet the irrigation and water supply demands. The power generation in this project is only incidental. The command area is irrigated by Left Bank and Right Bank Canal systems. The sill levels of sluices of Left Bank and Right Bank Canal systems are at 334.243 m and 338.328 m respectively. The Minimum Draw Down Levels for the Left Bank and Right Bank Canal are 336.804 m and 340.462 m respectively. The lengths of main canal, Bagra branch canal and Pipariya branch canal are 7.5 km, 24 km and 60 km respectively. The gross command area, culturable command area, irrigable command area under LBC are 288956 ha, 186162 ha and 186162 ha respectively. The gross command area, culturable command area, irrigable command area under RBC are 112729 ha, 98079 ha and 60702 ha respectively. The annual irrigation demands to be met by the LBC and the RBC are 1895.863 Mm³ and 431.495 Mm³ respectively. The annual water supply demand to be met by the system is 11.596 Mm³. The upstream use accounted at reservoir site is 222.027 Mm³. This demand is to be met in the months of July, August and September. The power plant is situated on the LBC and its power generating capacity is 13 MW (two units of 6.5 MW). The rainfall data for 4 stations Betul, Shahpur, Tamia and Pachmarhi in the

catchment and of one station at Bagra Tawa is being used for average annual rainfall calculation. The average annual rainfall values are available from 1891 to 1971. The monthly inflow values are available from 1948 to 1971.

2.3 TARGET DEMANDS FROM THE DAM

The Tawa reservoir project is mainly operated to meet the irrigation demands of LBC and RBC, water supply demands for the Ordinance Factory, Itarsi and upstream use demands. This reservoir is not operated for flood control. The power production at the LBC is 13 MW and the power generation is incidental. Various demands which are met from the reservoir are detailed in the following section.

a) Water Supply Demands for Domestic Purposes

The agreement between the Government of Madhya Pradesh and the Ordinance Factory, Itarsi is to supply water for domestic purposes at the rate of 8.4 million gallons per day from the reservoir through a separate pipe. This amounts to 11.596 Mm³ annually and has been considered for developing the operating policy of the reservoir.

b) Upstream Use Demands

The upstream use demands for the months of July, August and September are 88.811, 66.608 and 66.608 Mm³ respectively. These values have been subtracted from the inflow series of corresponding months as there is no control on the water supply for these demands from the dam.

c) Irrigation Demands

The Tawa reservoir is constructed to provide irrigation through its Left Bank Canal and Right Bank Canal Systems. The gross command area and culturable command area under LBC are 288956 ha and 186162 ha respectively. The gross command area and culturable command area under RBC are 112729 ha and 98079 ha respectively. The cropping intensity under LBC for kharif, Rabi and summer crops are 67%, 67%, 4%. The cropping intensity under RBC for kharif, rabi and summer crop are 58%, 67%, 0%. The command area lies in the districts of Hoshangabad, Narasinghpur and Betul. The annual requirement of the irrigation system is estimated to be around 2327.358 Mm³.

CHAPTER 3

GENERALISED MULTIRESERVOIR SIMULATION MODEL

A software package, named SOFTWARE FOR RESERVOIR ANALYSIS (SRA) has been developed by the water resources systems group of N.I.H. The software is capable of carrying out various kinds of analysis associated with the planning and design of reservoirs. The software is a menu-driven, interactive package. The various modules of this software are Reservoir Capacity Computation, Interpolation of Elevation-Area-Capacity Table, Hydropower Analysis (Firm Power Determination, Hydropower Simulation). Reservoir Inflow Estimation, Multireservoir Simulation, Reservoir Routing (Coefficient Method, Goodrich Method, Mass Curve Method, Modified Puls Method), Stretched Thread Method, Trial Rule Curve Derivation, Yield-Storage Analysis [(Storage known, Yield unknown),(Yield known, Storage unknown)]. Multireservoir Simulation and Trial Rule Curve Derivation are the important modules as per this study is concerned.

The module **MULTIRESERVOIR SIMULATION** consists of a model for simulating the operation of a multipurpose multireservoir system for conservation operation. The various conservation purposes considered in the model are water supply for domestic and industrial purposes, irrigation, hydropower generation and minimum flow requirements in the downstream river channel. In a multireservoir system, the model can help in finalizing the optimum rule levels for each storage location. The rule curves based policy has been adopted in this module.

For each storage location, the model operates the reservoir in accordance with the given trial rule curves (given for each reservoir) and carries out the reliability analysis. Correspondingly, it calculates the time and volume reliability of each reservoir for the given set of rule curve levels and for the given period of operation. Detailed simulation table is also prepared. Based on the observations from the simulation table, trial rule curves are modified till optimum results are achieved.

In the program, several checks have been introduced to detect the likely errors while preparing the input data. After reading a group of data items, the program displays a message

on the screen showing that the corresponding data items have been read properly. This facility is very helpful in locating the possible error as the user knows that for which structure and at which group of data, the error is encountered. The program reads the entire data for a structure at a time. After reading the input data, the simulation of operation for each structure is performed.

Γ

The report on "Software for Reservoir Analysis (SRA)" of the N.I.H. can be consulted to know more about the rule curves, data requirement and output of this module.

CHAPTER - 4

DEVELOPMENT OF OPERATION POLICY FOR TAWA RESERVOIR

4.1 CURRENT OPERATING POLICY OF THE RESERVOIR

From the working table (1948-1971) of the reservoir, prepared by the reservoir operating authority, it is noted that top priority is given for meeting full water supply demand (11.596 Mm³ annually) while next higher priority is given for meeting irrigation demand by Right Bank Canal (431.495 Mm³ annually). The irrigation demand of Left Bank Canal is met from the water in excess of the above mentioned demands. If the water is in excess of all demands, the extra water is spilled after filling the reservoir upto the FRL. It is also mentioned that the requirement of June for RBC is neglected as the nursery crop of this period is grown by the tube well irrigation. Water is supplied to late variety of paddy in the middle or end of July for RBC. The failures for LBC, as noted from the working table and as given by the project authority, are for the years 51-52, 52-53, 54-55, 56-57, 57-58 i.e., 5 years in 23 years and it amounts to 22 percent. There is no control on the water supply for upstream use demands from the dam site.

4.2 ASPECTS CONSIDERED FOR POLICY DEVELOPMENT

In the present study, the reservoir operation policy for the Tawa reservoir has been developed for the conservation demands like water supply for domestic and industrial use, upstream use and irrigation. Hydropower generation is incidental. The policy development does not include flood protection as this reservoir is not operated for flood control.

4.3 DATA USED FOR THE STUDY

The monthly inflow series (in million cubic meter) at the Tawa reservoir for the period 1948 to 1971 has been considered for the policy development and is given in Table 1. The Elevation - Area - Capacity table based on the 1990 survey has been used for the operation study and the same is given in Table 2. It is noted from the survey that the reduction in storage capacity at DSL is 52.8 percent and at FRL is 10 percent of the storage capacity. The normal values of monthly evaporation for the reservoir are presented in Table 3. The Elevation - Area curve and the Elevation - Capacity curve for this reservoir have been plotted

in Figure 2 and Figure 3 respectively. The average monthly irrigation demands have been calculated from the details collected from the project authority. The details about power plants have been obtained from the officials in charge of the reservoir. Various demands from the reservoir are given in Table 4.

4.4 THE SOLUTION STRATEGY ADOPTED

The rule curves based policy has been adopted for the conservation regulation of the Tawa reservoir. This policy tries to distribute the deficit equitably much in advance so that the reduced supply can be maintained throughout the demand period. Based on the number, nature and priority of demands, three initial trial rule curves have been derived for this reservoir.

The methodology of simulation has been adopted for refining the rule curves. Simulation, in essence, is to duplicate the system behaviour under given hydrologic conditions. Though this approach is not useful for deriving any operation policy directly, it helps in policy evaluation. Simulation is one of the important techniques which can represent the true behaviour of a system. The analysis has been carried out for the monthly time interval. The results of simulation have been used for refining the policy so as to achieve the maximum possible benefits from the available resources.

The module **MULTIRESERVOIR SIMULATION** of **SRA** package has been used to simulate the monthly operation of this reservoir for conservation regulation. For each time step, the module calculates the release to be made for each demand in accordance with the trial policy, spill (if any) and the actual evaporation losses at the prevailing water spread area etc. In the end, the module calculates the number of months when the release is less than the demand and thus calculates the monthly time reliability of the reservoir for the trial policy. It also calculates the volumetric reliability. In addition, it also calculates the number of months of critical failure when the release is less than 75% of the target demand.

Initially, three trial rule curve levels have been derived for the reservoir using various scenarios of reservoir inflows and level of demands. Upper rule curve provides empty storage space in the reservoir for flood absorption. The middle rule curve is critical for

irrigation demands and the lower rule curve is critical for water supply demands and upstream use demands. An exhaustive reservoir operation simulation study has been undertaken using these rule curves. The rule curves have been refined as long as the monthly time reliability of the reservoir could be increased without increasing the number of critical failure (supply less than 75 percentage of the demand) months. The detailed operation table of simulation has been utilised for this purpose. The operation policy which has met the objectives of the conservation storage regulation has finally been recommended for adoption. Monthly reservoir inflows of different probabilities have been considered in deriving different rule levels. The derivation of inflows of different probability is explained in the following section.

4.5 MONTHLY DEPENDABLE INFLOW

The monthly inflow series for the reservoir has been analyzed using the statistical approach. For Tawa reservoir, inflow data are available for 23 years of historical record and the different reliable inflows have been arrived at the dam site using the available inflow data series. The power transformation approach has been used for this purpose. This approach is a standard technique and is being extensively used in the hydrological analysis. The monthly inflows for the reservoir have been estimated for 50%, 60%, 70%, 75%, 80% and 90% probabilities using this approach. The results of this analysis for the 12 months have been presented in Table 5. The report on "Software for Reservoir Analysis (SRA)" can be consulted to know more about the power transformation approach.

4.6 DERIVATION OF INITIAL TRIAL RULE CURVE LEVELS

The computations for deriving various rule curve levels have been made using the monthly inflow series for different probability levels along with the average monthly demands. Using the monthly dependable inflow series, the water availability has been assumed as corresponding to particular monthly inflow series. Computations of end-of-month reservoir levels have been made for 12 months after allowing for water demands in full or partial and the evaporation losses from the reservoir surface. The Elevation-Area-Capacity table has been used and the intermediate values have been linearly interpolated whenever required. The evaporation losses have been considered at normal monthly rate over the surface area of the reservoir corresponding to a particular elevation.

The module **TRIAL RULE CURVE DERIVATION** of **SRA** package has been used to derive the initial trial rule curves. Since the Tawa reservoir is meant to serve for irrigation, upstream use and water supply demands, it has been considered appropriate to decide three rule levels for this reservoir. First, the upper rule level up to which the reservoir should be filled if there is sufficient inflow in the reservoir and all the demands are met in full. Second, the irrigation rule level, below which the supply for irrigation demands should be curtailed so that the reduced supply can be maintained for a longer duration. Third, the water supply level, below which, the supply will be made to meet domestic and industrial water supply demand and upstream use demand. The report on "Software for Reservoir Analysis (SRA)" can be consulted to know the procedure adopted to derive the trial rule curves. The initial trial rule curves are presented in Table 6.

4.7 SIMULATION OF MONTHLY OPERATION OF THE RESERVOIR

The module **MULTIRESERVOIR SIMULATION** of the **SRA** package has been used to simulate the monthly operation of the reservoir. The monthly inflow data, target monthly domestic and industrial water supply demand (M Cum), upstream use demand (M Cum), irrigation demand (M Cum), storage details and the normal values of monthly evaporation are input to the program. Various trial rule curve levels, derived earlier, are also input to the program. The module simulates the operation of the reservoir in accordance with the policy. Detailed working table for the reservoir operation is prepared. The following quantities are computed monthly:

- 1. Initial storage (M Cum) in the reservoir at the start of each time step (monthly in the present case).
- 2. Actual evaporation losses (M Cum) from the reservoir based on the initial and final water spread area. This is calculated iteratively till sufficient accuracy is achieved.
- 3. The total demand in the month (M Cum) for all the purposes.
- 4. Release of water from the reservoir for different purposes like domestic & industrial demand, upstream use demand and irrigation demand, based on the policy.
- 5. The amount of incidental power produced (M Kwh) in different months.
- 6. The spill (M Cum) from the reservoir, if any.
- 7. End level (m) in the reservoir after each time step.

In addition, some ratios are also calculated and if a ratio is less than a specified target, letters signifying different failures are indicated. These are indicated below:

- 1. Total release for irrigation/Total irrigation demand. If this ratio is less than 1.00, "I" appears after the Tot_Rel column indicating that it is a failure month for irrigation.
- 2. Total Release for water supply and upstream use/Total domestic & industrial water demand and upstream use demand. If this ratio is less than 1.00, "W" appears after the Tot_Rel column indicating that it is a water supply and upstream use failure month.
- 3. Total release for all demands/Total of all demands. If this ratio is less than 0.75, "C" appears in the column instead of "I" indicating that it is a critical failure month.

In the end, the module calculates the total number of failure months when the supply is less than the total of target demand for all purposes and hence, calculates the monthly time reliability of the reservoir for the test policy. It also calculates the total number of critical failure months when the supply is less than 75% of the demand and hence, the monthly time reliability for critical failure. The module also calculates the total volumetric demand and supply from the reservoir and hence, the volumetric reliability of the reservoir.

An exhaustive operation simulation study for the Tawa reservoir has been undertaken using the initial trial rule curve levels. The results of the simulation analysis have been intercompared using above performance indices. The policy has been refined using the detailed operation simulation table of the reservoir. The operation policy which has best met the objectives of the conservation storage regulation has finally been recommended for adoption.

For refining the initial rule curve levels (those giving the highest monthly time reliability with the least number of critical failure months), the detailed operation table has been utilised. Initially, the upper rule curve levels were changed to store spilled water to increase the time reliability and volume reliability of the reservoir. Then based on the

observations from the simulated operation table, the rule curves for irrigation were adjusted till the number of failure months could be reduced without increasing the number of critical failure months. The rule level for water supply demand and the upstream use was not modified as the failure for the same was zero at the initial run of the module. A number of simulation runs have been taken in the process and the various rule curve levels have been finalised. In the next step, the final rule curves for irrigation, water supply demands and upstream use demands were kept unchanged and the initial upper rule curve levels were lowered till there is any increase in the number of critical failure months. In this way, the various rule curves have been finalised.

4.8 RESULTS OF SIMULATION RUNS FOR THE TAWA RESERVOIR

Simulation runs for the Tawa reservoir with the present policy (SLOP) and the recommended policy have been presented in Table 8 and Table 9. The abstract of the simulation runs for the present policy and for the recommended policy is presented as follows.

Sl. no	Items	Present policy	Recommended
			policy
1	Number of failures for water supply	0	0
2	Time reliability for water supply	1	1
3	Number of failures for irrigation	32	45
4	Time reliability for irrigation	0.886	0.84
5	Number of critical failures	27	4
6	Volume reliability for irrigation and water	0.94	0.95
	supply		

The results are discussed as follows.

In the present operating policy (SLOP), the demand for the water supply and industrial use and upstream use is met fully in all the periods. The demand for RBC and LBC is met to the possible extent. The month in which the release for the irrigation is less than 75% of the total demand is treated as critical failure month. From the simulation table it is observed

that the monthly time reliability for water supply and upstream use and irrigation comes out to be 100% and 88.6% respectively. The volume reliability comes out to be 94%. Out of the 281 months, there are 27 critical failure months (when the total release is less than 75% of the total demand). The reliability of the reservoir for meeting partial demands is also carried out. With this policy, the reliability for supplying 75% assured supply for irrigation comes out to be 100%. For calculating the average annual values, three drought years, i.e. 1951-52, 1952-53 and 1957-58 have been discarded. From the simulation table using the current operating policy, the average annual release for irrigation is 2254.91 M Cum while average annual incidental hydropower production is 92.15 M kwh.

At the beginning the simulation run has been carried out with initial trial rule curve levels. From the simulation table it is observed for this run that there are so many periods with unnecessary spills. The critical failures observed are 4. The number of irrigation failures are 78 with the time reliability for the irrigation is 72.2%. The volume reliability for the irrigation and water supply is 92%.

Later number of simulation runs have been taken considering various rule curve levels and the refinement has been made till the reliability of the reservoir for various demands could be improved and the unnecessary spill water could be reduced. In the recommended policy, it has been assumed that for the reservoir level above the irrigation rule level, all the demands will be satisfied in full. If the water level falls below the irrigation rule level then the water supply demand and upstream use demand will be satisfied in full while the irrigation demand will be curtailed to 75%. If the reservoir level falls below the water supply rule level, only water supply demands and upstream use demands will be met for as longer the duration as possible. Based on this policy, the monthly time reliability for water supply and upstream use and irrigation comes out to be 100% and 84% respectively. The volume reliability comes out to be 95%. Out of the 281 months, there are 4 critical failure months (when the total release is less than 75% of the total demand). The reliability of the reservoir for meeting partial demands is also carried out. With the recommended policy, the reliability for supplying 75% assured supply for irrigation comes out to be 99.3%. From the simulation table using the recommended rule curve policy, the average annual release for irrigation is 2500.39 M Cum while average annual incidental hydropower production is 95.70 M kwh.

From the operation table, water lost through evaporation per year is around 223.53 M Cum. Thus, roughly 2750 M Cum of water is required annually. From the inflow data, it is observed that the years 1951-52, 1952-53, and 1957-58 are drought years with annual inflow less than 2088 M Cum. From the operation table, it can be seen the most of the irrigation failure months happen to fall in these drought years.

From the above analysis, it is observed that the presently followed policy gives higher reliability than the recommended policy for meeting the full as well as partial demands. However the presently followed policy gives more critical failures than the recommended policy. So the recommended policy tries to distribute the deficit equitably in all the months. As soon as shortage of water is anticipated, the supply is curtailed much in advance so that reduced supply can be maintained throughout the year. The recommended operating policy improves the volume reliability for irrigation and water supply by 1% over the current operating policy. The recommended operating policy improves the average annual release and also the incidental power generation. By comparing the two simulation tables, it is observed that the recommended operating policy reduces the spill considerably over the current operating policy. So the recommended policy gives the optimum upper rule level which gives the storage space for flood absorption without affecting conservation demands.

Three rule curves have been provided in the recommended policy. The upper rule curve has been termed as Curve A, irrigation rule curve as Curve B and water supply and upstream use rule curve as Curve C. These have been presented in Table 7 and have been plotted in Figure 4.

4.9 RECOMMENDED CONSERVATION OPERATION PROCEDURE FOR THE TAWA RESERVOIR

The recommended procedure for conservation operation of the Tawa reservoir using the three rule curves A, B and C is as follows:

For a particular month:

Try to maintain the reservoir at rule level A while meeting all the demands in full.
 If the reservoir level overtops the rule level A, spill the excess water and bring the reservoir level back to level A.

- 2. If it is not possible to maintain the reservoir level at A, meet all the demands as long as the reservoir is at or above level B.
- 3. If it is likely that the reservoir level will go below level B, meet 75% of target irrigation demands.
- 4. If the reservoir is at or below rule level C, make release to meet only full water supply demand and upstream use demand for as longer the duration as possible. In this case, stop supply for irrigation demands completely.
- 5. It is advisable to periodically review the situation within a month and modify the previous decision (for the remaining duration of that month) and follow steps (1) to (4) to operate the reservoir.

CHAPTER - 5 CONCLUSIONS

To overcome the uncertain nature of monsoon, it is imperative to use our available, water resources efficiently. Reservoir operation is an important aspect of water resources management. In the present study, a policy to optimally utilise the conservation storage of Tawa reservoir has been developed. The operation of the reservoir with the recommended policy has been simulated using 23 years of inflow data series. Some of important conclusions from the simulation study are as follows:

- The presently followed operating policy (SLOP) gives more critical failures than recommended rule curve policy. The recommended policy gives the irrigation rule curve to distribute the deficit equitably much in advance.
- The average annual incidental hydropower and the average annual release are improved by the recommended operating policy. In the months of August and September, the reservoir is filled to its capacity in number of years.
- The volume reliability for irrigation and water supply is improved by the recommended operating policy. The value is improved by 1% over the current operating policy.
- 4. The present study has been carried out based on the water supply demands and upstream use demands and the irrigation demands from the left bank canal and right bank canal. From the analysis, it appears that the demands can be satisfied almost in all the months except for the drought years 1951-52, 1952-53 and 1957-58.

This reservoir is not operated for flood protection. Eventhough The upper rule levels in all the months have been lowered below the FRL, to the extent possible, as long as the conservation requirement of the reservoir is not affected. It is required to maintain the reservoir at the upper rule level and spill the additional water at the rate or inflow so that the reservoir can be kept at this level.

REFERENCES

- "Flood Frequency Analysis Using Power Transformation", Report No. DP-1, National Institute of Hydrology, Roorkee.
- Hall, W. A. and Dracup, J. A., "Water Resources Systems Engineering", Tata McGraw-Hill Publishing Company, New Delhi, 1979.
- Investigation Report on Tawa Multipurpose Project, Part II
- Loucks, D. P., Stedinger, J. R. and Haith, D. A., "Water Resources Systems Planning and Analysis", Prentice Hall Inc., New Jersey, 1981.
- "Multipurpose Operation of a Reservoir", Report No. M-6, National Institute of Hydrology, Roorkee.
- Ralph A.wurbs (1993), "Reservoir-System Simulation and Optimization Models", Journal of Water Resources Planning and Management, Vol 119, No. 4, pp 455-472.
- Ralph A. wurbs (1994), "Modelling and Analysis of Reservoir/River System Reliability" Trends in Hydrology, 1.
- Ralph A.wurbs, "Modelling and Analysis of Reservoir System Operations", Prentice Hall PTR, Upper Saddle River, NJ 07458, 1996.
- Ricardo Harboe, "Optimization, Simulation and Multiobjective of Operating Rules for Reservoir Systems", Water Forum '86, Federal Republic of Germany.
- Slobodan, P. Simonovic (1992), "Reservoir System Analysis: Closing gap between theory and practice", Journal of Water Resources Planning and Management, Vol 118, No. 3, pp 262-80.

SALIENT FEATURES OF TAWA RESERVOIR

River Latitude Longitude Toposheet No.	Village - Tawa Nagar, Ranipura Distt Hoshangabad State - Madhya Pradesh Tawa 22° 33′ 40" 77° 58′ 30" 55 F/14
HYDROLOGY Catchment Area Maximum Rainfall in the catchment Minimum Rainfall in the catchment Average Rainfall in the catchment Maximum Annual yield Minimum Annual yield Average Annual yield 50% dependable yield 75% dependable yield 90% dependable yield Maximum designed flood Moderated flood	5982.90 sq. km 2506.22 mm 873.76 mm 1546.13 mm 9400.373. M Cum 1927.934 M Cum 3706.616 M Cum 4086.529 M Cum 3076.306 M Cum 2142.560 M Cum 30800 Cumecs 20500 Cumecs
RESERVOIR DATA River Bed level Minimum draw down level (MDDL) Spillway Crest level Full Reservoir level (FRL) Maximum Reservoir level (MWL) Top of Bund Level (TBL) Sill level (LBC) Sill level (RBC) Tail water level Dead Storage Capacity at MDDL Gross Storage Capacity at FRL Live Storage Capacity at FRL Area under Submergence	RL 309.677 m RL 334.243 m RL 343.205 m RL 355.397 m RL 356.692 m RL 359.664 m RL 334.243 m RL 338.328 m RL 316.992 m 123.844 Mm ³ 2079.698 Mm ³ 1955.854 Mm ³

DAM DATA

Type of Dam	Composite earthen & masonry
Length of masonry Dam	417.576 m
Length of earthen Dam	1587.340 m
Maximum height of masonry Dam	57.912 m
Maximum height of earthen Dam	33.528 m
Free Board above MWL	2.972 m

SPILLWAY

Length of Spillway Number of Gates Size of Gates Type of Gates

13 Nos. 15.240 m x 12.192 m Radial

237.744 m

OUTLETS

Length of LBC
Handia Branch Canal
Maximum discharge capacity of LBC
Length of RBC
Bagra Branch Canal
Pipariya Branch Canal
Maximum discharge capacity of LBC

Bagra Branch Canal
Pipariya Branch Canal
Maximum discharge capacity of LBC

131 km
94.76 cumecs
7.5 km
24 km
60 km

POWER HOUSE

Location Installed Capacity

Left Bank Canal 2 x 6.5 MW

Table 1 Monthly Inflow (M Cum) Series at the Tawa Reservoir for the Period (1948-71)

Year	Jan	Feb	Mar	Apr	Мау	Jun	Jul	Aug	dəs	Oct	Nov	Dec .
1948	1		1	•	ı	1	1449.342	2647.054	1800.885	246.697	222.027	50.573
1949	33.304	12.335	8.634	6.167	3.700	27.137	564.935	648.812	2362.120	424.318	99.912	30.837
1950	19.736	23.436	9.868	4.934	3.700	4.934	1434.541	474.891			18.502 1	16.035
1951	17.269	7.401	4.934	3.706	3.700	82.643		1038.593		94.978	12.335	7.401
1952	8.630	11.101	3.700	2.467	2.467		736.389	1087.932		18.502	7.401	6.167
1953	4.930	4.934	3.700	2.467	2.467	16.035	548.900	1883.528	314.538	80.176	14.802	9.868
1954	8.630	7.401	3.700	2.467	1.233	18.502	532.865	588.371	890.575	130.749		14.802
1955	11.100	7.401	3.700	2.467	2.467	102.379	125.815	1124.936	1300.091	869.605		24.670
1956	14.800	7.401	6.167	2.467	2.467	65.375		590.838	421,851	74.009		37.137
1957	30.840	3.700	17.269	13.568	3.700	24.670	145.551	1150.840	478.591	34.538	101.11	7.401
1958	4.930	3.700	3.700	2.467	2.467	25,903		1224,849	1586.259	241.763		27.137
1959	8.630	6.167	3.700	2.467	2.467	72.775		1807.052	3220.624	398.415		38.811
1960	99.910	24.670	20.969	0.000	0.000	59.207		1874.894	356.477	275.067		45.639
1961	19.740	9.868	0.000	0.00.0	0.000	18.502		2138.859	5442.127	540.366		71.542
1962	55.510	28.370	18.502	19.736	9.868	28.370		1145.906		130.749	-	46.872
1963	9.870	8.634	33.304	3.700	3.700	27.137		1873.661		130.749		31.410
1964	62.908	51.806	37.004	23.436	18.502	97.445		3332.871	418.151	974.452	49.339	44.405
1965	40.705	28.370	28.370	25.903	18.502	65.375	485.992	294.802		91.278		19.604
1966	17.269	9.868	9.868	4.934	3.700	N	1662.735		218.326	25.903		6.167
1967	4.934	3.700	4.934	2.467	2.467	111	1349.430	1010.223				53.040
1968	18.502	9.868	18.502	4.934	2.467	4	965.817	964.584		54.273	• •	17.269
1969	16.035	11.101	9.868	6.167	4.934	9.868	869.605	1614.629	859.738	44.405	18,502 3	11.101
1970	8.634	4.934	12.335	4.934	3.700	69.075		1427.140	2302.913	43.172	16.035 1	101.11
1971	7.401	4.934	3.700	2.467	1.233	88.811	1174.276	209.692	1633.131	128.282	1	1

Table 2 Elevation-Area-Capacity table for Tawa Reservoir (Based on 1990 Survey)

Elevation	Area	Capacity
m	M Sq.m	M Cu.m
m 326.140 327.664 329.188 330.712 332.236 333.760 335.284 336.808 338.332 339.856 341.380 342.904 344.428 345.952 347.476 349.000 350.524 352.048	M Sq.m 13.796 15.941 18.089 22.962 27.834 32.707 37.579 44.252 51.411 58.331 65.248 75.713 86.186 96.651 107.124 120.010 132.891 154.100	33.057 42.925 55.753 70.802 90.168 114.344 144.318 183.172 233.745 296.653 375.596 471.067 581.587 713.447 867.138 1044.760 1245.694 1470.188
353.572	175.342	1717.748
355.097	196.346	2028.586
356.621	217.373	2288.234
358.145	233.896	2602.032

Table 3 Normal Monthly Evaporation Depths (m) for Tawa Reservoir

Month	Evaporation	Month	Evaporation
Jan	0.121	Jul	0.091
Feb	0.121	Aug	0.091
Mar	0.152	Sep	0.121
Apr	0.213	Oct	0.121
May	0.304	Nov	0.121
Jun	0.243	Dec	0.121

Table 4
Target Monthly Demands from the
Tawa Reservoir

	Demands LBC		Upstream use Demands (M Cum)	Water Supply Demands (M Cum)
Feb Mar Apr May Jun Jul Aug Sep Oct	224.658 153.528 143.669 57.045 79.581 205.643 88.737 84.511 219.024 252.829 216.912	25.432 5 9.129	66.608	0.985 0.889 0.985 0.953 0.953 0.985 0.985 0.985 0.985 0.985

Table 5
Monthly Yield Estimation for Tawa Reservoir

Monthly Yield (M Cum))	
Month	Probabili	ty Level	for Value	to be Equ	salled or	Exceeded
	50%	60%	70%	75%	80%	90%
Jan Feb Mar Apr May Jun Jul Aug Sep Oct	14.249	11.760	9.706	8.750	7.846	5.985
	8.452	7.230	6.202	5.715	5.248	4.260
	7.545	6.321	5.323	4.860	4.423	3.518
	3.480	3.146	2.870	2.740	2.615	2.346
	3.174	2.741	2.371	2.194	2.023	1.657
	31.357	23.916	17.917	15.209	12.720	7.929
	701.127	580.966	467.807	410.206	352.599	225.151
	1189.448	1015.313	846.378	758.104	667.941	459.655
	949.810	755.282	589.391	511.172	437.001	286.530
	109.893	83.472	62.858	53.740	45.443	29.620
	31.392	23.831	17.804	15.105	12.636	7.914
	23.801	19.181	15.168	13.247	11.407	7.605

Table 6
Initial Rule Curve Levels (m) for Tawa Reservoir

Month	Upper Rule Curve	Irrigation Rule Curve	Water Supply & Upstream use Rule Curve
Jan	349.40	. 348.30	335.80
Feb	347.80	345.50	335.80
Mar	346.00	342.90	335.80
Apr	345.10	339.70	335.60
May	343.80	338.10	335.40
Jun	355.40	349.20	335.80
Jul	355.40	346.90	335.80
Aug	355.40	348.70	335.80
Sep	355.40	352.70	335.80
Oct	354.30	353.60	335.80
Nov	352.90	351.90	335.80
Dec	351.50	350.00	335.80

Table 7
Recommended Rule Curve Levels (m) for Tawa Reservoir

Month	Upper Rule Curve	Irrigation Rule Curve	Water Supply & Upstream use Rule Curve
Jan	352.55	347.60	335.80
Feb	351.30	342.20	335.80
Mar	350.05	339.60	335.80
Apr	349.40	336.60	335.60
May	348.45	335.90	335.40
Jun	353.40	337.00	335.80
Jul	354.90	339.60	335.80
Aug	355.40	348.00	335.80
Sep	355.40	352.40	335.80
Oct	355.40	353.00	335.80
Nov	354.60	351.60	335.80
Dec	353.75	349.10	335.80

Table 8
Simulation of Tawa Reservoir with Current Operation Policy

YYY-Mn-D	Ini_Sto m m3	Loc_Flo	Evapr m m3	Tir Dam m m3	Pw_Dem M kwh	Ws_Dem m m3	Tds_Dem n m3	Releas in #3	Pw Gen M kwh	Spill m m3	End_Lev	Dal m	Frl m
948-06-0 948-07-0 948-08-0	678.8 414.2 1647.4	.0 1449.3 2647.1	19.9 10.9 16.2	243,8 115.4 100.9	.0	1.0 39.8 67.6	.0	244.7 205.2 177.5	9.4 7.6 7.1	.0 .0 2216.3	342.00 353.14 354.39	334.24 334.24 334.24	354.3 354.3 354.3
948-08-0 948-09-0	1684.5	1800.9	22.6	278.2	. 0	67.6	.0	345.7	9.4	1432.6	354,39	334.24	354.3
348 10:0	1884.5	246.7	22.3	298.4	. 0	1.0	. 0	299.4	9.4	.0	354.02	334.24	354.3
948-11-0	1889.5	222.0	21.7	275.4	. 0	1.0	.0	276.4	9.4	.0	353.65	334.24	354.3
949-12-0	1793.5	50.6 33.3	20.4	214.0 285.5	. c	1.0	. 0	215.0 246.5	9.4	.0	352.53 350.74	334.24	354.3
943-31-0	1277.6	12.3	15.6	193.1	.0	.9	. 0	193.8	9.4	. 0	349.27	334.24	354.3
947 83 0	1080.5	8.6	17.5	177.2	.0	1,0	. 0	178.1	9.4	. 0	347.70	334.24	354.3
349-84-0	693.4	6.2	22.7	57.3	, 0	1.0	. 0	58.0	3.7	, n	347.00	334.24	354.3
949-85 0	818.9	3.7	30.5	79.6	. 0	1.0	. 0	80.6	5.1	.0	345.93	334.24	354.3
243.36.0	711.6	27.1	20.0	243.6	, 0	1.0	.0	244.7	2.4	.0	342.93	334.24	354.3
945-07-0	473.0	564.1	8.2	115.4	.0	85.8	. 0	205.2	7.0	.0	347.05	334.24	354.3
349-38-0	824.5	648, H	10.0	109.9	. 0	67.6	.0	177.5	s.a	.0	330.79	334.24	354.3
9429-10-0	1244.8	2362.1	19.6	278.2	, u	67.6	. 0	345.7	9.4	182.4	354.39 354.39	334.24	354.3
247-10-0	1884.5	424.3	22.6	275.4	.0	1.0	.0	275.4	3.4	.0	353.38	334.24	354.3
943-12-0	1686.3	30.8	19.0	214.0		1.0	. 5	215.0	9.4	.0	352.12	334.24	354.3
250-01-0	1482.3	19.7	17.2	285.5	. 0	1.0	. 0	286,5	9.4	.0	350.16	334.24	354.3
350-02-0	1190.3	23.4	15.0	172.3	. 0	. 9	. 0	193.6	9.4	.0	348.73	334.24	354.3
950 05 0 950:04 0	1013.0	9.9	16.9 21.7	177.2 57.0	.0	1.0	. B	178 : 58 :0	9.4	.0	347.09	334.24	354.3
950-05-0	753.0	3.7	29.0	79.6	.0	1.0	. 0	80.6	4.9	.0	345.19	334.24	354.3
950:06 0	647.2	4,9	19.2	243.8	. 0	1.0		244.7	9.4	.0	941.58 952.89	334.24	354.3
950-07-0	386.2	1434.5	10.6	115.4	. 9	89.8	.0	205.2	7.5 6.7	3.7	354.33	334.24	354.3
950 08:0 950 09 0	1606.9	474.9	22.6	278.2		67.6 67.6		345.7	5.4	275.6	354.32	134.24	354.3
950-10 0	:884.5	22.2	41.2	290.4		1.6	10	299.4	9.4	.0	352.76	334.24	164.1
950-11-0	1506.1	18.5	18.3	275.4		1.0	. 0	276.4	9.4	.0	350.06	334.24	354.0
950 12 0	1309.9	15.0	15.9	214.0	:	1.6	. 0	215.3	9.4	.0	343.38	334.24	354.0
951 21:0	1005.1	17.3	19.7	285.5	. 6	1.0	. 0	286.5 123.8	9.4	, 0 0	344.80	334.24	354.0
951-03-0	612.2 (14.1	7.4 4.7	11.6	192.3	. 0	1.0	. 0	178.1	9.4	.0	342.23	334.24	35.6
951-04-0	428.8	3.9	14.4	57.0	. 6	1.0	. 0	58.0	3.0	. 0	341,Dd	334.24	354.0
951 05 0	340.1	5.9	1:1.1	99.6	.0	1.0	. 6	80.6	4.0	, U	339.09	334.24	354.0
				243.8	. 0	1.0	r.	253,11		, α	334.24	334.24	354.3
551-06 C 981-07 C	265.0	82.6 213.4	10.8	115.9	.0	60.a		205.2	4.3	.0	334.50	334.24	354.3
551 - 03 - B	128.8	1038.6	6.9	109.9	.0		. 0	177.5	4.5	, 0	346.49	334.24	774.
151 09 0	983.1	915.6	14.6	278.2	. 3	67.6	.0	345.7	9.4	. 0	349.71	334,24	354.0
951-10-0	1100.3	95.0	14.3	298.4	. 0	1.0	. 0	297.4	9.4	. 0	347.93	334.24	354.5
951 11 0	919.6	12.3	12.2	275.4	.1;	1.0	.0	276.4 215.0	9.4	. 0	345,14	334.24	354.3 354.3
951-12-0	543.3 425.9	7.4	5,5	285.5	.0			286.5	1.4	.0	335.14	334.24	354.3
352-02-0	141.0	11.1	4.3	192.5	, 0		. a	04.50	1.7	.0	534.24	334.24	354.
352-03-0	125.9	3.7	5.2	1.19.5	.0		. 3	1,80	1.15	.0	334.12	334.24	354.0
952-04-0	121.4	2.5	7.1	57.0	, ()		. 3			. 0	333.63	334.24	354.
952 95 9	115.9	2.5	9.8	79.0	. 0			1.00		. 0	333,33	334.24	354.
952 06 0	107.5	58.0	8.0	243.8	, n			33.70		. 6	334.24	334.24	354.1
952 07 0	123.8	736.4	5.7	115.4	.0			265.2	50,7	.0	345.21	334,24	354.
952-08-0	649.3	1087.9	11.5	105.9	.0			177.5	5.3	, ti	352.53	334.24	354.
952 09 0	1548.2	1557.4	18.3	278.2	. 0		.0		9.4	.0	351.16 348.98	334.24	354.
.452 10 0 .952-11-0	1339.6	16.5	15.8	290.4 275.4	.0		.0		9.4	.0	346.42	334.24	354.
.952-11-0 .952-12-0	760.6	7.4 6.2	11.0	214.0	. 13	1.0		215.0	9.4	.0	345.87	334.24	354.
953-01-0	540.H	4.9	4.2	265.5	, 13	1.0	. 9	236.5	9.4	.0	338.75	534.24	354.
953 02:0	251.0	4.9	5.3	192.3	. 0	. 9	. 0		1.1	. 0	334.24	334.24	354.
953 03 0 955-04-0	123.8	3.7	7.1	177.2	. 0			1.00	0	. 0	334.12	334.24	354.
953-04-0	115.9	2.5	5.8	79.6	.0				.0	. 0	333.33	334.24	354.
953-06-0	187.5	16.0	7.8	243.8	. 11	1.0	. 0	1.00	5.3	.0	333.78	334.24 334.24	354.
953-07-0 953-08-0	114.8 453.6	54a.9 1883.5	4.8	115.4	.0		o, o.		5.9	.263.3	342,53	334.24	354.
953-08-0	1884.5	314.5		274.2					9.4	.263.3		334.24	354.
353 10-0	1650.9	80.2		298.4				239.4	9.4	.0	352.79	334.24	354.
753 - 11-8	1590.7	14.8	18.4	275.4	100	1.0		276.4	3.4	. 0	350,97	334.24	354.
953-12-0	1310.9	9.5	10.8	214.0	. 0				9.4	. 0	349.34	334.24	354.
954-01-0	1080.8 798.4	H.6 7.4	13.6	285.5	.0	1.0	.0	193.6	9.4	. 0	346.79 344.65	334.24	354.
1954 - 02 - 0 1954 - 03 - 0	798.4 600.4	3.7		177.2			.0		9.4	.0	341.99	334.24	354,
954 - 04 - 0	414.0	2.5	24.1	57.0	. 5	1.0		50.0	3.3	.0	340.78	334.24	354.
954-05-11	344.4	1.2	17.5	79.6	i, i	1.0		80.6	3.9	.0	338.67	334.24	354.
1954-06-0	247.6			243.8		1.6		T31.E0			339.24	334,24	354.
1954-06-0	123.8		4.9	115.4					5.3	. 0	342.51	334.24	354.
1954-08-0	146.6	588.4	8.1	109,9	. (177.5	5.1	. 6	347.30	334.24	354.
954-09-0	899.3	890,6	15.2	278.2	. (67.3		345.7	9.4	.0		334.24	354.
1954 - 10 - 0	1978.0	130.7	26.6	298.4	- 4				9.3	.0		334.24	354.
1954-11-0	1199.7			275.4					9.4	. 0 . 0	348.02	334.24	
	931.1								9.4	.0		334.24	
1954 12-0													
1954 12-0 1955-01-0	718.2 452.7			192,9		:	, ;		9.4	. 3		334.24	354.
1954 12-0	716.2 452.7 270.0 123.0	7.4	7.5 6.6	177.2		1.0) .:	112.10	3 4.1	. 0	374.24	334.24 334.24	354

AAAA Wu D	Ini_Sto m_m3	Lec_Flo m m3	Evapr m m3	Tir_Dem m_m3	Pw_Dem M kwh	Ws_Dem m_m3	Tds_Dem Tails3	Releas m m3	PwjGen Mixwh	ვლე11 თ თვ	End_Lev m	Dal m	Fri m
955-06-0	109.7	102.4	8.0	243.8	. 0	1.0	.0	80.30	2.2	. 0	334.24	334.24	354.3
1955-07-0	123.8 123.8	125.8	7.1	115.4	. 0	89.6 67.6	. n	122.70 177.5	1.6	.a .a	334.24 349.25	334.24	354.3
1955-09-0	1364.2	1300.1	18.6	278.2	. 0	67.6		345.7	9.4	115.4	354.39	334.24	354.3
1955-10-0	1004 5	869.6	22.6	258.4	. 9	1.0	. 9	239.4	9.4	547.6	354.39	334.24	354.3
1955-11-0 1955-12-0	1628.6	41.9 24.7	21.4	275.4	. 0	1.0	.0	276.4	9.4	. 0	353.02	334.24	354.3
1956-01-0	1414.1	14.8	15.6	285.5	. 0	1.0	.0	286.5	9.4	. с	349.65	334.24	354.3
1956-02-0	1130.6	7.4	14.4	192.9	. 9	. 9	,0	193,8	9.4	. 0	348.02	334.24	354.3
1956-03-0 1956-04-0	930.0 742.1	5.2 2.5	16.0 20.4	177.2 57.0	. 0	1.0	.0	178.1	9.4	, D	345.41	394.24 334.24	354.3
1956-05-0	666.2	2.5	26.9	79.6	. 9	1,0	.0	80.6	4.8	. 12	344.15	334.24	354.3
1956-06-0	561.1	65.4	18.0	243.8	. 6	1.0	.0	214.7	9.4	.0	341,15	334.24	354.3
1956-07-0	363.7	801.8	8.1	115.4	, 0	89.8	.0	205.2	6.9	. 0	348.21	334.24	354.3
1956-0H-D	952.2	590.8	11.7	109.9	. 0	67.6	.0	177.5	6.0	. 0	351.26	334.24	354
1956-09-0 1956-10-0	1353.8	421.9 74.0	16.7	278.2	.0	67,6	.0	345.7	9.4	. 0	351.66	334,24	354.3
1956-11-0	1170.2	150.5	14.9	275.4	. 0	1.0	.0	276.4	9.4	.0	348.67	334.24	354.3
1956-12-0	1029.4	27.1	13.5	214.0	. 0	2.0	.0	215.0	3.4	, α	347,09	334.24	354.3
1957 0)-0 1957-02-0	828.0 561.0	30.a 3.7	21.4	265.5	. 0		.0	206.5 193.d	9.4	.0	344.14	334.24	354.3
1957-03-0	361.6	17.3	8.3	177.2	. 5	1.0	.0	174.1	8.5	.0	337.09	334.24	354.3
1957-04-0	192.6	13.6	8.8	57.0	, a	1.0	.0	50.0	2.3	, 0	335.04	334.24	354.3
1957 05-1	139.4	3.7	10.8	79.6		1.0	.0	8.50	2	.0	334.24	334,24	354,3
1957:06 0	123.8	24.7	8,3	245.8	. 0	1.0	.0	16.30	. 4	. 0	334.24	334.24	354.3
1957-107-0	123.8	145.6	3.1	125.4	. 9	a9.e	.0	142.40	1.7	. 0	334.24	334.24	354.3
1957-08-0	125.8	478.6	7.2	109.9 278.2	. 0	67.6 67.6	.0	177.5	4.5	. C	349.34	334.24	354.3
1957-10-0	1207.5	34.5	14.6	298.4	. 0	1.0	. 0	209.4	9.4	. 0	348.00	334.24	354.0
1957-11-0	વ્2શ. ા	11.1	12.3	275.4	. 9	1.0	, а	276.4	2.4	. 0	345.22	334.24	354.3
1957 l2 u 1958-:1-0	650.5 433.0	7.4	7.7	214.0 265.5	. 0	1.0	.0	215.0	9.4	.0	392.30	334.24	354.3
1958-01-0	144.9	3.7	4.4	192.9	. 0	1.0	.0	20.40	9.4	, D	334.24	334.24	354
1058-03-0	123.8	3.7	5.2	177.2	. 0	1.0	.0	1.00	. 0	. 0	334.12	334.24	354.
1958-04-u 1958-05-U	121.4	2.5	7.1	57.0 79.6	. 0	1.0	.0	1.00	. 0	.0	333.63	334.24	354.3
1958 06-0 1958-07 0	107.5 123.8	25.9 421.9	4.9	249.8 115.4	, B	1.0	.0	1.60	. n 5 . 1	.0	334.24	334.24	354.3
1954-48-9	376.1	1224.8	9.4	109.3	. 0	67.6	. 0	177.5	5.3	.0	351.39	334.24	354.
1958-09-0	1374.0	1586.3	200.1	278.2	. 0	57.6	, п	345.7	9.4	710.0	354.39	334.24	354.3
1958-10-0 1958-11-0	1834.5	241.8 102.4	22.3	29H, 4 275, 4	. 0	1.0	. 0	276.4	9.4	. 0	354,00 352,91	334.24 334.24	354 3
1958 12 0	1609.6	27.1	19.0	275.4	0.	1.0	. 0	215.0	7.4	.0	351,59	334.24	354.3
1959 03-0	1402.8	81.6	10.4	205.5	.0	1.0	.0	286.5	9.4	.0	349.48	334.34	354.3
1959-02-0 1959-03-0	1108.5 306.7	6.2 3.7	14.2	192.9	.0	.9 1,0	. U	193.a 178.1	9.4	.0	347.82 345.98	334.24	354.3 354.3
1959-04-0	716.5	2.5	20.0	57.B	.0	1.0	.0	54.0	3.5	, 31	345.11	334.24	354.3
1959-05-0	641.0	2.5	26.3	79.6	. 0	1.0	. 0	80,6	4.7	. 0	343.81	334.24	354.3
1959 DG-D	536.6	72.8	17.6	243.8	.0	1.0	. 6	244.7	9.4	.0	340.83	334.24	354.3
1959-07-0	347.1	2243.9	11.3	115.4	.0	89.8	. 6	205.2	8.0	495.9	354.39	334.24	354.3
1959 08-0 1959-09-0	1884.5	1807.1	17.0	104.9	.0	67.6	. 0	177.5 345.7	7.2	1612.5	354.39	334.24	354.3
1959 10-0	1864.5	376.4	22.6	235.4	.0	1.0	, D	299.4	9.4	76.5	354.39	334.24	354.3
1959-11-0	1884.5	119.6	21.8	275.4	. 9	1.0	.0	276.4	9.4	. 0	353,50	334.24	354.3
1959 12:0 1960:01:0	1765.3	99.5	20.3	214.0	. 0	1.0	.0	215.0	9.4	.0	352.60 351,26	334,24	354.3
1960-02-0	1354.4	24.7	16.4	192.9	.0	. 9	.0	193.a	9.4	.0	349.94	334.29	354.3
1960-03-p	1160.0	21.0	18.6	177.2	.0	1.0	.0	178.1	9.4	. 3	348.56	334.24	354.3
1960-04-0 1960-05-0	993.1 911.0	. c	24.1	57.0 79.6	.0	1.0	.0	58.0 28.5	3.6 5.2	. 0 . a	347.85 346.79	334.24 334.24	354.3 354.3
1960-06-17 1960-07-13	798.1 589.5	59.2 445.3	23.0	243.8 115.4	.0	1.0 6.9.8	.0	244.7	9,4	.0	344.52	334.24	354.7
1960-08-0	820.9	1874.9	13.2	109.9	.0	67.5	.0	177.5	€.5	620.6	354.39	334.04	354.3
1960 09-0	1884.5	356.5	22.5	278.2	.0	67.6	.0	345.7	9.4	. 0	354.33	334.24	354.
1960-10-0 1960-11 D	1872.7	275.1 60.4	22.3	298.4 275.4	.0	1.0	.0	299.4	9.4	.0	354.10 352.78	334.24	354.1 354.1
1960-12-0	1589.2	45 €	16.9	214.0	.0	1.0	.0	215.0	9.4	.0	351.58	334.24	354.
1961-01-0	1401.0	19.7	16.5	285.5	. 0	2.0	. 0	246.5	9.4	. 9	349.55	334.24	354
1961-02-0 1961-03-0	919.5	9.5	14.3	192.9	.0	. 9	.0	178.1	9.4	. 0	347.93	334.24	354
1961-04-0	725 6	.0	20.1	57.0	. 0	1.0	.0	58.0	3.5	, D	345.19	334.24	354
1961-05 0	647.5	. 0	26.4	79.6	. a	1.0	٥.	ND.6	4.7	. 0	343.86	334.24	354.
1961-06-0	540.5	18.5	:7.1	243.8	. 0	1.0	, a	244.7	9.4	.0	339.87	334.24	354.
1961-07-0	297.2	1064.5	8.4	115.4	. 0	39.8	. 0	205.2	6.9	. 0	349.78	334.24	354.
1961-08-0 1961-09-0	1148.0	2138.9 5442.1	14.3 22.6	109.9 278.2	. 0	67.6 67.6	.0	177.5 345.7	6.7 3.4	1210.6 5073.8	354.39 354.39	344.24 334.24	354.
1961-10-0	1884.5	540.3	22.6	298.4	. 0	1.0	.0	299.4	9.4	218.3	354.39	334 24	354.
1961 11-0	1884	95.0	21.7	275.4	. 0	1.0	- 0	276.4	7.4	.0	353.35	334.24	354.
1961-12-0	1681.4 1517.9	71.5 55.5	20.0 17.7	214.0 285.5	.0	1.0	.0	215.0 286.5	9,4	.0	352.34	334.24 334.24	354. 354.
1962-02 0	1269.2	28.4	15.6	192.9	.0	. 9	. 0	193.8	9.4	.0	349.33	334.24	354
1962-03-8	1888.2	18.5	17.7	177.2	, 0	1.0	, 0	178.1	9.4	. 0	347.85	334.24	354.
1962-04-0 1962-05-0	910.8 649.5	19.7	31.1	57.0 79.6	.o.	1.0	.0	50.0 80.6	3.6 5.1	. ú	347.30	334.24	354.1 354.1

YYYY-Mii-1:	Taljaro n an	boo Flo m m3	Evapr m m3	Tir_Dem m m3	Pw.Dem M.kwh	₩a_l/em m m3	Tds_Dem m. m3	Reloas m m3	Pw_Gen M kwh	Spill m.mb	End_Lev	Dsl m	Fr1 m
1982-03-0 1652-03-0 1962-03-0 1962-03-0 1962-10-0 1962-11-0 1963-02-0 1963-02-0 1963-03-0 1963-03-0	747. 7 509. 6 819. 1 1774. 6 1884. 5 1884. 1 1410. 6 1225. 5 704. 2 706. 3 577. 4	28.4 525.9 7(45.7 2299.2 130.7 12.3 46.9 9.7 8.6 93.3 3.7	21.7 2.3 12.9 22.1 21.6 13.5 16.9 14.7 12.7 14.0 17.5 22.4	243.8 115.4 109.9 278.2 256.4 275.4 214.0 285.5 192.9 177.2 57.0 79.6	.0	1.0 49.8 67.6 67.6 1.0 1.0 1.0 1.0 1.0	.0 .0 .0 .0 .0 .0 .0 .0 .0	249,7 295,2 177,5 345,7 253,4 276,4 215,0 265,5 173,3 178,1 58,0 60,6	9.4 7.0 6.1 9.4 9.4 9.4 9.4 9.4 9.4 9.4	.8 .3 .8 1921.4 .0 .0 .0 .0	343,44 347,00 353,85 354,35 351,64 350,37 348,65 346,18 346,18 343,38 341,27	334.24 334.24 334.24 434.24 334.24 334.24 334.24 334.24 334.24 334.24	354.39 354.39 354.39 354.39 354.35 354.35 354.39 354.39 354.39 354.39
1983 06 0 1983 07 6 1963 08 6 1963 08 6 1965 09 0 1984 10 0 1983 11 0 1984 01 6 1984 02 0 1984 02 0 1984 02 0 1984 02 0	400.3 175.1 150.4 1810.6 1804.5 1604.1 1464.2 1532.7 10.5.7 15.6.8 775.5 728.6	27.1 164.1 1873.2 1762.6 130.7 do.5 41.4 60.9 51.8 37.0 23.4 18.5	13.5 3.6 9.9 22.5 21.d 19.0 17.9 16.0 14.2 16.2 21.1 28.5	243.8 115.4 109.9 278.2 288.4 275.4 214.0 285.5 102.9 177.2 57.0 79.6	.0	1.0 89.8 69.6 57.6 1.0 1.0 3.0 1.0 1.0		244.7 205.2 177.5 345.7 205.4 276.4 215.0 286.5 178.1 56.0 80.6	0.4 4.6 5.0 9.4 9.4 9.4 9.4 9.4 9.4 9.4 9.4	.0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0	335.49 334.58 354.06 353.49 352.13 351.11 349.37 348.07 346.61 346.05 345.02	334.24 334.24 334.24 334.24 334.24 334.24 334.24 334.24 334.24 334.24	354.39 354.39 354.39 354.39 354.39 354.39 354.39 354.39 354.39
1964-97-0 1964-97-0 1964-98-0 1964-98-0 1964-11-0 1964-11-0 1965-91-0 1965-92-0 1965-92-0 1965-93-0 1965-95-0	633.2 465.8 1148.1 1884.5 1884.5 1636.8 1440.0 1180.3 1502.9 836.3 780.2	97.4 896.7 3332.9 418.2 874.5 44.4 40.7 28.4 29.4 25.0 18.5	20.1 9.2 14.3 22.6 22.4 21.5 19.4 17.0 14.9 16.9 22.3	243.8 115.4 109.9 278.2 238.4 275.4 214.0 235.5 192.0 177.2 57.0 70.6	,0 .0 .0 .0 .0 .0 .0 .0 .0	1.0 89.a 67.6 1.0 1.0 1.0 1.0 1.0 1.0	.0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0	244.7 205.2 177.5 545.7 299.4 276.4 215.0 266.5 193.8 178.1 56.0 a6.6	9.4 9.4 9.4 9.4 9.4 9.4 9.4 9.4	.0 .0 2404.7 49.9 652.5 .0 .0 .0	342.82 349.78 354.39 354.39 353.07 353.07 351.88 350.05 348.64 347.17 346.63	334.24 334.24 334.24 334.24 334.24 334.24 334.24 334.24 334.24 334.24 334.24	354.39 354.39 354.39 354.39 354.39 354.39 354.39 354.39 354.39 354.39
1965 46.0 1965-07 0 1965-08-6 1965-0-0 1965-10-0 1965-11-0 1966-01-0 1966-01-0 1966-01-0 1966-01-0 1966-01-0	690.3 410.0 762.7 970.5 1864.8 1397.5 1105.3 911.6 715.1 533.3 463.6	65,4 486.0 294.4 1425.9 91.5 38.2 29.6 17.3 4.0 9.3 4.9	26.9 6.1 7.4 17.8 21.6 19.2 18.8 14.5 12.5 13.6 16.7 21.2	243.8 115.4 109.9 276.2 298.4 275.4 214.0 245.5 132.9 177.2 57.0 79.0	.0	1.0 89.8 67.6 67.6 1.0 1.0 1.0 1.0 1.0	.0 .0 .0 .0 .0 .0 .0 .0	244.7 205.2 177.5 345.7 299.4 275.4 215.0 286.5 170.1 5d.0 80.6	9.4 6.9 9.4 9.4 9.4 9.4 9.4 9.4	. 0 . 0 . 0 . 0 . 0 . 0 . 0 . 0 . 0	543.17 345.44 347.51 354.39 353.18 351.55 350.14 347.86 345.97 342.76 342.76	334,24 334,24 334,24 334,24 334,24 334,24 334,24 334,24 334,24 334,24 334,24	354.39 354.39 354.39 354.39 354.39 354.39 354.39 354.39 354.39 354.39
1960-06 0 1960-07 0 1960-03 0 1960-09 0 1960-11 0 1964-12 0 1967-03 0 1967-03 0 1967-03 0 1967-03 0	365.5 109.6 1592.4 1864.5 1795.1 1441.8 154.8 031.5 437.7 437.7 255.6 184.9	2.5 (6) 2.7 (1) 73.0 (2) 8.3 (5) 2 (6) 2 (4) 9 (4) 9 (4) 9 (2) 5 (2) 5	12.0 9.0 15.5 22.0 19.8 16.6 10.5 12.2 9.6 5.6 10.5	249.8 115.4 109.9 278.2 298.4 275.4 214.0 205.5 132.9 179.2 57.0 79.6	.0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0	1,0 67.6 67.6 1.0 1.0 1.0 1.0 1.0	. 6 . 6 . 6 . 6 . 6 . 6 . 6 . 6 . 8	232.21 205.2 177.5 345.7 297.4 276.4 215.0 286.5 193.8 178.1 58.0 55.50	9.4 6.6 7.0 5.4 4.4 9.4 9.4 5.0 2.4	,0 ,0 ,0 ,0 ,0 ,0 ,0 ,0 ,0 ,0 ,0 ,0 ,0 ,	334,24 352,68 353,66 353,66 343,83 348,83 348,83 345,88 542,87 338,85	334.24 334.24 334.24 334.24 334.24 334.24 534.24 534.24 334.24 334.24 334.24	354.39 354.39 354.39 354.39 354.39 354.39 354.39 354.39 354.39 354.39
1967-06-0 1967-07-0 1967-08-0 1967-08-0 1967-10-0 1967-11-0 1967-11-0 1968-01-0 1968-03-0 1968-03-0 1968-03-0	12%, 8 12%, 8 12%, 8 12%, 4 1884, 5 1637, 6 1637, 6 1638, 6 1638, 6 1638, 6 17, 8 444, 2	111.0 1349.4 1010.2 1040.3 74.0 95.0 18.5 9.1 18.5	8.3 7.7 14.6 22.6 31.5 16.4 14.3 12.3 13.3 16.3 20.7	263.8 115.4 109.9 278.2 208.4 214.0 285.5 1927.2 57.0 77.6	. 0 . 0 . 0 . 0 . 0 . 0 . 0 . 0 . 0 . 0	1.0 d5.8 67.6 1.0 1.0 1.0 1.0 1.0	.0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0	103.70 205.2 177.5 345.7 207.4 207.4 215.0 236.5 103.3 178.1 58.0 69.0	7.5 5.3 6.4 9.4 9.4 6.4 6.4 6.4 6.4 7.4	.8 304.8 677.7 .6 .9 .8 .9 .9	354.24 350.52 354.30 354.30 354.30 351.21 349.94 347.64 348.60 348.60 342.55	334.24 A34.24 A34.24 334.24 334.24 334.24 334.24 334.24 334.24 334.24 334.24	354,39 354,39 354,39 354,39 354,39 354,39 354,39 354,39 354,39
1964 06-11 1968 09-11 1968 08-1 1968 10-1 1968 11-1 1968 11-1 1968 12-1 1969 01-1 1969 02-1 1969 03-1 1969 03-1 1969 03-1	120.4 120.8 100.0 160.1 160.1 1548.0 100.5 100.5 823.3 628.8 448.1 581.5	1.9 954.0 954.0 954.0 97.1 17.5 16.1 9.2 4.7	11.8 - 6.5 - 10.6 - 21.4 - 11.1 - 18.4 - 16.0 - 14.8 - 11.7 - 16.4 - 14.8 - 18.7	24%, 8 115, 4 100, 5 278, 2 208, 4 214, 0 285, 5 192, 9 177, 2 57, 0	. 0 . 0 . 0 . 0 . 0 . 0 . 0 . 0 . 0 . 0	1.9 87.6 87.6 67.5 1.0 1.0 1.0 1.0 1.0	.6 .0 .0 .0 .0 .0 .0 .0	218.71 200.2 177.5 345.7 217.4 275.4 275.0 285.5 173.0 178.1 52.0 00.6	1.55 5.6 9.4 1.4 2.4 9.4 9.4 9.4 14.0	.0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0	456.24 267.57 254.25 354.25 352.26 352.84 442.66 147.04 344.77 342.44 341.67 341.67	334, 14 334, 24 334, 24 334, 24 334, 24 334, 24 334, 24 334, 24 334, 24 334, 24 334, 24	354,39 354,30 354,30 354,30 354,30 354,39 354,39 354,39 354,30 354,30

YYYY Mn-D	ini_Ste m m3		Evapt m m3	Tir_Dem m m3	Pw_Dem M kwh	Wis_Dem เกิดเวิ	Tds_Dem m m3	Releas m m3	Pw_Gen M_kwh	Spill m m3	End_Lev m	[38] [8	Fri m
1969-06-0	287.1	9.9	11.1	243.8	. 0	1.0	.0	162.0C	5.7	. 0	334.24	334.24	354.39
1369-67 0	123.8	862.6	6.2	115.4	. 0	39.6	.0	205.2	5.8	. 0	346.63	334.24	354.33
1969-08-0	782.0	1614.6	13.1	109.3	. 12	67.6	. 0	197.5	6.3	321.6	354.39	334.24	354.39
1969 09-0	1864.5	859.7	22.6	278.2	.0	67.6	. 0	345.7	9.4	491.4	354.39	334.24	354.33
1969-10-0	1884.5	44.4	21.3	296.4	.0	1.0	. 9	299.4	9.4	. 17	352.90	334.24	354.39
1369-11-0	1608.2	18.5	18.6	275.4	.0	1.0	. 0	276.4	9.4	. 0	351.11	334.24	354.39
1960-12-0	1331.8	11.1	16.1	214.0	. 0	1.0	. 0	215.0	9.4	. 0	349,51	334.24	354.19
1970 01-0	1111.8	8.6	13.8	265.5	. 0	1.0	.0	286.5	9.4	. 6	347.01	334.24	354.39
1970-02-0	820.2	4.9	11.7	192,3	. 3	. 9	. 0	133.8	9.4	. 0	144.17	334.24	354.39
1970-03-0	h10.6	32.3	12.3	177.2	. 11	1.0	.0	178.1	9.4	.0	342.43	334.24	354.37
1970 04-0	441.5	4.9	14.6	57.0	. 0	1.6	. 0	50.0	3.1	.0	341.54	334.24	354.30
1970-05-0	373.8	3.7	18.5	79.6	. 0	1.0	. 0	80.5	4.0	.0	339.41	134.24	354.32
1270 06-0		65.1	11.0	243.8	.0	1.0	. 0	212.71	ხ.6	.0	334,24	334.24	354.39
1970 06-0	278.4 123.6	551.4	5.0	115.4	.0	69.8	. 10	205.2	5.4	. 0	342.81	334,24	354.39
1970-08-0	465.0	1427.1	11.3	109.9	.0	67.6	.0	177.5	5.7	. 0	353,49	334.24	354.39
1970-08-0	1703.3	2302.9	21.8	278.2	. 5	67.5	. 0	345.7	9.4	1754.2	354.39	134.24	354.35
1970-10-0	1884.5	43.2	21.3	298.4		1.0	. 0	293.4	9.4	.0	350.83	434.24	354.59
1970-10-0	1607.0	16.0		275.4		1.0	. 0	276.4	9,4	, 0	551.08	334.24	354.39
1970-11-0	1328.1	11.1	16.0	214.0	.0		. 9	215.0	9.4	.0	349.48	311.24	354.35
1971-01-0	1108.2	7.4	13.8	285.5	.0		. 0	286.5	9,4	. 0	346.36	334.24	354.39
		1.9	11.6	192.9	.0			193.0	9.4	. 0	344.81	334.24	354.39
1971-02 0	815.3	3.7	12.1	177.2	, 0			178.1	9.4	. 6	342.22	354,24	354.39
	614.8	2.5		57.0	.0			58.0	9.0	.0	341.05	334.24	354.35
1971-04-0	428.2	1.2			. 0				9.0	. 0	333.97	334.24	354.39
1971-05-0	358.3	1.2	18.0										
1971-06-0	261.0	63.63	10.8	243.8	.0	1.0		215.21		. 0	334.24	334.24	354.39
1971-07-0	123.8	1174.3	7.1	115.4	. 0		. 13		6.2	. 11	349.31	334.24	354.35
1971-08 0	1085.7	209.7			.0	67.4	. 0	177.5	5.9	.0	349.47	334.24	354.39
1971-09-0	1105.7	1633.1	18.8		.0		.0		9.4	490,8	354.39	334.24	354,30
1971 10 0	1864.5	128.3			. 0				2.4	.0	959.41	334.24	354.39

Number of Failures for WS. = 0, Time Reliability = 1.00

Number of Failures for IRR. = 32, Time Reliability = .886

Number of Critical Failures = 27 (Release < .75 * Total Demand)

Volume Reliability for IRR. & WS. = .94

Table 9
Simulation of Tawa Reservoir with Recommended Operation Policy

YYYY Mn-D	Ini 22 o	log Kin	F	m1 - 1				141101					
ļ	.0 10 3	lec_Flo □ π3	Evapr 6 m3	11, 1,3	M kwh	m m3	Tris_Dom m m3	Releas m m3	Pw_Gen M_kwh	Spili m m3	End hev	Md1 Rul	Upr_Rul
1998 36-0 1998 38-0 1948 38-0 1948-09-0 1948-10-0 1948-11-0 1948-12-0 1949-01-6 1949-01-6 1949-01-6 1949-01-6	1406.0 723.2 1964.1 2079.7 2079.7 2043.1 1925.5 1739.0 1465.4 1266.8 1677.8	.0 1447.3 2647.1 1808.9 246.7 222.0 50.6 33.3 121.3 8.6 6.2 3.7	26.1 13.2 17.8 24.3 23.9 23.2 22.2 20.0 17.5 19.4 25.4	243.8 115.4 109.5 278.4 275.4 214.0 285.5 192.5 177.2 57.0 70.6	0.0 0.0 0.0 0.0 0.0 0.0 0.0	1.0 69.8 67.6 57.6 1.0 1.0 1.0 1.0 1.0		244.7 205.2 177.5 345.7 296.4 276.4 215.0 266.5 193.8 176.1 56.8 80.6	9.4 8.3 7.2 9.4 9.4 9.4 9.4 9.4 9.4 9.4	.0 .0 2332.1 1433.9 .0 .0 .0 .0 .0 .0	346.11 354.76 355.40 355.40 355.47 354.59 353.68 352.02 350.67 349.25 348.62 347.67	337.00 339.40 348.00 352.40 353.00 351.60 347.50 347.50 339.60 336.62	353,40 354,90 355,40 355,40 355,40 354,60 352,55 351,30 350,05 349,40 348,45
1945 08-8 1242-17 0 1242 08-0 1244 08-0 1244-18 0 1245-11 0 1245 11 0 1250-02-0 1850-03 0 1850-03 0 1850-03 0	889, 4 947, 5 97, 7 145, 7 2079, 7 2075, 7 1675, 6 1674, 6 1884, 1 1200, 7 1013, 9 936, 3	27.1 554.9 (48.8 2362.1 424.3 57.9 30.8 19.7 23.4 5.9 4.9 3.7	24.3 9.5 12.3 21.4 24.3 23.4 21.7 19.2 16.7 18.8 24.5 32.9	243.8 115.4 107.3 278.2 288.4 275.4 219.6 285.5 192.7 57.0 79.6	.0.00	1.0 89.8 67.6 57.6 1.0 1.0 1.0 1.0	.0 .0 .0 .0 .0 .0 .0 .0	244.7 205.2 177.5 145.7 290.4 276.4 215.0 286.5 193.8 176.1 58.0 80.6	9.4 7.5 6.1 9.4 9.4 9.4 9.4 9.4 9.3	.0 .0 .0 1372.9 .0 .0 .0 .0	045.19 348.60 351.96 955.40 355.40 355.30 351.40 340.18 348.73 348.07 347.07	337.30 339.60 348.00 358.40 353.60 351.60 347.30 347.30 347.30 347.30	353.40 354.20 355.40 355.40 354.60 353.75 352.55 351.30 350.05 349.40 346.45
1950 06-0 1950-09-0 1950-09-0 1950 00-0 1950 10-0 1950 11-0 1950 12-0 1951 01-0 1951 02-0 1951 03-0 1951 04-0 1951 05-0	826.5 563.9 1781.1 2061.2 2072.7 1779.5 1501.3 1284.6 1064.1 612.4 540.4	4,9 1434.5 474.9 643.9 22.2 18.5 16.0 17.3 7.4 4.9 3.7	22.9 12.0 17.2 24.2 23.0 20.3 17.7 15.5 14.5 22.5	243.8 115.4 109.9 278.2 298.4 214.0 285.5 107.2 57.0 79.6	.0	1.0 89.8 67.6 67.6 1.0 1.0 1.0 1.0 1.0	,0 ,0 ,0 ,0 ,0 ,0 ,0 ,0 ,0 ,0 ,0 ,0 ,0 ,	244.7 265.2 177.5 345.7 293.4 275.4 215.0 286.5 173.8 176.1 56.6 80.6	9.4 8.9 9.4 9.4 9.4 9.4 9.4 9.4 9.4	.0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0	344.18 353.88 355.29 355.40 353.86 352.24 350.79 348.62 344.79 343.86 342.40	337.00 339.60 348.00 352.40 353.00 351.60 347.60 347.60 342.20 339.60 330.60	353.40 354.90 355.40 355.40 354.60 354.60 354.60 354.60 354.60 354.60 354.60
179.1 (18. 0) 1951 (07. 0) 1951 (08. 0) 1951 (08. 0) 1961 (19. 0) 1962 (11. 0) 1962 (11. 0) 1962 (03. 0) 1962 (04. 0) 1952 (04. 0) 1952 (05. 0)	400.8 262.3 286.1 113a.8 13e1.0 (215.5 1e65.5 607.7 612.6 427.1 24e.1 219.2	62.0 213.4 1034.6 514.6 04.0 14.3 7.4 8.0 11.1 4.7 2.5	15.4 5.1 8.3 16.3 16.3 15.0 13.4 11.8 9.7 12.3 13.3	243.8 115.4 101.5 274.2 253.4 275.4 214.0 285.5 130.9 177.2 57.0 76.6	.0 .0 .0 .0 .0 .0 .0 .0	1.0 82.8 67.6 67.6 1.0 1.0 1.0 1.0 1.0	. 6 . 6 . 8 . 9 . 9 . 0 . 0 . 0 . 0 . 0	244.7 184.57 177.5 276.21 224.81 207.51 161.57 215.11 193.8 195.04 58.0 66.71	9.4 4.6 5.0 9.4 9.6 0.4 8.5 3.4 6.9 2.7 2.5	.0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .	339.92 339.40 349.71 351.31 350.27 348.66 347.18 344.47 342.20 339.60 337.89	337,00 339,60 348,00 352,40 353,00 351,60 347,60 347,60 342,20 339,60 335,90	353,40 354,30 355,40 355,40 354,60 354,60 354,60 351,75 351,70 351,70 349,40 348,45
1952 0 - 0 1952 0 4 - 0 1952 0 4 - 0 1952 0 6 - 0 1952 10 0 1952 11 0 1952 12 0 1953 01 0 1953 03 0 1953 04 0	147.7 167.6 362.5 561.2 1441.4 1218.0 1002.5 894.1 612.3 627.0 186.1 217.3	58.0 756.4 1647.9 155.4 16.5 7.4 6.2 4.0 4.9 3.7 0.5	9,5 6,1 11.9 19,1 17,1 15.0 13.4 11.7 9,6 0,7 11.5	263.4 115.4 108.9 278.2 298.4 275.4 216.0 285.5 192.9 177.2 57.0 77.6	. 9 . 0 . 0 . 0 . 0 . 0 . 0 . 0 . 0	1.0 89.8 67.6 1.0 1.0 1.0 1.0 1.0 1.0	.0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0 .0	38.70 205.2 177.5 276.2 224.8 207.5 161.5 181.5 180.6 194.8 60.7	1.1 5.9 5.4 9.4 9.4 9.4 9.5 9.4 9.5 9.4 9.5	. 0	335.80 345.53 352.73 351.85 350.31 348.84 347.15 344.78 342.20 339.60 337.89	337,00 339,60 348,00 352,40 353,00 351,60 349,33 947,60 539,60 539,60 536,60 335,90	353.40 354.90 355.40 355.40 355.40 354.60 359.75 351.30 351.30 350.05
1955 06 0 1 1955 07 0 1 195 08 08 0 1953 18 0 1953 18 0 1953 12 0 1954 12 0 1954 03 0 1954 04 6 1954 05 0	147.7 193.4 401.7 2070.7 2024.5 1782.6 1580.7 1277.3 484.8 745.2 526.4	80.2 14.8 9.3 8.6 7.4 3.7 2.5	7.4 5.3 12.7 24.8 22.7 20.4 17.7 15.2 13.1 14.4 17.1 22.3	243.8 115.4 108.9 278.2 298.4 275.4 214.0 285.5 192.9 177.2 57.0 79.6	.0	1.0 89.8 07.6 1.0 1.0 1.0 1.0 1.0 1.0	.0 .0 .0 .0 .0 .0 .0 .0	1.00 205.2 177.5 345.7 276.4 215.0 286.5 193.8 178.1 58.0 80.6	5.0 5.0 9.4 9.4 9.4 9.4 9.4 9.4 9.4 9.4	.0	335,64 343,17 355,40 355,08 353,89 352,24 350,74 346,66 346,66 343,62 342,10	353.00 351.60 349.10 347.60 342.20 339.60 336.60	353.40 354.90 355.40 355.40 355.40 353.75 352.55 351.30 350.65 349.40
1954 06-0 1-04 07-0 1-04 08-0 1-054-09-0 1954-19-0 1954-12-0 1954-12-0 1955-01-d 1955-03-0 1956-04-0 1955-05-0	428.7 189.5 511.5 928.2 1526.2 1413.6 1217.5 185.6 837.7 619.7 452.7 342.3	18.5 532.9 588.4 530.6 130.7 28.4 14.8 11.1 7.4 3.7 2.5	.4.0 5.7 8.7 16.4 18.6 15.9 15.2 13.7 11.9 12.5 14.9	243, a 115, 4 119, 9 278, 2 298, 6 275, 4 214, a 285, 5 192, 9 177, 2 57, 0 79, 6	.0 .0 .0 .0 .0 .0 .0 .0 .0 .0	1.0 d9.6 67.6 67.6 1.0 1.0 1.0 1.0 1.0 1.0	.0	235.71 205.2 163.01 276.21 224.81 207.51 161.51 215.11 193.8 178.1 50.0 80.6	9.4 5.8 9.4 9.4 9.4 9.4 9.4 9.4 9.4	0.0 0.0 0.0 0.0 0.0 0.0	337.00 343.46 348.00 352.39 351.36 350.31 349.08 347.10 345.10 345.61	357.00 339.60 3348.00 352.40 351.50 349.10 347.60 347.60 337.60 337.60	353.40 354.90 355.40 355.40 355.40 355.75 352.55 351.30 358.80 358.80 358.80

YYYY · Mrt · D	Inleto	Loc_Flo m m3	Evapr	Tir_Dem	iw_Dem M kwh	Wa Dem	Tols_Dem m m3	Keleas in in3	Pw_Gen M_kwh	Spili #. #3	Znd_i.ev m	Mdl_Rul m	Upr_Rul
1955 - 06 - 11	285.5	102.4	12.4	243.8	.0	1.0	.0	105.91	8.5	. 0	337.00	337.00	353.40
1955 - 07 - 0 1955 - 68 - 0	189.5 157.5	125.8	3.9	115.4	.0	89.8	.0	154.01	2.6	.0	335.60	339.60	354,90
1955-09-0	1097.4	1300.1	19.4	278.2	.0	67.6 67.6	.0	349.7	9.4	. 0	355.12	348.00 352.40	355.40 355.40
1955-10-0	2032.5	869.6 41.9	24.0	238.4	.0	1.0	.0	299.4	9.4	499.0	355.40	353.00	355.40 354.60
1955-12-0	1822	24.7	21.1	214.0	.0	1.0	.0	215.0	9.4	. 0	352.91	349.10	353.75
1956-01-0 1956-02-0	1610.7 1320.5	14.8	18.5	285.5 192.9	.0	1.0	, D , D	286.5 193.8	9.4	.0	351.83 349.56	347.60	352.5° 351.30
1956-03-0	1118.1	6.2	18.0	177.2	. 0	1.0	. 0	178.1	9.4	, D	349.00	339.60	350.05
1956-04-0 1956:05 0	928.1 849.4	2.5	23.2	57.0 79.6	.0	1.0	.0	58.0 80,6	3.8 5.1	. 0	347.30 346.22	336.60 335.90	349,40 348,45
1956-06-0	740.3		21.9	243.6	.0	1.0		244.7	9.4		343.84	337.00	353.40
1956 07-0	539,0	601.6	9.4	115.4	. 0	89.8	.0	205.2	7.4	. 0	349.62	339.60	354.90
1956-08-0 1956-09-0	1126.0 1526.4	590.8 421.9	12.9	109.9 278.2	.0	67.6	. 0	177.5 345.7	6.2 9.4	. 0	352.39 352.74	348.00	355.40 355.40
1956-10-0	1583.6	94.0	18.9	298.4	. 0	1.0	, ii	224.81	9.4	. 0	351.66	353,00	355,40
1956-11-0 1956-12-0	1413.4	150.5 27.1	17.6	275.4	.0	1.0	. C	207.51	9.4	.0	351.16 349.68	351.60 349.10	354.60 353.75
1957-01-0	1134.7	30.E	14.2	285.5	.0	1.0	. 0	269.01	9.4	.0	347.60	347.60	352.55
1957-02-0 1957-03-0	881.6 679.2	3.7 17.3	12.2	192.9 177.2	.0	.9 1,0	. D	193.8 178.1	9.4	.0	345.56 343.37	342.20 339.60	351.30 350.05
1957-04-0 1957-05-0	505.2 444.6	13.6	16.2	57.0	.0	1.0	.0	58.0 80.6	3.2 4.2	.0	342.48 340.83	336.60 335.90	349.40
1957-06 0 1957-07-0	347.2 175.2	24.7 145.6	12.8	243.6 115.4	.0	1.0	.0	163.81	7.2 2.8	.0	335.49 335.80	337.00 337.60	353.40
1957-06-0 1957-09-0	157.5	1150.8	7.5	109.9	.0	67.6	.0	177.5	4.7	. 0	349.60	348.00	355.40
1957-10-0	1123.3	478.6	16.0	278.2	.0	67.6	.0	276.21	9.4	.0	350.96 349.45	352.40 353.00	355.40
1957-11-0 1957-12-0	1103.6	11.1	14.1	275.4	.0	1.0	. 0	207.51	9.4	. 0	347.70	351.60	354.60
1958-01-0	893.1 726.5	4.9	12.5	214.0 285.5	. 0 . 0	1.0	.0	161.51 215.10	8.2 9.4	. 0	343.38	349,10	353.75 352.55
1958 02-0 1958-03-0	505,6 355,1	3.7	a.∈ a.5	192.9	.0	1.0	. D	145.6F	7.9	. 0	340.98	342.20 339.60	351.30 350.05
1958-04-0	216.4	2.5	9.6	57.0	.0	1.0	. 6	43.71	1.8	. 0	336.12	336.60	349.40
1958-05-0	165.6	2.5	12.1	79.6	. 0	1.0	, G	8.70	. 3	. 0	335.40	335.30	348.45
1958 06-0 1958-07-0	147.3	25.9	9 - 5	243.8	.0	1.0	.0	6,2C	. 2	. 0	335.80	337.00	353.40
1958-07-0	157.S 369.3	421.9 1224.8	4.8 9.7	115.4	.0	89.8 67.6	.0	205.2 177.5	5.4	. 11	341.26 351.62	339,60 348.00	354.90 355.40
1958-09-0	1407.0 2079.7	1586.3 241.8	21.1	278.2	. 0 , a	67.6	.0	345.7	3.4	546.7	355.40 354.95	352.40	355.40 355.40
1958-11-0	1998.2	102.4	22.7	275.4	.0	1.0	.0	276.4	9.4	.0	353.98	351.60	354.60
1958-12-0 1959-01-0	1801.5 1592.7	27.1 8.6	20.9	214.0 285.5	.0	1.0	.0	215.0	9.4	0.0	352.80 350.87	349.10	353.75 352.55
1959-02-0	1296.6	6.2	15.a	192.9	. 3	. 9	, a	193.8	9.4	. a	349.37	342.20	351.30
1959-03-0 1959-04:Ć	1093.2 901.0	3.7	17.7	177.2	. 0	1.0	.0	178.1 58.0	9.4	. 0	347.77	339.60	350.05
1959-08-0	822.7	2.5	30.5	79.6	. 0	1.0	.0	80,6	5.1	. a	345.96	335.90	348.45
1959-06-0	714.1	72.8	21.5	243.8	. 0	1.0	.0	244.7	9.4	.0	343.59	337.00	353.40
1959-07-0 1959-08-0	520.6 1988.4	2249.9 1807.1	12.5	115.4	.0	89.8 67.6	, D	205.2	9.4	564.4	354.90 355.40	339.60 548.00	354.90 355.40
1959-09-0	2079.7	3220.6	24.3	278.2	. 0	67.6	. 0	345.7	9.4	2650.7	355.40	352.40	355.40
1959-10 0 1959-11-0	2079.7	398.4	24.3	298.4	. 0	1.0	.0	299.4	9.4	74.8	355.40 354.46	353.00	355.40
1959-12-0	1899.5	46.6	22.1	214.0	.0	1.6	.0	215.0	9.4	. 0	351.74	349.10	353.75
1960-02-0	1751.2 1544.2	99.9 24.7	20.5	265.5 192.9	. n . a	1.0	0.	286.5	9.4	. 0 . D	352.50	347.60	352.55 351.30
1960-03 0 1960-04-0	1356.7	21.0	20.7	177.2	. 0	1.0	.0	178.L	9.4	. 0	350.02 349,37	339.60	350.05
1960-05-0	1034.0	.0 ,r	36.2	79.6	.0	1.0	.0	50.0 80.6	4 . II 5 . 5	, a , 5	348.42	336.60	349.40
1960-06-0	977.2	59.2	26.2	243.8		1.0		244.7	9.4	. 0	346.47	337,80	353.40
1960 07-0	765,5	445.3	9.9	115.4	. 0	89.8	. U	205.2	7.6	. 0	340.58	339.60	354.90
1960-00-0	995.7 2079,7	1074.9 356.5	24.2	109.9 27a.2	. 0	67.6	, 0	177.5	6.6 9.4	599.0	355.40 355.32	348,00	355.40 355.40
1960-10-0	2066.3	275.1 60.4	23.3	298.4	. 0	1.0	. 0	293.4	9.4	.0	355.05	353.00	355.40
1960-12-0	1779.4	45.6	20,8	214.0	. 0	1.0	.0	276.4	9.4	0.0	353,87 352,78	351,60 349:10	354.60 353.75
1961-01-0 1961-02-0	1589.3 1304.2	19.7 9.9	16.3	265.5 192.9	. 0	1.0	. 0	286,5	9.4	.0	350.92 343.45	347.60	352.55 351.30
1961-03-0	1104.4	. 0	17.8	177.2	. 0	1.0	.0	178.1	9.4	. 0	347.83	339.60	350.05
1961-04-0 1961-05-0	208.5 827.6	.0	22.9	57.0 73.6	.0	1.0	. 0	58,0 30,6	3.8	0	347.0B	336.60	349.40 348.45
1961-06-0	716.5	18.5	20.9	243.8	α,	1.0		244.7	9.4			337,00	353.40
1961-07-0	469.3	1064.5	9.8	115.4	.0	89.8	. ш	205.2	7.5	, II . G	351.02	339.60	354.90 355.40
1961-08-0 1961-09-0	1318.7	2138.9 5442.1	15.5 24.3	109.9 276.2	.0	67.6	. 11	177.5	6.9 9.4	1184.9	355.40	348.00 352.40	355.40
1961-10-0	2079.7	540.3	24.3	298.4	- 0	1.0	, п	299.4	3.4	216.6	355.40	353.00	355.40
1961-11-0 1961-12-0	2079,7 1874.9	95.0 71.5	23.4	275.4	.0	1.0	.0	276.4	9.4	.0	354.34 353.52	351.60 349.10	354.e0
1962 01 D	1709.7	55.5	19.8	285.5	.0	1.0	.0	286.5	3.4	, a	351.97	347.60	352.55
1962-02-0 1962-03-0	1450.9	28.4 18.5	17.5	192.9	.0	1.0	.0	193.8	9.4	.0	350.73 349.39	342.20	351,30 350.05
1962-04-0 1962-05-0	1096.6	19.7	25.8 35.0	57.0 79.6	.0	1.0	.0	50.0 80.6	4.0	.0	348.90	336.60	340.4U
, 302-03-0	1432.5	7.7	٠.٠٠	17.6	.0	1.0	.0	54.6	3.4	.0	347.99	333.30	244.43

Y Y Y Y - M(1) - 23	Ini Sto	Loc_Flo	€Vapr n-n3	Tir_Dem n. m3	PW_Dem M_kwli	Wa Deτ.		Kolone		Spil:	End_larv	Yal Ruj	
1962-04-0	926.E	28.4	25.0	243.a	, г	fi fi3	in in/s	m m3	M kwii	en omla , (i)	345.63	m 337.00	a 353.40
1962-97-0 1962-04-0	685.4 993.6	523.0 1145.9	9.6	115.4	. 6	89.8 67.6	.0	205.2	7.5	.0	348.56 354.70	399.60 348.00	354,90
1962 0 10 0 1750 10:0	1948.0	2239.2	23.7 23.4	278.2	. n	67.6	.0	34577	9.4	1798.1	155.40	352.40	355.40
1 - 2 1 0 1 - 0 1 - 0	1687.6	12.3	21.3	275.4	. 0	1.0	.0	293.4	9.4	.0	352.26	353.00 351.60	355.48
3.763 - 03 0	1602.3 1415.2	46.9 9.9	19.0 16.6	214.0 285.5	.0	1.0	.0	215.0	9.4	.0	349,59	347.40	353.75
1963-02-0	1122.0 922.5	83.3	14.3	192.9	.0	1.0	.0	175.8	9.4	. 0	547.95 345.43	342.00	350.30
1963-04-0 1963-05-0	761.5 646.6	3.7	20.7	57.0 79.6	.11	1.0	.0	58.0	3.6	.0	345.64	336.80	349.40
1±63-06-0	582,3	27.1		243.8		1.0		241.7	9.4		340.82	337.00	353.40
1963 07-0	344.5 235.1	164.1	11.5	115.4	. 0	17.6		205.7	5.6	. 0	339.92	339.60	354.90
1963-09 0 1963-10-0	1984.5	1762.6	23.d	270.2	. 6	67.6	. 0	345.7	5. c 9. q	1297,9	154.8# 355.40	348,00 352,40	355.40 355.40
1963-11-0	1547.6	86.3	21.7	298.4 275.4	. ::	1.0	. u . 0	299.4 276.4	9.4 9.4	n, 0.	354.41	353.00	355.48
1963 12-0 1964-01-0	1675.9	81.4 62.9	20.0	214.0 285.5	. 11	1.0	.0	215.0	9,4	.0	352.37 350.76	349.10 347.60	353.75 352.55
1964-02 K	1281.0	51.8 37.0	18.2	172.9	. 31	1.0	.0	193.8 178.1	9.4	.0	349,50 348,31	342.20 339.60	351.30 350.05
1964 04-0 1964-05-0	963.8 905.4	23.4	23.9	57.0 79.6	. 0	1.0	.0	58.0	3.8	.0	347.80	336.60	349.40
1364:06 0				-,-		1.6		80,6	5.2	.0	346.92	335.90	348.45
1964 07-0	810.9 640.0	97.4 896.7	10.5	243.8 115.4	. 3	as.a	.0	244.7	9.4	. D	345.18 351.04	337.00 339.60	353.40 354.90
1964-06-0 1964-09-0	1321.0	3332.9 418.2	24.3	107. 278.2	.0	67.6	0.	177.5	9.4	2381.2	355.40 355.40	348,00	355.40
1964-10-0	2079.7	974.5 49.3	24.3	278.4 275.4	. 0	. 0	.0	299.4	9.4	650.8	355.40 354.12	353.00 351.60	355,40 354,60
1964-12-0	1829.5	44.4	21.3	214.0 285.5	. 0	1.0	. ŭ	215.0	9.4	. 0	353.08	349.10 347.60	353.75
1965-02-0 1965-03-0	1372.9	26.4	16.6	132.9	.0	. 9	. 0	193.8	9.4	.0	351.39 350.11	342.20	351.30
1965-04-0	1190.a 1022.2	28.4 25.9	16.8 24.8	177.2 57.0	. D	1.0	. s	178.1	9.4	. 0	348,81 348,32	336,60	350.05 349.40
1965-05-0	965.4	18.5	33.7	79.6		1.0	. 0	30.6	5.3	. 0	347.50	335.90	148.45
1965-06-0 1965-07-0	869.6 665.9	65.4 486.0	24.3	243.d 115.4	. 0	1.0	. 0	244.7	9.4 7.4	. o	345.40 348.08	337.00 339,68	353,40
1965-08-0 1965-09-0	937.4 1044.1	204.8 1425.9	10.6	109.9 27a.2	. 0	67.6 67.6	. 0	177.5 345.7	5.H 9.4	.0 25.2	348.99 355.40	348.00 352,40	355.40 355.40
1965-10-0	2079.7	91.3 38.2	23.3	298.4	. 0	1.0	. 0	299.4	9.4	,0	354.21	353.00	255.40
1965-12-0 1966-01-0	1589.1	29.6	18.8	275.4 214.0	.0	1.0	. 0	276.4 215.0	9.4	.0	352.78 351.47	351.60 349.10	354,60 353,75
1966-32 ∪	1384.9	17.3 9.9	16.3	285.5 172.9	. 0	1.0	. 0 , a	286.5	9.4	.0	349.41	347.60	352.55 351.30
1566-83-0	901.4 717.4	9.9	15.7	177.2 57.0	. 0	1.0	.0	178.1 58.0	3.4	.0	345.99 345.15	339.60	350.05 349.40
1966-05-0	644.3	3.7	26.4	79.6	. 31	1.6	.0	80.63	4.7		343.87	335.90	348.45
1966-06-6 1966-07-8	541.1 261.9	2.5 1662.7	16.9 10.6	244.8 115.4	. 0	0.1 89.8	. 0	244.7	9.4 7.4	.0	339,50 353.63	337.00 339.60	353.40 354.90
1966-08-0 1966-09-0	1728.8	1179.0 218.3	17.1	105.9 278.2	. 0	67.6	.0	177.5	7.1	627.5	355.40	348,80	355.48
1966 10-0 1965-11 0	1928.7	25.3	21.6	298.4	. 6	1,0	.0	299.4	9.4	.0	354.61 353.05	352.40 353.80	355.40 355.40
1969 12-6	1404.2	6.2	19.1 16.7	275.4 214.0	.0	1.0	.0	216.45 215.0	9.4	.0	351.60 350.02	351.60	354.60
1967-01-0 1967-02-0	1178.6 862.8	3.7	14.3	285.5	.0	1.0	.0	286.5 193.8	9.4	n.	347.61	347.60	352.55 351.30
19-7 03-0 19-7-04-0	494.1	4.9	13.1	177.2 57.0	.0	1.0	. 6	178.1 58.0	9.4		343.22 342.13	339.60 336.60	350.05 349.40
1987 05-0	422.8	2.5	19,9	79.6	.0	1.0	. 0	30.6	4.2	. 9	340.40	335.90	348.45
1967-06-0 1967-07-0	324.8	113.0	12.9	243.6	.0	1.0	. 6	213.41	9.4	. 0	337.00	337.00	353.40
1967-08-0	189.5 1325.3	1949.4 1010.2	8.4 15.5	109.1	.0	69.a	. 0	177.5	6.7	62.8	351.0a 355	339.60	355.40
1967-09 0 1967-10-0	2079.7	3046.0 74.0	24.3	27a.2 29a.4	. 0	67.6	. 11	345.7	9.4	676.0	355.40	352.40	355,40
1967-11-0	1831.1	4.9 53.0	20.8	275.4	. 0	1.0	. 0	276.4	9.4	. 11	350:47	351.60 349.10	354.60 353.75
1968-01-8 1968-02-0	1359.6 1074.6	18.5	16.1	285.5 192.0	0.0	1.0	. 11	236.5	9.4	. 0	349.23	347.68	352.55 351.30
1968-03-u	876.7	18.5	15.5	177.2	. 0	1.0	. 0	176.3	9.4	. 0	345.82	339.60	350.05
1968-04-0 1968-05-0	701.6 628.8	4.9 2.5	19.8 26.0	57.0 79.6	. 0	1.0	.0	50.0 20.6	3.5 4.7	.0	344.97 343.64	336.60	349.40
1968-06-0	524.7	4.9	16.5	243.8	. 0	1.0	.0	244.7	9.4	.0	339.17	337,::0	353.40
1968-07-0	268.4	964.6	7.9	115.4	.0	89.8 67.6	.0	205.2	6.7	.0	348.80	339.00	354,90 355,40
1968-09 0 1968-10-0	1794.5 1996.0	569.9 54.3	22.7	278.2 298.4	.0	67.6	.0	345.7	9.4	. 0	354.94	352.40	355.40
1968-11 0	1724.5	27.1	19.9	275.4	. 0	1.0	. 0	276.4	9.4	.0	353.62 351.97	353,00 351,60	355.40 354.60
1969-01-0	1244.4	17.3	17.3	214.0 285.5	. 0	1.0	.0	215.0	9.4	.0	350.51 348.26	349.10 347.60	353.75 352.55
1969-02-0 1969-03-0	959.0 763.3	11.1	12.9	192.9	.0	1.0	.0	193.H 170.1	9.4	.0	346.45	342.20	351.30 350.05
1969-04-0 1969-05-0	580.0 511.4	6.2	17.6	57.0 79.6	. 0	1.0	.0	58.0 80.6	9.3	.0	949.46	336,60 335,50	349.40 348.45
			200	10.00		4 - 11		9	7 . 7		200	222,29	200.30

YYYY-Mn-D	Ini_Sto	Loc_Flo	Evapr m m3	Tir_Dem m m3	Pw_Dem M kwh	Ws. Och n. m3	Trike_Denu m m3	Relear m m3	Pw_Cen M kwh	Soill n m3	End_Lev m	Mdl Rul Fi	Upr_Kul
1969-06-0	413.2	9.9	13.9	243.8	.0	1.0	. 0	219.61	9.4	.0	337.00	337.00	353.40
1969-07-0	189.5	869.6	6.9	115.4	.0	69.8	. 0	245.2	6.3	, 0	347.26	339.60	354.90
1969-06-0	847.0	1614.6	13.9	109.9	.0	67.6	.0	177.5	6.4	190.5	355.40	348.00	355,40
1769-09-0	2079.7	859.7	24.3	278.2	. 0	67.6	. 0	345.7	9,4	469.8	355,40	352.40	155.40
1969 10-0	2079.7	44.4	23.1	298.4	. 0	1.0	, 0	299.4	9.4	. 0	353.98		354.60
19-9-11 0	1801.6	18.5	20.5	275.4	. 0	1.0	.0	276.4	9.4	. 0	352.37	351.60 349.10	353.75
1569-12-0	1523.2	11.1	18.0	214.0	.0	1.0	. 0	215.0	9.4	. 6	350.90	349.10	
1120 01-0	1301.4	8.6	15.5	285.5	.0	1.0	. L:	286.5	9.4	. 0	348.69		
1970-82-0	1000.1	4.9	13.3	192.9	. 0	.9	.0	193.8	9.4	.0	346.87		
1979 03-0	805.9	12.3	14.6	177.2	. 0	3.0	.0	170.1	9.4	.0	344.93 344.05	336.60	
1970 04-0	625.4	4.9	18.4	57.0	. 0	1.0	. 0	58.0	3.4	. 0			
1970-05-0	553.9	3.7		79.6	. 0	1.0	. 0	an.6	4.5	٠, ۵	342.62	333.70	
				243.8	.0	1,0	.0	244.7	9.4	. 0	339.01	337.00	
1970-06-0	453.1	69.1			.0		.0	205.2	6,2	. 0	344.66	339,60	
1970-07-0	261.9	551.4	6.5		.0			177.5	5.9	.0	354.17		
1970 - 08-0	601.5	1427.1						345.7	9.4	1693.1	355.40		
1974-19-0	1838.8	2302.9			,0			299.4	9.4	. 0	353.98		
1976-10-0	2079.7	43.2						276.4	9.4	. 11	352.35		
1970 - 11 - 0	.1800.4	16.0						215.0	3.4	.0	350.88	349.10	
1970-12-0	1519.6	11.1	17.9					286.5	9.4	.0	348.64		
1971 01-0	1297.7	7.4						193.8	9.4	. 0			
1971-02-0	1003.3	4.9						178.1	9.4	. 0	344.78		
197)-03-0	801.1	3.7						58.0	3.4	. 1	343.63		
1971-04-0	612.1	2.5						80.6	4.5	. 0	342.34	335.90	348.45
1971-05-0	538.4	1.2	23.4	12.6									
1971 - 06-0	435.7	88.8	15.4	243.6		1.0			9.4	, (
1971-07-0	264.3					69.8		205.2	6.9	. 0			
1971-07-0	1224.9				, ,	67.6		177.5	6.1				
1971-09-0	1245.0					67.6			9.4	432.6			
1971-10-0	2079.7					1.0	. 0	299.4	9.4		354.35	353.00	, 335.40

Number of Failures for WS. = 0, Time Reliability = 1.00

Number of Failures for IRR. = 45, Time Reliability = .840

Number of Critical Failures = 4 (Release < .75 * Total Demand)

Volume Reliability for IRR. & WS. = .95

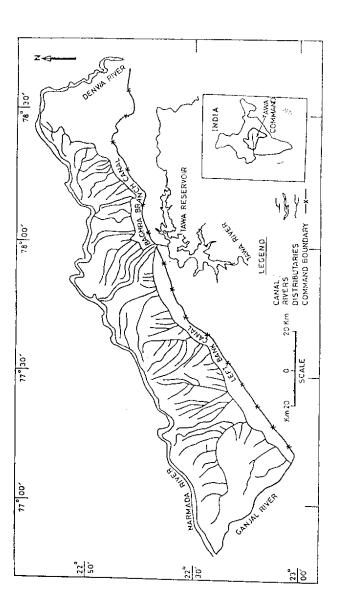
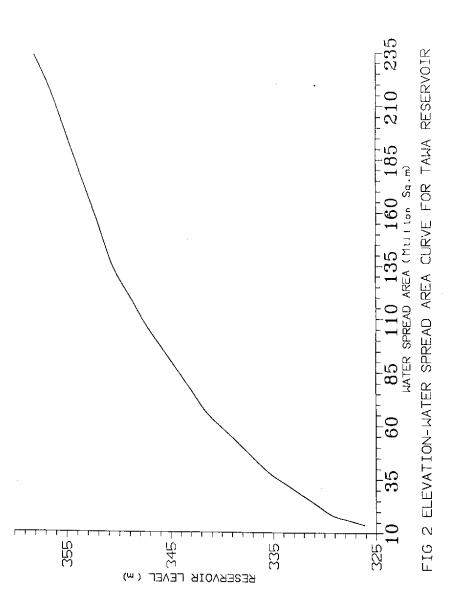
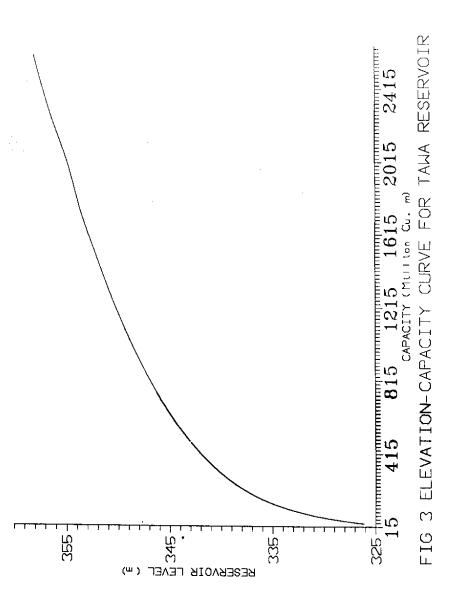


FIG 1 MAP OF TAWA BASIN SHOWING RESERVOIR LOCATION AND MAIN CANAL SYSTEMS

STUDY GROUP





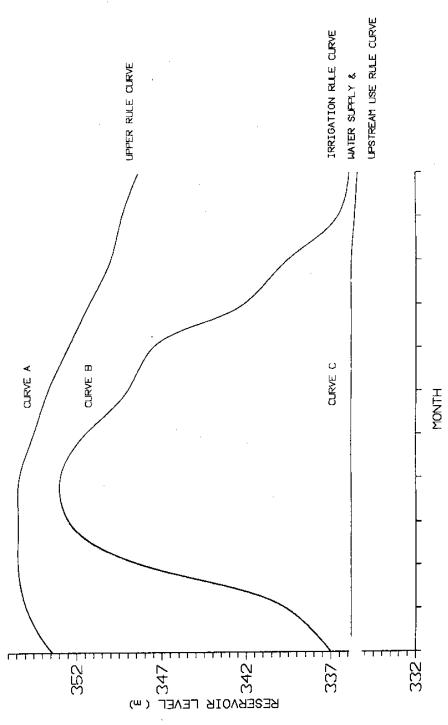


FIG 4 RECOMMENDED RULE CURVES FOR TAMA RESERVOIR

÷,

STUDY GROUP

Director

Dr S M Seth

Scientists

Sh A R Senthil Kumar

Dr S K Jain

Sh' M K Goel

Scientific Staff

Sh R K Neema