$$
D P-2
$$

# RATING CURVE ANALYSIS 

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List of Figures
Abstract $\qquad$ ii
1.0 INTRODUCTION

1
2.0 PURPOSE OF THE PROGRAMME .................... 2
3.0 METHOD USED . . . . . . . . . . . . . . . . . . . . . . . . . . . . 3
4.0 COMPUTER PROGRAMME . . . . . . . . . . . . . . . . . . . . . . 7
5.0 INPUT SPECIFICATION ................................ 9
6.0 EXAMPLE $\operatorname{DALCULATION~...........................~} 11$
7.0 APPLICATION OF THE PROGRAMME .............. 21
8.0 RECOMMENDATIONS . . . . . . . . . . . . . . . . . . . . . . . . 23

REFERENCES . . . . . . . . . . . . . . . . . . . . . . . . . . . . . 24
APPENDICES

LIST OF FIGURES

Figure Number

1
CROSS SECTION OF THE RIVER
NARMADA AT MANDALESHWAR

2

$$
\text { STAGE } \because 9 I S C H A R G E \text { PLOT }
$$

3
DOUBLE LOG PLOT OF STAGE-
DISCHARGE
20

## ABSTRACT

Many hydrologic analyses are made using discharge data of a river at a site. Often these discharge values are obtained from stage measurements. In order to compute the discharge from the stages a rating curve is used. A rating curve is a steady-state relation between the complex inter-relations of stages and discharges. When rating curves are available one can go in for interpolation methods. Some time physically sound rating curves may have to be established before computation of discharge is taken up. Two computer programmes for this purpose are given in this documentation.

In the case of use of available rating curves, a rating table is prepared. This table consists of stage and discharge values. After removal of data error in the table by finding first and second differences of the discharge values this table can be used as interpolating table. The Hermition method, which provides a smooth curve taking continuity of slope and curvature is used. A FORTRAN subroutine capable of interpolating discharge values for given stage values has been developed and documented.

For the establishment of rating curve from the measured discharge and the corresponding water level, after data processing, the available data is plotted on a simple graph and also on double logarithmic paper. This helps in grouping the data and to find its suitability for fitting simple curve. The grouped data is used to fit a relationship of the following form $Q=a(S-e)^{b}$ where, $Q$ is the discharge, $S$ is the stage and ' $a^{\prime}$, ' $b^{\prime}$, ' $e$ ' are parameters defining the relation. The well known method fitting a curve is to minimise the
sum of squares of differences between observed and computed $Q$ using least squares criterion. Assuming a value of parameter ' $e$ ' and taking logarithms of $Q$ and ( $s-e$ ) the parameters ' $a$ ' and ' $b$ ' are found. For different values of ' $e$ ' the method is repeated and various sets of parameters ' $a$ ' and ' $b$ ' along with the sum of squares of departures are found. The set having minimum of above departure can be taken to define the relation. But a physical interpretation of the results may suggest different set of parameters.

The stage and discharge data is given as input to the programme and it gives values of parameters' $a$ ' and ' $b$ ' using least squares method Necessary flow charts, input specifications and sample output are given in the documentation. Interpretation of the physical information regarding the cross section for arriving at appropriate values of parameters ' $a$ ' and ' $b$ ' of the rating curves are also provided.
Many hydrologic analyses are made using discharge data of a
river at a site of interest. These data can be obtained by direct
measurements. In case of direct measurements the data can be acquired
at the expense of lot of labour and money. Sometime the observations
can not be made in the time interval in which they are required. Hence
alternative ways are deviced to obtain the necessary information.
One of the way is to go in for measurement of water level at the
required time interval which is easier and to compute the discharges
from them. The computations are achieved through the use of rating
curve which depicts an approximate relationship between the discharge
and the stage.

While computing discharges at hourly interval from stage measurements at that interval for Narmada flood studies three different cases were categorised as follows:

1. Use of interpolation technique along with available(supplied by (the concerned field organisations) rating curves.
2. Extension of rating curve using conveyance analysis.
3. Establishment of rating curve using observed stage and corresponding discharge values together with a physical analysis. Computer programs have been developed for dealing with the case 1 and case 3 and they are described in this documentation.

### 2.0 PURPOSE OF THE PROGRAM

2.1 Case 1 Interpolation

The discharge computation for a given stage is achieved by computing the particular intermediate value given by the rating table with little extrapolation if necessary. The rating table is a representation of the rating curve in a discrete form. The interpolation employed should provide smooth values(that is if plotted should form a continuous smooth curve). With this object an interpolation technique of Hermitian type(3) was used and a computer program was developed.
2.2 Case 3 Establishment of Rating Curve

The observed discharges and corresponding water stages provide the valuable information about the relationship between them. This relationship is represented in mathematical form for easy use in models. This functional representation can be developed using least squares techniques. The computer program developed for this purpose uses number of pairs of stage and discharge measurements of a particular site as input data and provide the mathematical form of the rating curve.

### 3.0 METHOD USED

```
3.1 Interpolation
    Preparation of rating table:
    Direct values of stage and corresponding discharge are read
```

out from the rating curve at an equal interval of stage values. These
values form the raw rating table. Smoothening of table is undertaken
to remove off the possible error that would have entered in while reading
the rating curve causing a deviation in the monotonic character of
the relation. This is done by preparing a difference table of discharge
values and adjustments are made by adding or subtracting certain small
quantity to /from the discharge values so that the difference in disch-
arge increases steadily with increase in stage. (There may be cases
when the second difference is constant for one or two continuous inter-
vals.This is acceptable).
Hermition Interpolation
In order to have a smooth continuous function the interpolaotion
used here takes not only the rate of change of discharge with respect
to stage(first difference) but also the rate of change of this change
(second difference) into account. This type of interpolation is known
as Hermitian (3)

```
3.2 Establishment of Rating Curve
    The observed discharge and corresponding stage values are basic
data used. Before using computer program a sample graphical analysis
```


### 3.2.1 Simple graphical analysis

The stage and discharge data is plotted on a simple graph sheet. This is done with two objectives(1) to identify obviously erronous data pairs which plot much away from the general trend of the scattered points,(2) to diagnose whether a single curve would be alright or to go in for more number of curves. If it is diagnosed that two or more curves are needed, the data are approximately grouped so that each group will form a single curve. In the data seperated for a single curve the identified obvious errors are removed. Moreover the same set of observations should be repeated in the data used for computer analysis.

### 3.2.2 Lease squares technique The following form of the equation is used <br> $$
\begin{equation*} Q=a(G-e)^{b} \tag{1} \end{equation*}
$$

where
$G$ is the gauge height in ( $m$ ) with respect to a datum
$Q$ is the discharge in $\left(\mathrm{m}^{3} / \mathrm{sec}\right)$
$a, b$ and $e$ are parameters defining the relationship.
An initial value of ' $e$ ' approximately equal to the zero of gauge of the site under consideration is assumed, thus reducing number of unknowns to two ('a','b') parameters.

The currently approved analytical method of fitting curves to scattered points is to minimise(R)the sum of square of departures $(D Q)_{i}=\left(Q_{i}-Q_{C}\right)$, where $Q_{i}$ refers to the scattered data and $Q c$ refers to the calculated value by equation 1. This equation can be written
in the form

$$
\begin{equation*}
\log Q=\log a+b \log (G-e) \tag{2}
\end{equation*}
$$

which is equivalent to

$$
\begin{equation*}
\bar{\chi}_{i}=\bar{a}+b \bar{G}_{i} \tag{3}
\end{equation*}
$$

Let there be $N$ pairs of stage and discharge and the logarithmic values of ( $G-e$ ) of each of them represented by $\bar{G}_{i}$ and $Q_{i}$ by $\bar{Q}_{i}$. In order that the curve given by 1 or 3 may have minimum of $\Sigma(D Q)_{i}^{2}$, all partial derivaties of this sum with respect to the parameters $\bar{a}, b$.

$$
\begin{align*}
& R=\sum_{i=1}^{N}\left(D Q_{i}\right)^{2}  \tag{4a}\\
& \partial R / \partial \quad \bar{a}=0 \\
& \partial R / \partial b=0 \tag{4c}
\end{align*}
$$

The equations $4 \mathrm{a}, \mathrm{b}$ and c lead to the following

$$
\begin{gather*}
N \bar{a}^{N}+b_{i=1}^{N} \bar{G}_{i}=\sum_{i=1}^{N} \bar{Q}_{i}  \tag{5a}\\
\bar{a} \sum_{i=1}^{N} \bar{G}_{i}+b \sum_{i=1}^{N}\left(\bar{G}_{i}\right)^{2}=\sum_{i=1}^{N} \bar{Q}_{i} \bar{G}_{i} \tag{5b}
\end{gather*}
$$

These two equations $5 a$ and $5 b$ form the simultaneous equations to solve for $\bar{a}$ and $b$, and $\bar{a}$ can be converted to $a$ by antilogarithms.

For different values of ' $e$ ' different sets of parameters $a, b$ are obtained. The value of ' $e$ ' are varied by desired increments. For each of the value ' $e$ ' different sets of parameters are computed along with their respective sum of squares of departures (i.e.,function R). After a number of trials the set of parameters which produce the minimum value of $R$ can be chosen as the parameters providing best fit.

The computer program (LSF) does these computations and tabulates all the sets of values of parameters obtained in trials along with corresponding value of function R. The program also calculates Error and Shifts as given by $\quad$ Error $=\left[\frac{Q_{C}-Q_{C}}{Q_{i}}\right] \times 100$

Shift $=S_{i}$ - (Stage calculated for the $Q_{i}$ from fitted relation) These are tabulated in the order of magnitude of discharge and in the sequence of observations. If these values are random, they indicate no systematic error is present in grouping of the data. In case of any systematic error a different grouping of data is to be done and different rating curve equations are obtained for each.

### 3.2.3 Physical reasoning

From among the tabulated parameters and corresponding values of $R$ the best curve may not be the one chosen by computer based on least value of $R$ from the point of view of physically realistic values of parameters. One may have to go in for the second best or third best depending upon the parameter values being physically realistic as explained in section 5.2 .

This choice is done by judgement using computer results and range of parameter values obtained by physical analysis.
4.1 Interpolation INTPOL

The computer program INTPOL is capable of interpolating from a given equal interval table. It picks up the interval in which the specified node lines. However, it sets the interval not to be lesser than the second and not to be greater than the last but one interval. In other words it finds the value of $J$ which will never be less than 2 or greater than $N-1$ where $N$ is the length of the table, since the interpolation method uses second difference also. If $X$ is the specified node and $S(J)$ and $Q(J)$ are the table values, then the ordinate to be computed Y , will be

$$
\begin{aligned}
Y= & Q(J)+\frac{(X-S(J))}{2 H}[Q(J+1)-Q(j-1)] \\
& +(X-S(J))^{2}\left[\frac{Q(j+1)-2 Q(j)+Q(j-1)}{2 H^{2}}\right]
\end{aligned}
$$

where $H$ is the equal interval of the table.

### 4.1 Least Squares Fit (LSF)

The computer programme LSF is capable of giving a rating analysis. The program takes daily gauge eand discharge values together with daces. The program needs an initial value of ' $e$ ' as specified in the input. It accepts a maximum of 30 iterations. Applying least squares technique it finds the two parameters of equation 1 for each of the initerations and stores them. The program prints the parameters and the corresponding sum of squared errors and picks up those parameters for which the error
is minimum and prints them. The table of stage,discharge observed and also as calculated using the above picked parameters are printed. The 'Shifts' and the Error as explained are also calculated and printed.

### 5.0 INPUT SPECIFICATION

5.1 INTPOL ( Program for interpolation)

This is a subroutine requiring a table of stage and discharge (rating table). Also required is the value of stage which is to be converted to discharge. The call of this subroutine provides back the values of stage, discharge and also the original rating table to the calling program.
5.2 LSF ( Program for least square fit)

| SNo. | Name | Description | Format |
| :--- | :--- | :--- | :--- |
| 1 | SNA(I) | Station name | 30 A1 |
| 2 | IWRIT | Index '1' cause <br> to write all the <br> tables as per text <br> Other values cause <br> to print only <br> coefficients. | Free format |
| 3 | HD(1) <br> DHD <br> $N$ N | initial guess of'e <br> increment of trial <br> Number of trials <br> Number of pairs in <br>  <br> discharge. | Free format |
| 4 | H,Q,DATE | Stage, discharge and <br> date respectively <br> N sets of values. | Free format |

### 5.3 Output Specification

### 5.3.1 INTPOL (Program for interpolation)

The output of this subroutine goes into the calling program. The computed discharge, the stage and the rating table are sent back to the calling program.
5.3.2 LSF (Program for least square fit)

1. The name of the site is printed as read by the program.
2. A table of trials and error.
3. A table of coefficients found in each one of the trials.
4. Best set of parameter as per Mathematical Criteria.

If IWRIT is equal to 1 the following are also printed.
5. A table of stage, observed and computed discharge.
6. A table of stage, discharge, Error, shift in its time sequence (as given).
7. A table of stage, discharge, Error and shift in the order of magnitude of discharge.

### 6.0 EXAMPLE CALCULATION

6.1 INTPOL

SITE : GADARWARA

RIVER : SHAKKAR

| STAGE (m) | Q $\left(\mathrm{m}^{3} / \mathrm{s}\right)$ | Q | ${ }^{2} \mathrm{Q}$ |
| :---: | :---: | :---: | :---: |
| 1 | 2 | 3 | 4 |
| 323.0 | 10 |  |  |
|  |  | 40 |  |
| 323.5 | 50 |  | 30 |
|  |  | 70 |  |
| 324.0 | 120 |  | 35 |
|  |  | 105 |  |
| 324.5 | 225 |  | 35 |
|  |  | 140 |  |
| 325.0 | 365 |  | 40 |
|  |  | 180 |  |
| 325.5 | 545 |  | 45 |
|  |  | 225 |  |
| 326.0 | 770 |  | 55 |
|  |  | 280 |  |
| 326.5 | 1050 |  | 55 |
|  |  | 335 |  |
| 327.0 | 1385 |  | 60 |
|  |  | 395 |  |
| 327.5 | 1780 |  | 85 |
|  |  | 480 |  |
| 328.0 | 2260 |  |  |


|  |  |
| :---: | :---: |
| SITE : GADARWARA |  |
| RIVER : SHAKKAR |  |
| YEAR : 1979 |  |
| S. No. | Stage |
| 1. | 323.0 |
| 2. | 323.5 |
| 3. | 324.0 |
| 4. | 324.5 |
| 5. | 325.0 |
| 6. | 325.5 |
| 7. | 326.0 |
| 8. | 326.5 |
| 9. | 327.0 |
| 10. | 327.5 |
| 11. | 328.0 |

6.2 Example calculation (for Program LSF)

Physical Analysis and Related Calculations
The rating analysis should be verified by physical analysis.
Each item of equation(1) can be explained as follows:

Manning's equation as a basis:

The physical analysis is based on Manning equation.

$$
\begin{equation*}
Q=\frac{1}{n} A R^{2 / 3} S^{1 / 2} \tag{2}
\end{equation*}
$$

where,

Q is discharge in cumecs
n is Manning's coefficient
A is wetted cross sectional area in sq.m.
R is hydraulic radius.
$S$ is engery slope.
A rating that follows manning's equation will plot as a straight
line on a logarithmic paper since the equation(2) is of the form $Q=a H^{b}$

Taking logarithms on both the sides
$\log Q=\log a+b \log H$
where,
$H=G-e$

G is the gauge height
e is the effective zero flow
If $R$ is approximately taken equal to $H$ the equation 3 turns
out to be

$$
\begin{aligned}
& \log Q=\log \frac{1}{n} A R^{2 / 3} S^{1 / 2} \\
&=\log \frac{1}{n}{A H^{2 / 3} S^{1 / 2}} \\
&=\log \frac{1}{n} W H H^{2 / 3} S^{1 / 2} \\
& \equiv \log \frac{1}{n} \mathrm{WS}^{1 / 2}+\log H^{5 / 3} \\
&=\log a+b \log H
\end{aligned}
$$

The parameter 'a'

The parameter a is approximated by

$$
a=\frac{1}{n} w s^{1 / 2}
$$

The Manning's ' $n$ '
This is estimated from photographs/site visits, or by experience. Generally $n$ will vary with depth for unsteady flow. But for rating analysis a single value is adopted. A wide range of values are expected. The value varies from 0.017 for alluvial stream with no vegetation to 0.05 for mountain stream with rocky beds with vegetation on the bank.

The width ' $W$ '
The width varies with stage. However, this is determined at approximately mean annual flood. This is surface width.

The slope 'S.'
This is computed from topographic map. The bottom slope is often irregular. The slope used here is effective slope sustaining the flow. One easy measure could be water surface slope at low flow or peak flow.

The parameter ' $b$ '
This is often approximated to5/3or slightly more. Actually this parameter depends on the side slope of the channel. The hydraulic radius' $R$ ' seldom equals the depth 'H'. For trapezoidal channel the increase in area as a result of increase in width as stage rises, causes
the discharge ' $Q$ ' to increase rapidly than for a rectangular section. In this case ' $b$ ' varies from 1.67 to 1.9. For parabolic channels ' $b$ ' is approximated to 2. For a triangular channel ' $b$ ' equals 2.5. Most of the natural channel sections vary between trapezoidal to parabolic in shape.

The parameter ' $e$ '
This is a level by which the elevations(water level) are to be reduced. This represents an effective zero flow. Approximately this is equal to zero of gauge of the site. Estimation can be made by

$$
e=G-A /(W+2 A / W)
$$

for a mean annual flow.
Log Plots:
As the next analysis, plotting of daily stage discharge pairs are taken up.

A smooth curve is drawn by visual estimation on rectangular coordinate system after plotting $Q$ and $H$. Three values of discharges $Q_{1}, Q_{2}$ and $Q_{3}$ are selected from the smooth curve drawn, such that $Q_{2}^{2}=Q_{1} Q_{3}$ If the corresponding values of the stages are $H_{1}, H_{2}$ and $H_{3}$ the equation (1) will yield

$$
\mathrm{e}=\frac{\mathrm{H}_{1} \mathrm{H}_{3}-\mathrm{H}_{2}^{2}}{\mathrm{H}_{1}+\mathrm{H}_{3}-2 \mathrm{H}_{2}}
$$

Now plotting on Log-Log scale is taken up. The gauge height is shifted by a depth 'e'. Now the shifts in ' $e$ ' on Log Log scale are in easy control. Choose a series of measurements from a peak as high as possible downward on a recession. Plot measurements sequentially as long as they form a smooth curve. Stop when a group of measurements begins to depart consistently from the curve. Draw in the trend line with a curve. By trial and error determine the change in shift required
to obtain a straight line rating $(\Delta$ e). Lower discharge measurements may not fit the curve if a section control exists for lower stages, similarly higher discharge may not fit if over bank flow exists or if a back water condition is caused by a downstream constriction. A single rating can be adopted over that range of flow where similar section control do exist. The coefficients of the equation are determined as follows:
1.The gauge height required to straighten the curve determines 'e'
2. The discharge for the gauge height ( $G-e=1$ ) is equal to 'a'. 3. $\mathrm{b}=\log Q_{1}-\log Q_{2}$

$$
\log \left(G_{1}-e\right)-\log \left(G_{2}-e\right)
$$

where $Q_{1}$ is greater $Q_{2}$
6.3 Application of physical analysis

Site: Mandeleshwar of the river Narmada
Width $(W)=600 \mathrm{M}-650 \mathrm{M}$ (Figure 1 \& 2)
Slope (S) $=0.00046$
$s^{1 / 2}=0.0215$
$s^{1 / 2}=$ appraximately
$a=\frac{W S}{n}=400$
a is in the range of 400 to 450
Effective zero flow - 138.7 to 139.0
$b-1.7$ to 1.8
Rating analysis:
A plot in rectangular coordinates is enclosed. The point marked - are removed from the analysis and rest of the daily gauge \& discharge were fed to LSF.FOR The results are enclosed.

FIGURE 1- CROSS SECTION OF THE RIVER NARMADA AT MANDALESHWAR

FIGURE 2 - STAGE-DISCHARGE PLOT

$$
\begin{aligned}
\mathrm{b}= & \frac{\log Q_{1}-\log Q_{2}}{\log \left(G_{1}-\mathrm{e}\right)-\log \left(\mathrm{G}_{2}-\mathrm{e}\right)} \\
& \text { (From Figure } 3) \\
\mathrm{G}_{1}= & 148.0 \quad G_{1}=16,000 \\
\mathrm{G}_{2}= & 141.2 \quad G_{2}=1300 \\
\mathrm{~b} & =\frac{\log 16000-\log 1900}{\log (148-138.8)-\log (141.2-138.8)} \\
\mathrm{b} & =\frac{4.20412-3.11394}{.963787-0.3} \\
\mathrm{a}= & 400 \\
\mathrm{e}= & 139
\end{aligned}
$$



### 7.0 APPLICATION OF THE PROGRAMME

### 7.1 Sample Input (LSF)



0
138.5 0.1 in 18
$1.22 \% 2890 . \quad 29.657$
342.24 2480. 13.0775
$112.16 \quad 2592, \quad 17.0775$
145.41 4922. $19+075$
142.58 2848, 200075
342.28 2522: 220375
143.59 273\}. 31.6735
$542.342603 . \quad 1,6875$
142.08 227ム, 7.0895
$143.774150 .36,485$
147.5:1250. $17 \times 125$
143.20 16406, 14.68 B

$145,838904,10,385$
$\{43.75 \quad 5559,120.275$
144.94 714... $6.68 \%$



### 7.2 Sample Output




| DATMM | SIM W BMAME 35 Enane | merficht 3u cosergrout | 等 |
| :---: | :---: | :---: | :---: |
| : 138.500 | 0.012 | 0.897 | 0.08 |
| ! 138,000 | 0.12 | $0.9 \bigcirc 9$ | 3 , 0\%\% |
| ! 138.700 | 0.011 | 0.59? | 0.025 |
| $!138.300$ | 0.012 | $\cdots \mathrm{Or}$ | O.fers |
| ! 138,900 | 0.013 | 3.997 | 0, me |
| ! 135.600 | 0.111 | Q, \%90 | 4, \% 4 |
| -129,100 | 4,011 | 0.507 | ¢ |
| ! 139.200 | 4, 310 | 8.897 | $\therefore \therefore \therefore \sim$ |
| 1 135.300 | 0,010 | 0.997 | 6.02. |
|  | 4, 110 | 0.97 | 0.322 |


| A | M: | \$. |
| :---: | :---: | :---: |
| 197,22 | -70.500 | 1.934 |
| 217.382 | 220.569 | $\therefore \cdots \cdots$ |
| 230.023 | 128,700 | 1, 路 |
| 262.746 | 198.990 | 1.07: |
| 288.710 | 130,900 | 1,797 |
| 317.125 | 159.000 | 1.75 |
| 348.213 | 139.100 | 1.728 |
| 382.212 | 139,300 | 1.495 |
| 419.382 | 179.300 | 3 scs |
| 460.004 | 179.430 | . 22 |

The iztum is: 135100




### 8.0 RECOMMENDATIONS

A combined analysis of the river cross-section and flow data provides useful physical basis for development of rating curves. Two programmes for specific cases of development/use of rating curves have been described above. The Hermitian interpolation is found to be the best suited for interpolation using available rating curves. The LSF progoramme along with physical analysis provides a useful method for the establishment of rating curves.

## REFERENCES

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## APPENDIX I

THE PROGRAMME LSF AND INTPOL


C．EMRHAK FOR CURME FITTSHE
© THE OF CURUE SISER： $3=8 *(4-$ HO）＊＊R
BUK


DSHEMSIOH KMEITODDAESSOO：






$5 \mathrm{~K}=0$



Suk $=\mathrm{C}$
SUMYSR＝0
$3010 I=13$ ，
Y（5）＝ALOKJORTI！


no B $\mathrm{I}=1 \mathrm{~s} \%$
IY！！！＝Y！！！－Y納

U
$30100\{=1+2 \mathrm{NT}$
Suk $=$ ？
Su＊K $=0$
SUAYSB＝
$E R O, 3 j=0,0$

30 15 $\mathrm{J}=\mathrm{y}$ ；芹

15 SUK＝Suntxis）

MS IS $\mathrm{I}=1$ ：留



$813=\mathrm{S}$


$1085 \mathrm{~S}=1 \mathrm{zN}$




35 CONTIME



```
    S@n=1
```



```
    IFGHIS,ER,j60 [0 120
    [M] 120. I=2,4T\
```



```
    AmL:M=[%O!
    ISn=!
120 contimas
    N0 14E I={梅
```



```
    SWIF:C\:HCI:-4%A
```



```
145 c0MTIMUE
```





```
    W\mp@code{ITES:ON}
```











```
    IF{JMSITE.NE.1:STGF
```






```
    Lavefficrent of gurgasation iz: (F8.5')
```




```
    2F10.23515,2:2Y!'!'!
    4%IT(%)121)
121 FD&m的:1%,32%!'-):
    MOITE (Q322)
```



```
    KN= =3/2
    20 158 I=1; %N
    M二小坆
```





```
    IS=1
```




```
    M&JESO:I25;
```

```
125 FONKNT:/%
    CWHE=0.0
154 CH0, =2.0
    20 15S I: \sk
```




```
    IMX=1
255 COM\IM&
```




```
    1scume
```




```
    IS=IS4:
    IF\IS.LE,NGCTS :54
```






```
    W[ITE{9:153)
157 E0RKAT/10%,314*')
    $50F
    EMB
```




