

TR-157

NUMERICAL MODELLING OF GROUNDWATER FOR BULANDSHAHAR AREA



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PREFACE

In December 1985 the governments of The Netherlands and India agreed on an Indo-Dutch Training Programme on Water Management (WAMATRA). Phase I was concluded in 1986-87. Phase II of WAMATRA started in August 1990. National Institute of Hydrology was associated along with other institutions in this project in a few areas. One of the items was Mathematical Groundwater Modelling. The International Institute for Land Reclamation and Improvement, Wageningen, from The Netherlands side was to provide training under this item.

Although the total amount of water on earth is generally assumed to have remained virtually constant, the rapid growth of population, together with the extension of irrigated agriculture and industrial development, is stressing the quantity and quality aspects of the natural system. Because of the increasing problems, man has begun to realize that he can no longer follow a "use and discard" philosophy - either with water resources or any other natural resource. Utilization of ground water has become a tenet in every nation's water resources development policy.

The rational limit of the rate of ground water exploitation should be such that protection from depletion is provided, protection from pollution is provided, negative ecological effects are reduced to a minimum, and economic efficiency of exploitation is attained. Determination of exploitable resources should be based upon hydrological investigations. These investigations logically necessitate use of a mathematical model of ground water system for analyzing and solving the problems.

This report on "Numerical Modelling of Ground Water Basin" has been prepared by Shri Avinash Agarwal, Scientist 'C' as a part of training under INDO-DUTCH TRAINING PROGRAMME ON WATER MANAGEMENT, (WAMATRA), PHASE II at International Institute of Land Reclamation & Improvement.

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1. INTRODUCTION

Mathematical models of groundwater basins and aquifer have been used in solving the hydrologic problems for many years by using equation of groundwater flow for the management of groundwater system. The management here defines the planning, implementation, and adaptive control of policies and programs related to the exploration, inventory, development and operation of water resources including groundwater. Numerical techniques include most of the methods which require a digital computer. Some of hand relaxation techniques are given by Todd (1959), Walton (1970).

In general relaxation techniques used for the groundwater models are finite difference methods. This means that at a given instant of time the solution is computed to approach a steady state solution and the calculation is repeated at successive time. One method of numerical solution of groundwater is a modified Gauss-Seidel relaxation technique which can be used for polygonal shaped network.

This report covers about the package (SGMP) based on the above technique and can be used to predict the impact of man's interference in the hydrological equilibrium of a groundwater basin. It can simulate the effects of new irrigation schemes, new patterns and rates of groundwater abstraction, and artificial recharge of the basin, and can do so for any desired length of time. The model can be applied to an unconfined aquifer, a semi-confined aquifer, or a confined aquifer for saturated flow only. The model allows wide variations in aquifer parameters as hydraulic conductivity and storage coefficient.

The details of the package, its operation in different modes and the interpretation of the results are described in this report. Two case studies are handled with the package provided by the ILRI. The one is the study of Hansi farm in Haryana with the objective of estimation of drainage requirement and amount of drain water. The study yielded that the area has a drainage requirement of only 2.0 mm/day, too less to provide any drain in the area. It is also estimated that the amount of drain water by top to bottom approach is higher than the amount of drain water by bottom to top approach thus normally over estimate the drain design. The other study is carried out on the Bulandshehar district of Uttar Pradesh.

2. DATA REQUIREMENTS

The numerical groundwater studies requires the collection of all existing geological and hydrological data on the groundwater basin. This include information on surface and subsurface geology, water tables, precipitation, evapotranspiration, pumped abstractions, stream flows, soils, land use, vegetation, irrigation, aquifer characteristics and boundaries and groundwater quality. The whole data can be divided in to two categories as physical frame work data and other the hydrological stress data. The data falling in these two categories are reported in Table 2.1. These two sets of data are than used to asses the groundwater balance of the basin.

Table 2.1. Data requirement for numerical groundwater modelling.

Physical framework	Hydrological Stress
1. Topography	1. Water table elevation
2. Geology	2. Type and extent of recharge area
3. Type of aquifers	3. Rate of recharge
4. Aquifer thickness and lateral extent	4. Type and extent of discharge area
5. Aquifer boundaries	5. Rate of discharge
6. Lithological variations within the aquifer	
7. Aquifer characteristics	

2.1. Data Related to Physical Framework

2.1.1. Topography

An accurate topographical map of the groundwater basin to be modelled is a basic requirement. The topographical map should show all surface water bodies, streams and other natural and man made courses along with the contour lines of the land surface elevation. The scale of map depends on the size of basin and the aim of study. The inventory should be made of all the wells in the area i.e. pumped wells that withdraw substantial quantities of groundwater, observation wells. The location of wells and bores should be indicated on the topographical map.

2.1.2 Geology

Intensive geological studies of the groundwater basin are required to delineate its geomorphological features of land forms and to evaluate the manner and degree in which they contribute to the basin's hydrology. Some of these are;

- (a) Area open to deep percolation.
- (b) Subsurface areas where inflow or outflow to or from the aquifer occurs.
- (c) Location and nature of aquifer impermeable base.
- (d) Hydraulic characteristics of the basin.
- (e) Location of any structure affecting groundwater movement.

2.1.3. Type of aquifers

The type of aquifers that can be handled by model provided the flow is laminar are : Unconfined, Confined, Semi-confined.

2.1.4. Aquifer thickness and lateral extent

The lateral extent, thickness and depth of the aquifer may vary from one place to another. The lateral extent of the aquifer, as found from the well and bore logs, and geological data should be indicated on the map. For an unconfined aquifer the net thickness of the aquifer can be calculated subtracting the local clay lenses within the aquifer.

2.1.5.5. Aquifer boundaries

The conditions and the boundaries of the aquifer must be properly defined. Different types of boundaries exists, which may or may not be a function of time. The boundary conditions which can be handled by package are; (a) zero flow boundary, (b) head controlled boundaries and (c) flow controlled boundaries.

A zero flow boundary is a boundary through which no flow occurs. In practice, zero flow boundaries can be defined as those places where flows are insignificant compared with the flows in the main aquifer. In mathematical terms, the condition at the zero flow boundary is $Jh/Jx = 0.0$, where h is the groundwater potential and x is the direction normal to the boundary. In the groundwater model, zero flow is simulated by setting the hydraulic conductivity at the boundary to zero ($k=0.0$).

A head controlled boundary may or may not be a function of time. Mathematically, a head controlled boundary that changes with time is expressed as $h = f(x,y,t)$, i.e. the head is function of both place and time. A fixed head at the boundary is expressed as $h = f(x,y)$, i.e. the head is a function of place only.

A flow controlled boundary is a boundary through which a certain volume of groundwater enters the aquifer per unit time. Flow controlled boundaries are simulated by setting the hydraulic conductivity at the boundary equal to zero ($k=0.0$), and entering the underflow in to the model as a recharge term.

2.1.6. Lithological variation within the aquifer

No aquifer is lithologically uniform over its entire extent. Both lateral and vertical variations occur, which can be recognized as facies changes. In one part of the basin the aquifer may be predominantly sand and gravel, whereas in other parts fine sand or even silt and clay may predominate. Since grain size has a great bearing on hydraulic conductivity and porosity, and thus on the flow and storage of groundwater, a study of facies changes forms an intrinsic part of the groundwater basin modelling.

2.1.7. Aquifer characteristics

The magnitude and spatial distribution of the aquifer characteristics must be specified. Depending on the type of aquifer these characteristics may be;

- # hydraulic conductivity, k (for all types of aquifers)
- # storage coefficient, S (for confined and semiconfined aquifers)
- # specific yield, μ (for unconfined aquifers)
- # hydraulic conductivity, k' (for the confining layer overlaying a semiconfined aquifer)
- # specific yield, μ' (for the confining layer overlaying a semiconfined aquifer)

Various field, laboratory, and numerical methods are available to determine or estimate these characteristics.

2.2. Data Related to Hydrological Stress

Hydrological stress exerted on a groundwater basin, from the infiltration of rainfall, infiltration of irrigation water, from stream bed percolation, evapotranspiration, groundwater discharge by streams and springs, and pumping from wells, is reflected in the configuration and fluctuation of the water table in the basin. A part of the hydrogeological investigations, a water table survey should therefore be made.

2.2.1. Water table elevation

To study the magnitude and distribution of the hydraulic head in a basin, an appropriate network of observation wells and piezometers must be established throughout the basin and along its boundaries. In selecting proper sites for observation wells, one must consider variation in aquifer thickness, lateral and vertical variations in the aquifer's lithology, and the structural deformation of the aquifer, if any. Once the network of observation wells has been established, the water levels in all the wells should be measured. The water level data are used to draw the following maps;

- # water table contour maps
- # depth to water table maps
- # water table change maps
- # head difference maps.

2.2.2. Type and extent of recharge area

Estimating the recharge of an aquifer forms a basic part of any groundwater study, and it is not the easiest part. The difficulty is that the process by which rainfall or surface water infiltrates into the soil, moves downward through the unsaturated zone, and eventually joins the groundwater as deep percolation, is not well understood. The type and extent of recharge area and the source of recharge should first be determined. Although they will differ for different types of aquifers, the main factors governing the recharge of all aquifers are topography, surface geology, and climate.

Besides percolation of rainfall, other sources of recharge can be stream bed percolation and surface runoff from adjacent hilly terrain. The amount of deep percolation of each component should be specified either by taking direct measurement or by determining the deep percolation as the residual of the amounts of surface water supply, use, and discharge.

2.2.3. Rate of recharge

Several methods can be used to estimate the recharge of

unconfined aquifer. The use of groundwater flow, runoff hydrograph, rainfall water table relations, lysimeters, use of tensometers, use of isotopes and stream flow measurement are some of the methods for the estimation of rate of recharge.

2.2.4. Type and extent of discharge area

Discharge areas are areas where the aquifer loses water by overflow, evaporation diversion, migration through confining layers, and pumpage. A study of topographical maps and areal photographs, in addition to field surveys, will be of great help in determining the type of discharge areas and delineating them.

2.2.6. Rate of discharge

To determine the rate of discharge the total quantity of groundwater, discharge by springs, regular measurements of their flows must be taken.

Springs are the most obvious form of groundwater. They occur in all sizes. Some springs have fairly constant discharge, but most of them have variable discharge: high during and after rain, low or nil in the dry season. To determine the total quantity of groundwater discharge by springs must be taken.

Seepage of groundwater into a stream occurs when the water table adjacent to the stream is higher than the water level in the stream. The ground water discharge towards stream channels can also be estimated from the separation of hydrographs.

Evapotranspiration is the combined effect of evaporation of water from moist soil and the transpiration of water by natural vegetation and cultivated crops. Determining these water losses to atmosphere is not an easy task. In shallow water table areas, the ground water contributes to evapotranspiration through capillary rise. This discharge is determined by the depth of ground water below the root zone, the capillary and conductive properties of the soil, and the soil water content or soil water tension in the root zone.

An aquifer can lose water by upward seepage through an overlaying slightly permeable layer. This happens can be determine by placing piezometers in the aquifer and in the overlaying layer. All types of aquifers may lose ground water by lateral subsurface out flow through natural or imposed boundaries. The out flow rate must be determined or estimated, using the available methods.

Finally the tube wells and pumping stations that abstract ground water for domestic, industrial, or irrigation water supplies can be regarded as discharge areas. To estimate the total abstraction from the aquifer, an inventory of all pumped wells should be made.

3. DESCRIPTION OF MODEL

3.1 General

The present model has the following features and restrictions.

- #. The aquifer is treated as a two-dimensional flow system i.e. in horizontal plane.

- # The lower aquifer is bounded at the bottom by an impermeable layer.
- # The upper boundary of the aquifer is a free water table (water table conditions) or impermeable layer (confined condition).
- # The upper boundary of the aquifer is either an impermeable layer (confined conditions) or a free water table (water table conditions).
- # The calculation procedures for both confined and unconfined conditions, depending on the relative position of the piezometric level with regard to the upper level of the aquifer, have been incorporated into the model. The model calculates the change in storage with either a specific yield coefficient (water table conditions) or a storage coefficient (confined conditions).
- # Both specific yield and storage coefficients can vary in a horizontal direction.
- # Transmissivity are calculated as the algebraic product of hydraulic conductivity and saturated thickness; both can vary in a horizontal direction.
- # For unconfined aquifers the transmissivity varies with time; the model adjusts the saturated thickness according to the calculated water table elevation (nonlinear conditions) only the hydraulic conductivity and the bottom of the aquifer must be prescribed.
- # Both unsteady and steady-state conditions can be simulated by the model.
- # Darcy's law (linear resistance to laminar flow) and Dupuit's assumptions (vertical flow can be neglected) are applicable for each individual aquifer.
- # The groundwater model can simulate head-controlled, flow controlled, and/or zero flow boundaries; the first two may vary with time.
- # Limits between which the water-level in the aquifer is allowed to vary can be prescribed. If the water-level exceeds a certain limit, the model introduces an artificial flow rate that will keep the calculated water-level within that limit.
- # The model can be run in the so-called increase mode. It will then calculate the nodal net recharge values as function of the prescribed watertable elevations.
- # The processes of infiltration and percolation of rain and surface water and of capillary rise and evapotranspiration taking place in the unsaturated zone are not simulated by the model. This means that the net recharge to the aquifer system must be calculated manually and prescribed to the model.
- # Featheredge boundary conditions, where the saturated

thickness reduces to zero as the water-level drops below the base of the aquifer, are included in the model.

3.2. Physical background of model

The model is based on the two well-known equations: Darcy's law and the equation of conservation of mass. The combination of these two equations results in a partial differential equation for unsteady flow as:

$$\frac{\partial}{\partial x} \left(k D \frac{\partial h}{\partial x} \right) + \frac{\partial}{\partial y} \left(k D \frac{\partial h}{\partial y} \right) = -N \quad \text{---(1)}$$

(horizontal flow) (Vertical flow)

where:

$k(x, y)$ = hydraulic conductivity of the aquifer for horizontal flow ($L T^{-1}$)

$D(x, y, t)$ = saturated thickness of the aquifer at time t (L)

$h(x, y, t)$ = hydraulic head in the aquifer at time t (L)

$N(x, y, t)$ = source or sink term at time t ($L T^{-1}$)

For unconfined aquifers N is the sum of three terms :

$$N = R - P - u \frac{\partial h}{\partial t} \quad \text{---(2)}$$

where:

$R(x, y, t)$ = the net rate of recharge ($L T^{-1}$)

$P(x, y, t)$ = the net rate of abstraction ($L T^{-1}$)

$u(x, y)$ = specific yield of the aquifer (dimensionless)

$h(x, y)$ = hydraulic head in the aquifer (L)

t = time (T)

The term $u \frac{\partial h}{\partial t}$ is related to the movement of the free water table in case of unsteady flow. When the water table moves downwards or upwards, water is released from or taken in to storage. The specific yield u is defined as the volume of water released or stored per unit surface area of the aquifer per unit change in the component of head normal to that surface. In unconfined aquifers the saturated thickness D is not constant, but is a function of the position of the free water table at time t .

For confined aquifers N is the sum of only two terms :

$$N = -P - S \frac{Jh}{Jt}$$

---(3)

where:

S = the storage coefficient of the aquifer (dimensionless)

For confined aquifer S is called the storage coefficient and is defined in the same way as the specific yield for unconfined aquifers.

3.3. Numerical approaches

The solution of a partial differential equation can be obtained by using a finite difference method. This method requires that space be divided in to small but finite intervals. The sub areas thus formed are called nodal areas. It is assumed that all the recharge and abstraction in a nodal area occur at the node, in other words, each node is considered to be representative for that nodal area. An appropriate solution of the partial differential equation (1) can be obtained by replacing it with an equivalent system of difference equations, the simultaneous solution of which gives the function of h at a finite number of nodes. Since the water table at the nodes changes with time owing to change in recharge and abstraction, the model also requires a discretization of time.

3.4. Design of nodal network

To discretize a ground water basin in to small areas, a network of rectangles, squares, or polygons is superimposed upon it. It is impossible to give any hard and fast rules on what network to apply and how to design it. Because of different geological hydrogeological conditions, a network that is appropriate in one basin will be inappropriate in another. There is certain general procedure to be followed in designing a network considering the following factors;

- (a) the type of problems to be solved,
- (b) the required accuracy of the results,
- (c) the homogeneity or heterogeneity of the aquifer,
- (d) the availability of data,
- (e) the shape of boundaries,
- (f) the number of nodes.

4. PROGRAMME DESCRIPTION

4.1. Package description

The package consists of four computer programs, in which the calculation processes are recorded. The parts together form the 'Standard Groundwater Model Program, one-layer version (SGMP1)', which consists of :

- SGMP11 (reading of input data/nodal network)
- SGMP12 (calculation)
- SGMP13 (printing of results)
- SGMP14 (plotting of results)

One can choose to have the results of the calculations either printed out with SGMP13 or plotted out with SGMP14; if so

desired, the results can be both printed and plotted. With printouts one obtains the groundwater balances of each nodal area. With plot-outs, one can compare the calculated watertable elevations with the historical ones; in addition, plot-outs are most useful in indicating the long-term behaviour of the watertable. The file operation flow chart is given in Figure 3.1.

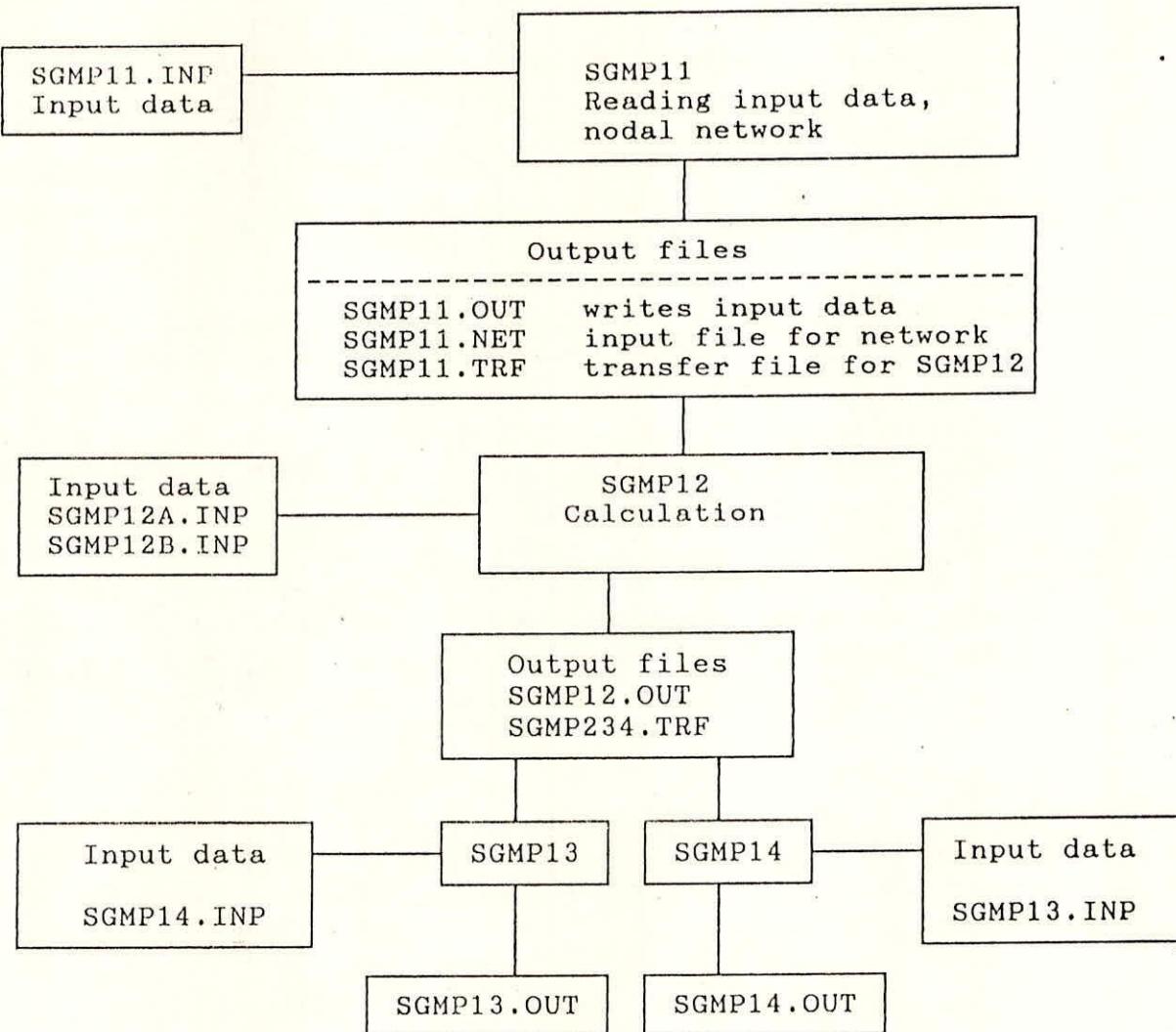


Fig. 3.1: File operation flow chart of SGMP package.

4.2. Programme, SGMP11.EXE

The data required for SGMP11 are the data of the nodal network, and those of the hydrogeological parameters and other variables. For the design of a nodal network and for techniques to produce the data on hydrogeological parameters reference is made to ILRI-publication 29. SGMP11 reads the required input data from a file SGMP11.INP.

4.2.1. Input file, SGMP11.INP

Each group of data is headed by an identification line, one or more lines with the actual data, and terminated with a blank line.

The structure of SGMP11.INP needs no specific explanation

and is as follows :

----- line with text for identification -----
----- line with text for identification -----
<blank line>
NN NSIDES
-- values -
<blank line>
TMBAS DMTIM T DELTA MINOR MAJOR LIST DL
-----values-----
<blank line>
SCALE ERROR COEFFA PREDIC DELQ LSW1 LSW2 NITER
-----values-----
<blank line>
K NTYP(K) CO(K,1) CO(K,2) SL(K) TL(K) BL(K) UL(K) LL(K)
-----NN lines with values-----
<blank line>
K AS(K) ASC(K)
- NN lines values -
<blank line>
IB IE CONDU
- NSIDES lines values -

The input variables of SGMP11.INP are presented in the sequence in which they appear in this data set. Their definition is as follows :

TITLE 1 = name of basin, or any specification the user wishes.
TITLE 2 = for identification (2 lines of 80 characters each).
NN = the total number of nodes in the nodal network. As has been explained, the area is split up into nodal areas, the nodes of which represents the specific aquifer characteristics in each area.
NSIDES = the total number of sides in the nodal network.
TMBAS = unit time of time step and boundary conditions. It is expressed as a word : week, month, etc.
DMTIM = number of days in the unit TMBAS.
T = actual time at the beginning of the calculation process, expressed as a numeric in the dimension TMBAS. If the computations start on 15 August and TMBAS has the dimension MONTH, T is equal to 7.5.
DELTA = time. The choice of the time step is initially arbitrarily, although it is influenced by two factors :
If the time step is chosen too large, the approximation of finite difference to differentials will cease to be valid and the results will be in error.
An interaction exists between the amount of data for the boundary conditions and the size of the time step chosen; if, for example, the boundary conditions are known on a weekly basis, one will not take a time step of a month and vice versa.
For any particular set of data, it is advisable to run the

program with various DELTA values. When the results are compared, it can readily be seen for what maximum value of DELTA the results do not appreciably change.

- # Common time steps are a week, a fortnight, or a month for regional studies and a day for local studies like aquifer tests.

The dimension of DELTA is the unit TMBAS.

MINOR	=	the number of DELTA periods in the first accumulation level.
MAJOR	=	the number of first accumulation level periods in the second accumulation level.
LIST	=	the number of second accumulation level periods in the total time period considered.
DL	=	a range in watertable elevation; it is included to make it possible that once a watertable has exceeded an upper limit and an additional abstraction rate is introduced by the package to keep the watertable below that level, this additional abstraction rate becomes smaller, (ultimately reduces to zero) once the watertable declines to a lower elevation. This reduction takes place when the watertable drops below the level UL(K) - DL. It is expressed in metres.
SCALE	=	scale of the map used for the nodal network and from which the coordinates of the nodes are read.
ERROR	=	a tolerance level that directly affects the accuracy of the final results. The dimension of ERROR is the same as for the other components of the water balance : a volume per time. The unit of volume depends on the value of the switch LSW1 (1 m ³ or 1,000,000 m ³) and the unit time is equal to TMBAS.
COEFFA	=	a relaxation coefficient. Tyson and Weber (1963) have suggested that such a coefficient can increase the speed of convergence. An optimum value for it can be obtained in the same way as for DELTA. The range of the coefficient is between 0.8 and 1.2.
PREDIC	=	coefficient which can increase the speed of calculations; especially in situations where there is a monotonous tendency in recharge or abstraction. Default value for PREDIC is 1; in the above described situations the value for PREDIC can be taken greater than 1.
DELQ	=	an additional percolation or abstraction rate which is introduced by the package once the watertable exceeds the UL(K) and LL(K) levels. Determining the value of DELQ is a matter of trial and error. When too high a value is used, the watertable will remain far from the

prescribed limit. If it is too low, too many iterations will be needed before the watertable remains within the limit.

The unit of volume depends on the value of the switch LSW1 (1m or 1,000,000 m); the unit of time is TMBAS. NITER = prescribes the maximum number of iterations which are allowed, before the calculation process is terminated. In such a situation a message 'RELAXATION FAILS TO CONVERGE' is printed, and the calculations are started for the next time step. For unsteady state runs a value of 250 for NITER has been found to work satisfactorily.

LSW1 = an external switch that can be given two values. If the scale of the map is very large, the nodal areas become very large when expressed in square metres; the same applies to pumping rates expressed in cubic metres. These large values can be avoided by using LSW1.

Setting LSW1 equal to 1 means :

unit area = 1,000,000 square metres
unit volume = 1,000,000 cubic metres

Setting LSW1 equal to 2 means :

unit area = 1 square metre
unit volume = 1 cubic metre

Note : The unit length is always fixed (metre); it is used in the hydraulic conductivity values (metre/day) and in the elevation values (metres above a certain reference datum).

LSW2 = an external switch that can be given two values. Setting LSW2 equal to 2 means that no specific input values for UL(K) and LL(K) are needed. The program automatically attaches the values of BL(K) to LL(K) and the value of 9999 to UL(K). If the aquifer is unconfined (SL(K)=TL(K)), the package attaches the values of SL(K) to UL(K).

Setting LSW2 equal to 1 means that the watertable elevations are prescribed within certain limits and that specific input values for these limits must be given for some or all of the nodal areas. For other nodal areas without prescribed limits, one must give UL(K) and LL(K) the values of respectively SL(K) and BL(K).

NITER = maximum number of iterations in each time step. A check has been included in the program: if, after the prescribed maximum number of iterations, the sum of the absolute values of all the residuals is still greater than the value of ERROR, the iterations are terminated: a message 'RELAXATION FAILS TO CONVERGE' is printed, and the calculations are started for the next time step.

NTYP(K) =	a node type can be given three values. Setting NTYP(K) equal to 1 means a node at the boundary of the nodal network for which the watertable elevations must be prescribed.
	Setting NTYP(K) equal to 2 means a node at the boundary of the nodal network for which the watertable elevations are calculated by the program. Setting NTYP(K) equal to 3 means a node inside the nodal network for which the watertable elevations are calculated by the program.
CO(K,1) =	x and y coordinates of all the nodes. These are measured in cm with regard to an arbitrarily chosen cartesian coordinate system laid on the map of the network configuration. When a triangle formed by three neighboring nodes possesses an angle greater than 90 degrees, a message is printed indicating which of the nodes this is.
SL(K) =	elevation of the land surface in each nodal area, weighted over the nodal area (m above a reference level).
TL(K) =	top boundary of the aquifer in each nodal area, weighted over the nodal area (m above a reference level). If the shallow aquifer is unconfined, TL(K) must have the same value as that of SL(K).
BL(K) =	bottom boundary of the aquifer in each nodal area, weighted over the nodal area (m above a reference level).
UL(K) =	upper limit to which the watertable in each nodal area is allowed to rise. The dimension is m above a reference level.
LL(K) =	lower limit to which the watertable in each nodal area is allowed to fall. The dimension is m above a reference level.
AS(K) =	mean specific yield of the aquifer in each nodal area, weighted over the nodal area (dimensionless).
ASC(K) =	mean storage coefficient of the aquifer in each nodal area, weighted over the nodal area (dimensionless).
CONDU =	mean horizontal conductivity of the aquifer at each nodal side, weighted over the length of the side. The dimension is fixed: m/day. In the program the hydraulic conductivity for each nodal side is represented by the pair of node numbers in between which the side lies. the value of the hydraulic conductivity of a side needs only to be prescribed once. IB and IE representing pair of node numbers.

WARNING : the value of the node number IB must always be less than that of the node number IE.

4.2.2. Output files

The output of SGMP11 consists of a reformatted playback of the input data and is stored on a file SGMP11.OUT. The data concerning the geometry of the nodal network are stored on a file SGMP11.NET to be used by NETWORK. With this program the user can visually check his design of the nodal network and make a plot of the network on paper. This file is unformatted and in binary form.

The data are further processed and stored on another file SGMP12.TRF to be used by SGMP12. This file is unformatted and in binary form.

4.3. Programme, SGMP12.EXE

Apart from the data read from SGMP12.TRF (see previous section), initial and boundary conditions and three external switches are prescribed for this program. SGMP12 reads the required input data from two files SGMP12A.INP and SGMP12B.INP.

For SGMP12 the data sets have been split up in two parts for the following reason. Usually, the recharge QNET(K) consists of different kinds of recharge and abstraction rates, like effective rainfall, seepage losses from rivers, irrigation losses from main channels, minors and fields, pumping rates from shallow wells and deep tube wells, etc. To prepare these manually, it is often very time consuming. The user can then better use a computer program which produces the values of QNET(K). Because the various components will be different for each study, no standard program, however, can be given. Such a program should produce values of QNET(K) according to the specifications of SGMP12B.INP.

4.3.1. Input file, SGMP12A.INP

Each group of data is headed by an identification line, one or more lines with the actual data, and terminated with a blank line. The structure of SGMP12A.INP is as follows :

LSW3	LSW4	LSW5	LSW6
----- values -----			
<blank line>			
HINIT(K) :	initial	watertable	elevations
	----- NN values -----		
<blank line>			
H(K) :	watertable	elevations	time step 1
	----- NN values -----		
<blank line>			
DELTA :	time step 1		
	----- value -----		
<blank line>			
H(K) :	watertable	elevations	time step 2
	----- NN values -----		
<blank line>			
DELTA :	time step 2		
	----- value -----		
<blank line>			

It must be noted that above structure is influenced by the values of the external switches LSW3, LSW4 and LSW5.

The set of H(K) is repeated LIST times if LSW3 = 1, LIST x MAJOR times if LSW3 = 2, and LIST x MAJOR x MINOR times if LSW3 = 3. This is the case for LSW4 = 2; for the combination of LSW4 = 1 and LSW3 = 2, the set of H(K) is repeated MAJOR times.

The set of DELTA must be skipped if LSW5 is equal to 1.

The input variables of SGMP12A.INP are presented in the sequence in which they appear in this data set. Their definition is as follows :

LSW3 = an external switch that can be given three values. Setting LSW3 equal to 3 means that for every time step DELTA the boundary conditions must be specified.

Setting LSW3 equal to 2 means that for every first accumulation level (MINOR time steps DELTA) the boundary conditions must be specified.

Setting LSW3 equal to 1 means that for every second accumulation level (MAJOR x MINOR time steps DELTA) the boundary conditions must be specified.

The actual value of LSW3 depends on the chosen time step DELTA and on the time basis for which the boundary conditions are available.

LSW4 = an external switch that can be given three values. Setting LSW4 equal to 2 means that the boundary conditions are read for each DELTA, first or second accumulation level depending upon the value of LSW3. LSW4 is equal to 2 in the calibration runs. In prediction runs the boundary conditions are usually prepared on a monthly basis for one year. For each year in the total time period, the same input data as given for the first year are used. LSW4 is then equal to 1 and LSW3 to 2. LIST then has the dimension year and MAJOR the dimension month.

LSW5 = switch that can be given two values. Setting LSW5 equal to 1 means that a fixed time step DELTA is used in the calculation process. Its value is read in the first program SGMP11. Setting LSW5 equal to 2 means that a variable time step is used. For each time step the value of DELTA must now be prescribed, starting with the first time step, at the same time overriding the value of DELTA given in the first program SGMP11. A variable time step is often used in simulating aquifer tests (logarithmic time step), but for regional groundwater flow problems a fixed time step is preferred.

LSW6 = an external switch that can be given two values. Setting LSW6 equal to 1 means that the model runs

in the so-called inverse mode. the UL(K) and LL(K) values are made equal to the prescribed H(K) values of each time step plus or minus half the DL value, at the same time overruling the values as they were prescribed in the first program SGMP11. The program will calculate the corresponding nodal net recharge values. It is advised to make the qnet(K) values equal to zero.

To simulate steady-state conditions, the user must repeat the initial watertable elevations a number of time steps. In this way it is not necessary to make the specific yield values or storage coefficients equal to zero.

Setting LSW6 equal to 0 means that the model runs in normal mode.

HINIT(K) = initial watertable elevations of the aquifer for all nodes, measured at the nodes themselves (m above a reference level).

H(K) = watertable elevations of the aquifer for all nodes.

If LSW6 equals zero, only values need to be prescribed for the nodes at the boundaries of the nodal network with prescribed watertable elevations (NTYP(K)=1); for the remaining nodes no specific value of H(K) is required (can be taken as zero). If LSW6 equals one, values of H(K) must be prescribed for all the nodes.

4.3.2. Input file, SGMP12B.INP

Each group of data is headed by an identification line, one or more lines with the actual data, and terminated with a blank line. The structure of SGMP12B.INP is as follows :

```
QNET(K) : net recharge time step 1
----- NN values -----
<blank line>
QNET(K) : net recharge time step 2
----- NN values -----
<blank line>
```

It must be noted that above structure is influenced by the values of the external switch LSW3 and LSW4.

The set of H(K) is repeated LIST times if LSW3 = 1. LIST x MAJOR times if LSW3 = 2, and LIST x MAJOR x MINOR times if LSW3 = 3. This is the case for LSW4 = 2; for the combination of LSW4 = 1 and LSW3 = 2, the set of H(K) is repeated MAJOR times.

The input variables of SGMP12B.INP are the net recharge values. Their definition is as follows :

GNET(K) = net recharge rate in each nodal area, being the sum of a number of external flows with the dimension volume per time. The unit of volume depends on the value of the switch LSW1 (1 m³ or 1,000,000 m³) and the unit time is equal to TMBAS. A positive sign means recharge to the

aquifer.

Only values need to be prescribed for the nodes for which the parameter NTYP(K) has the value 2 or 3. For the remaining nodes (NTYP(K)) = 1) no specific value of QNET(K) is required (can be taken as zero).

4.3.3. Output files

In SGMP12 the watertable elevations at the nodes and the groundwater balances of the nodal areas are calculated for each time step and the results are stored on a file SGMP234.TRF. This file is unformatted and in binary form.

As the results are stored on SGMP234.TRF, the only direct output is the heading, the values of the three external switches, and an overview of the subsurface in- and outflows along the head controlled boundary nodes of the nodal network.

Included in the calculation process is a check against certain situations. If they occur, one of the following messages will be printed.

'RELAXATION FAILS TO CONVERGE AT TIME'
'DELQ IS TOO SMALL AT TIME'
'WATER LEVEL AT NODE IS AT ITS BASE AT TIME'

The above-mentioned output is stored in SGMP12.OUT. In addition, the calculated watertable elevations for each time step are stored in a file HCAL.PLT. These data can be used for making watertable contour maps with commercial available programs like SURFER.

4.4. Programme, SGMP13.EXE

In addition to the required calculated watertable elevations and groundwater balance components which are read from SGMP234.TRF, SGMP13 only requires values for three external switches.

4.4.1. Input file, SGMP13.INP

The only input values are the values of three external switches. The structure of SGMP13.INP is as follows :

LSW7 LSW8 LSW9
----- values -----

The three switches are defined as follows :

LSW7 = an external switch that can be given three values. Setting LSW7 equal to 1 means that the watertable elevations are printed at each time step DELTA. Setting LSW7 equal to 2 means that the watertable elevations are printed at each first accumulation level (MINOR time steps DELTA).

LSW8 = an external switch that can be given three values. The meanings of the values 1, 2 and 3 for this switch are analogue to those of LSW7. Instead of

printing watertable elevations at various time levels, the components of the water balance of the nodal areas are calculated over the various time levels and printed after each particular time level.

- LSW9 = an external switch that can be given two values. Setting LSW9 equal to 1 means that only the watertable elevations of the internal nodes are printed. Setting LSW9 equal to 2 means that the watertable elevations of all the nodes are printed. This is usually done only once to check the head controlled boundaries numerically.

4.4.2. Output files

SGMP13 stores the results of the calculations in tabular form in a file SGMP13.OUT.

4.5. Programme, SGMP14.EXE

In addition to the required calculated watertable elevations which are read from SGMP234.TRF, SGMP14 requires various signs for the plot, selection of plot nodes, values for two external switches and historical watertable elevations for the selected nodes.

4.5.1. Input file, SGMP14.INP

Each group of data is headed by an identification line, one or more lines with the actual data, and terminated with a blank line. The structure of SGMP14.INP is as follows :

```
BLNK   HI   HMINUS   HX   ASTRSK   HY   PLUS   LSW10   LSW11   NNS
----- signs for plotting ----- ----- values -----
<blank line>
NS(I) : internal node numbers with historical w.t.e. available
----- NNS values -----
<blank line>
MINOR      MAJOR      LIST
----- values -----
<blank line>
HH(K) : historical watertable elevations time step 1
----- NNS values -----
<blank line>
HH(K) : historical watertable elevations time step 2
----- NNS values -----
<blank line>
```

It must be noted that above structure is influenced by the values of NNS and LSW10.

If NNS is equal to NN, the set of NS(I) must be skipped. When NNS is less than NN, a sequence of node numbers must be prescribed. It must be noted that in this sequence the nodes for which NTYP is equal to 1, are not required since they were already based on historical data.

Setting NNS equal to NN is usually done only once to provide

a visual check on the head-controlled boundary nodes.

If LSW10 is equal to 1, the sequence of HH(K) is as follows:

historical watertable elevation at node NS(1) for time step 1;
historical watertable elevation at node NS(2) for time step 2;
till node NS(NNS). This sequence of historical watertable
elevations must be repeated MAJOR times.

If LSW10 is equal to 2, the sequence of HH(K) must be 4
skipped. The definition of these variables is as follows :

BLNK	=	always a blank.
HI	=	a sign that indicates the unit of scale.
HMINUS	=	a sign that indicates the tenth of the unit of scale.
HX	=	a sign that indicates when a calculated watertable elevation falls outside the maximum range of the scale.
ASTRSK	=	a sign that indicates the position of a calculated watertable elevation in the graph.
HY	=	a sign that indicates when a historical watertable elevation falls outside the maximum range of the scale.
PLUS	=	a sign that indicates the position of a historical watertable elevation in the graph.
LSW10	=	an external switch that can be given two values. Setting LSW10 equal to 1 means that both calculated and historical watertable elevations will be plotted. This is required for the calibration process, in which the calculated watertable elevations are compared with the historical ones. Setting LSW10 equal to 2 means that only the calculated watertable elevations are plotted. This is done in prediction runs, when no historical water levels are available.
LSW11	=	an external switch that can be given two values. Setting LSW11 equal to 1 means that an error calculation is made of the deviations between the calculated and historical watertable elevations. For the selected nodes of the network, these deviations are calculated for each time step for which historical watertable elevations are available; the mean and standard deviation are then calculated and printed at the end of all the plots. Setting LSW11 equal to 2 means that no error calculation is made.
NNS	=	total number of nodes for which groundwater hydrographs must be plotted.

NS(I) = sequence of node numbers, selected for plotting hydrographs.

MINOR = In prediction runs these parameters should be given values that differ from those prescribed in the calculation program. Suppose, the model is run for 30 years with a time step of a fortnight and the boundary conditions are prescribed on a monthly basis. The parameters of the time discretization are then :

TMBAS : month
DELTA : 0.5
MINOR : 2
MAJOR : 12
LIST : 30

Using the same values in the plot program would mean that the water levels are plotted for each month, because they are plotted at the end of each first accumulation level. In general one will not need that much information, and water levels plotted after each half year will usually suffice. The values of MINOR, MAJOR and LIST are then :

MINOR : 12
MAJOR : 60
LIST : 1

H(NS(K),M) = historical watertable elevations of the aquifer for the selected nodes. It must be noted that these are not required for the nodes for which NTYP is equal to 1 since they were already based on historical data.

4.5.2. Output files

SGMP14 stores the calculated and, when available, historical watertable elevations at the selected nodes in a file SGMP14.OUT.

5. CASE STUDY I

5.1. Case Study of Hansi Form in Haryana

A rectangular farm, 1600 m long and 860 m wide (area = 136 ha), is located in the flat alluvial plain on Haryana. An irrigation canal crosses the farm approximately in the middle. The crops cultivated on the farm are irrigated with the water from this canal. Rice is grown in the strip on both sides of the canal, and cereals and other field crops on the remaining parts of the farm. The land surrounded the farm is also cultivated, but because of the storage of irrigation water, they are not supplied with water from the canal. During the irrigation season it was found that the water table in the parts of the form was rather shallow, and the question arose whether the land need artificial drainage. Shallow piezometers were placed in a regular grid, and monthly readings were taken of there water tables and is reported in Figure 5.1. The location, where the aquifer transmissivity has been determined are located in Figure 5.1 along with the transmissivity as 250, 260, 350, 170 and 290 m^2/day respectively.

To run the programme SGMP11 the input data files SGMP11.INP, SGMP12A.INP, SGMP12B.INP and SGMP13.INP are required. In order to run the programme the following major steps are taken.

- (a) To make a time descritization.
- (b) To draw a nodal network and to estimate the coordinates of each node.
- (c) To interpolate water table depth, depth to water table, ground level, electrical conductivity at each node and mean hydraulic conductivity by the sides of nodes.

5.2. Time discritization

Since the groundwater data is on monthly basis and only recorded during irrigation period the values of TMBAS is MONTHS and the average number of days on a unit DMTIM is 30.5. The calculation starts with beginning therefore value of T is equal to 0.0. According to the problem the values of DELTA, MINOR, MAJOR and LIST were taken as follows;

TMBAS = Month
DMTIM = 30.5
DELTA = 1
MINOR = 1
MAJOR = 12
LIST = 1

The time descritization should follow following relations:

$$\text{Total simulation period in TMBAS} = \text{DELTA} * \text{MINOR} * \text{MAJOR} * \text{LIST}$$

$$12 = 1 * 1 * 12 * 1$$

Since, only the monthly data, that too, after the irrigation period is available, it was assumed that the data remains unchanged at next time step i.e. next year at the same time and therefore, the problem becomes of a steady state condition.

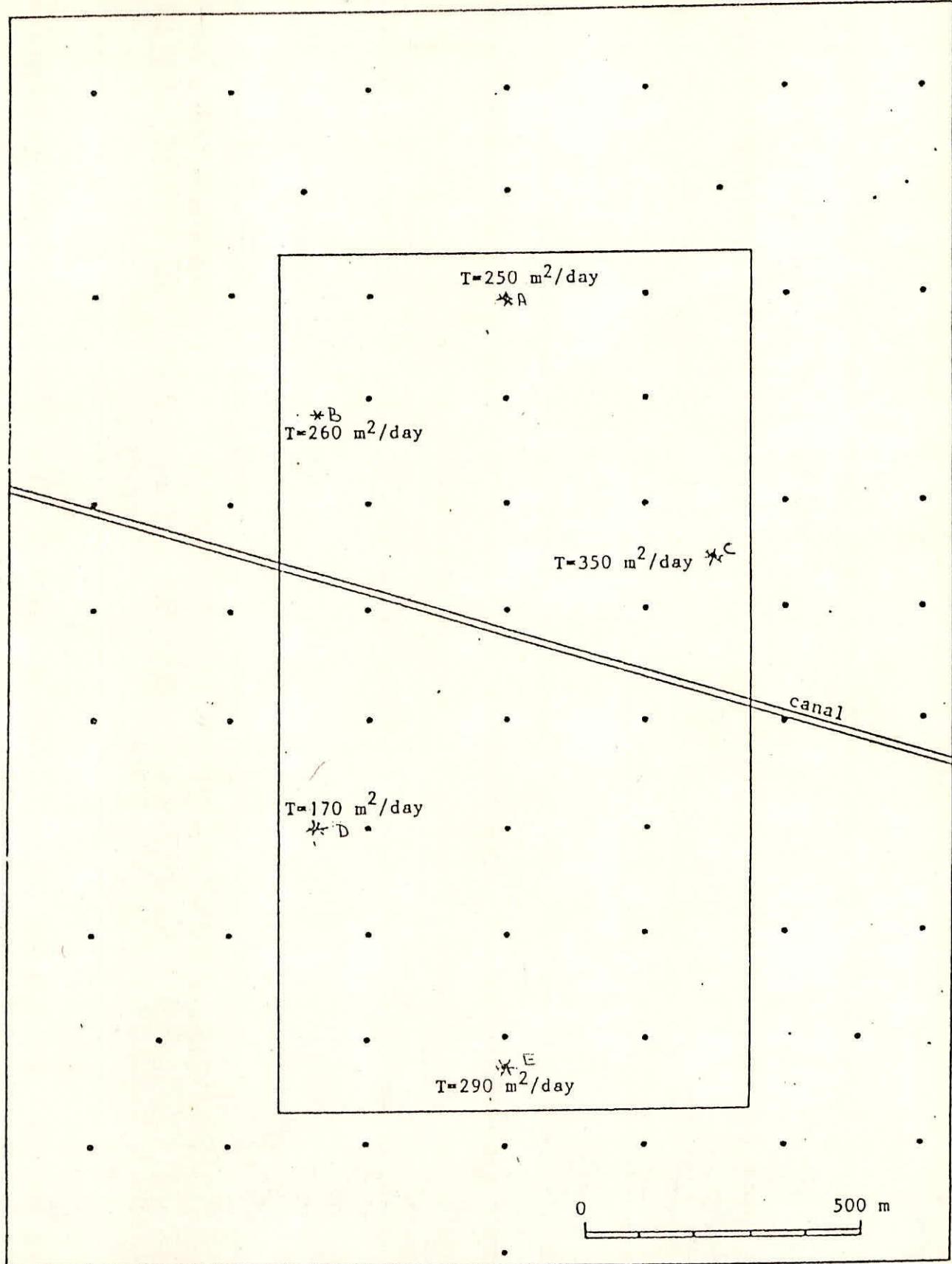


Fig. 5.1: Location of observation wells and transmissivity test sites.

• ; Location of observation well
* ; location of aquifer transmissivity site, A to E

Figure not to the scale.

*	*	*	*	*	*	*	*
W=6.7 D=3.2	W=7.3 D=2.6	W=7.2 D=2.7	W=7.2 D=2.2	W=7.9 D=1.6	W=8.4 D=1.5	W=8.9 D=1.4	
*	*	*	*	*	*	*	*
		W=7.1 D=3.4		W=7.0 D=2.5		W=7.9 D=1.9	
Irrigated farm							
*	*	*	: A	*	*	*	*
W=6.5 D=3.1	W=7.6 D=3.5		W=8.0 D=3.0	W=7.9 D=2.4	W=8.0 D=2.3	W=8.3 D=1.8	W=8.4 D=1.5
*	*	: B	*	*	*	*	*
			W=8.6 D=2.5	W=8.4 D=2.0	W=8.5 D=1.9		
#	*	*	*	*	*	*	*
W=7.8 D=1.3	W=8.2 D=2.1		W=9.1 D=1.4	W=9.5 D=0.7	W=9.1 D=1.1	W=9.0 D=1.3	W=8.5 D=1.7
*	*				: C	*	*
W=8.0 D=2.2	W=8.6 D=2.3		W=9.6 D=1.0	W=10 D=0.4	W=9.4 D=0.6	W=9.1 D=1.4	W=8.9 D=1.6
*	*					*	*
W=7.7 D=2.5	W=8.4 D=1.8		W=9.5 D=0.9	W=9.8 D=0.6	W=9.4 D=0.5	W=9.2 D=1.2	W=9.0 D=1.5
*	*	: D	*	*	*	*	*
			W=8.9 D=1.4	W=9.6 D=0.8	W=9.1 D=0.8		
*	*					*	*
W=7.1 D=2.9	W=7.4 D=2.5		W=8.3 D=1.9	W=9.1 D=1.5	W=8.7 D=1.7	W=8.5 D=2.1	W=8.5 D=3.2
*	*	*				*	*
W=6.8 D=3.3			W=7.9 D=2.2	W=8.3 D=1.6	W=8.8 D=1.9	W=8.1 D=2.5	
				: E			
*	*	*	*	*	*	*	*
W=6.5 D=3.7	W=6.9 D=3.5	W=7.4 D=2.9	W=7.7 D=2.4	W=7.8 D=2.2	W=7.6 D=2.6	W=7.7 D=2.5	
*	*	*	*	*	*	*	*
			W=7.1 D=3.6				

Fig. 5.2: Detailed observations during irrigation season of a rectangular farm of 1600 m long and 860 m wide in a flat alluvial plain with inner area irrigated by canal.

*; Location of observation well and nodes, #; straight canal runs between points, W; water table elevation, 8.0 m above sea level in m, D; depth of water level below ground surface in, :; location of aquifer transmissivity site, A to E

5.3. Design of nodal net work

Since the area and the piezometers are in rectangular shape, a regular grid system is imposed on the farm and is reported in Fig. 5.2. The total number of nodes ($N=84$) so formed are 84 and the number of sides ($NSIDES = 149$) observed are 149.

5.4. Interpolation of data

The interpolation of data at each node involves the plotting of contour maps. The contour maps are drawn for water table depths, water table, ground level, electrical conductivity and hydraulic conductivity with the help of commercially available soft were SURFER and are reported in Figures 5.3 to 5.5. The water table at each nodal point was also interpolated with the help of contours drawn by hand to check for any difference between the two drawn by hand and that drawn by SURFER. The interpolated values along with the coordinate as measured on Figure 5.1 are reported in Table 5.1. On comparing the interpolated data of the water table in column no. 6 and column no. 7 (Table 5.1), the differences can be observed. It is therefore in all subsequent calculations, the interpolated data of the water table by hand are used given in column no. 7.

5.5. Preparation of input files

It involves the preparation of SGMP11.INP, SGMP12A.INP, SGMP12B.INP and SGMP13.INP as per the structure and specified format.

5.5.1. Preparation of input file SGMP11.INP

So for we have decided for the data of N , $NSIDES$, $TMBAS$, $DMTIM$, $DELTA$, $MINOR$, $MAJOR$ and $LIST$. The values of DL , $ERROR$ and $DELQ$ are estimated by trial and error while running the package and will be discussed later. The scale of the map on which the coordinates were measured is $1 \text{ cm} = 100 \text{ m}$ and therefore, the input data to $SCALE$ will be $10,000$ ($1 \text{ cm} = 100 \times 100 \text{ cm}$). The number of iteration is selected as 50 and $LSW1=1$. Since the problem is of unconfined aquifer, the top level TL to which the water can rise is equal to surface level (SL). In the problem the depth of aquifer is not given but the case is of the flat alluvial plane of Haryana. It is therefore, the depth of aquifer is assumed around 100 m . Suppose the depth of aquifer is 120 m instead of 100 m , the error is estimation of hydraulic conductivity will be only of the order of 0.83 percent. Therefore the bottom level (BL) will be equal to $100 - TL$. The prescribed values of UL and LL to which water table is allowed to rise and fall is same as TL and BL .

The specific yield (AS) and mean storage coefficient (ASC) of the aquifer are assumed as 0.2 and 0.001 respectively. The hydraulic conductivity at each nodal point is estimated by using transmissivity data and the relationship used is $T=kb$. Since the transmissivity at one point is $250 \text{ m}^2/\text{day}$, the value of hydraulic conductivity will be as follows;

$$k = (T/b) = 250, \text{m}^2/\text{day} / 100, \text{m} (\text{assumed}) = 2.5, \text{m/day}$$

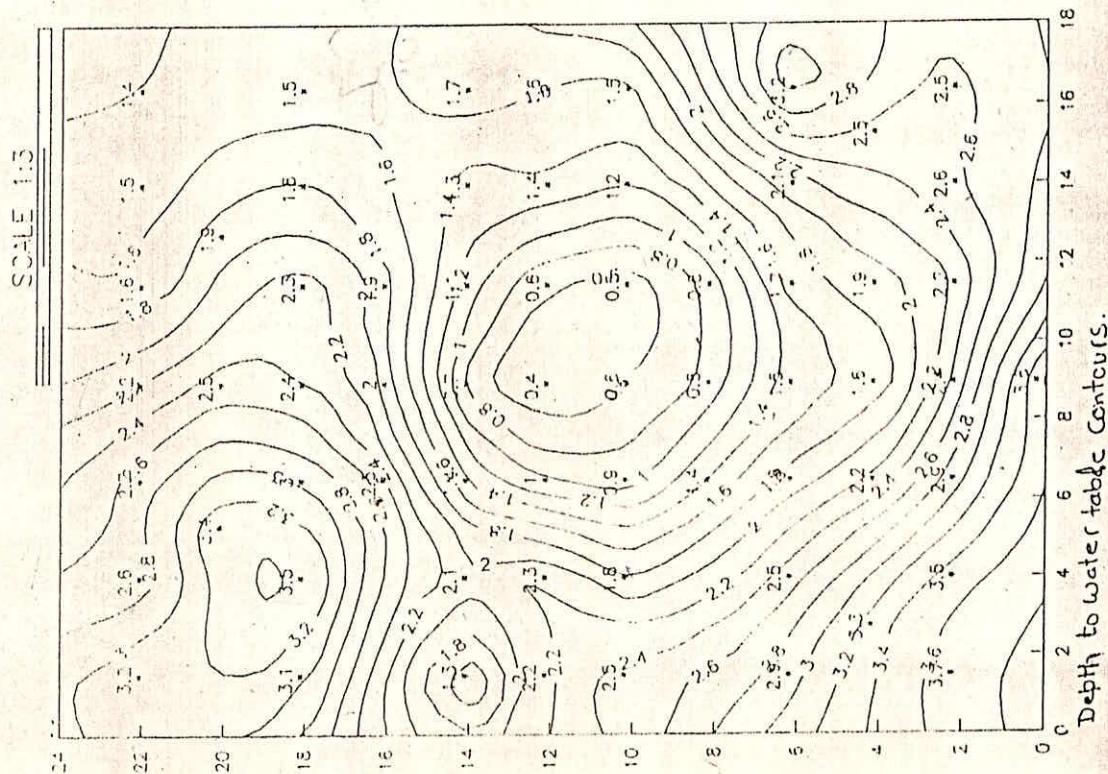
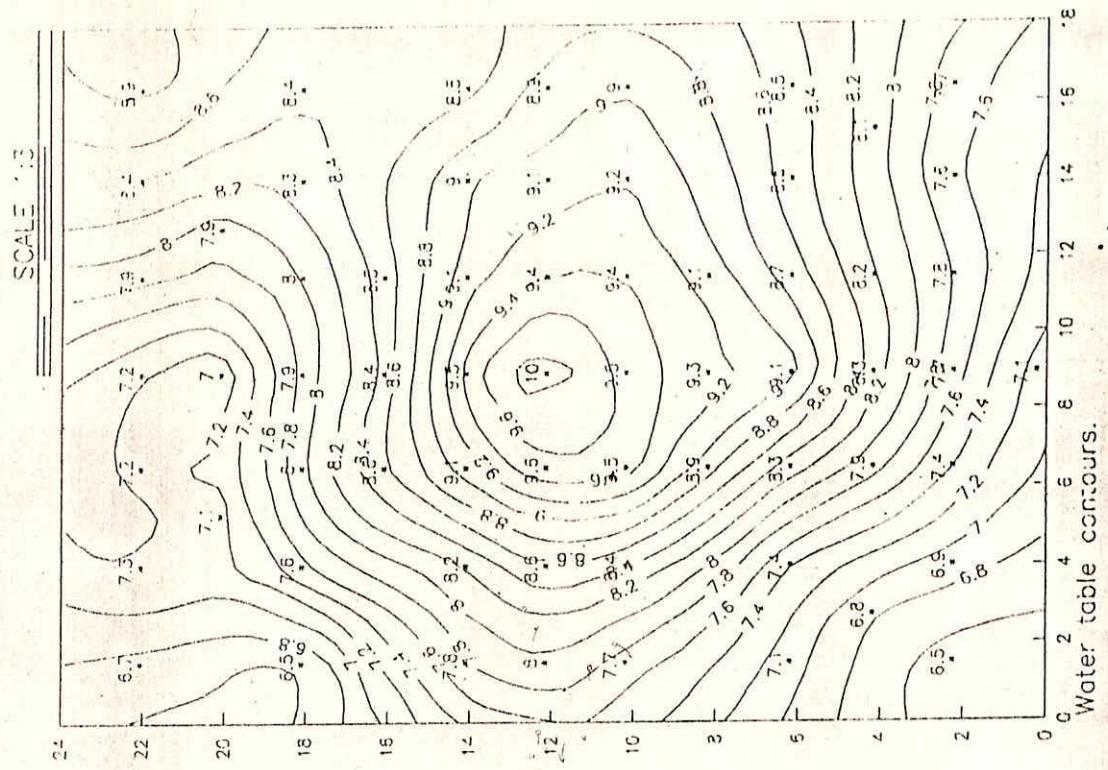


Fig. 5.3: Contour maps of depth to water table and water table.

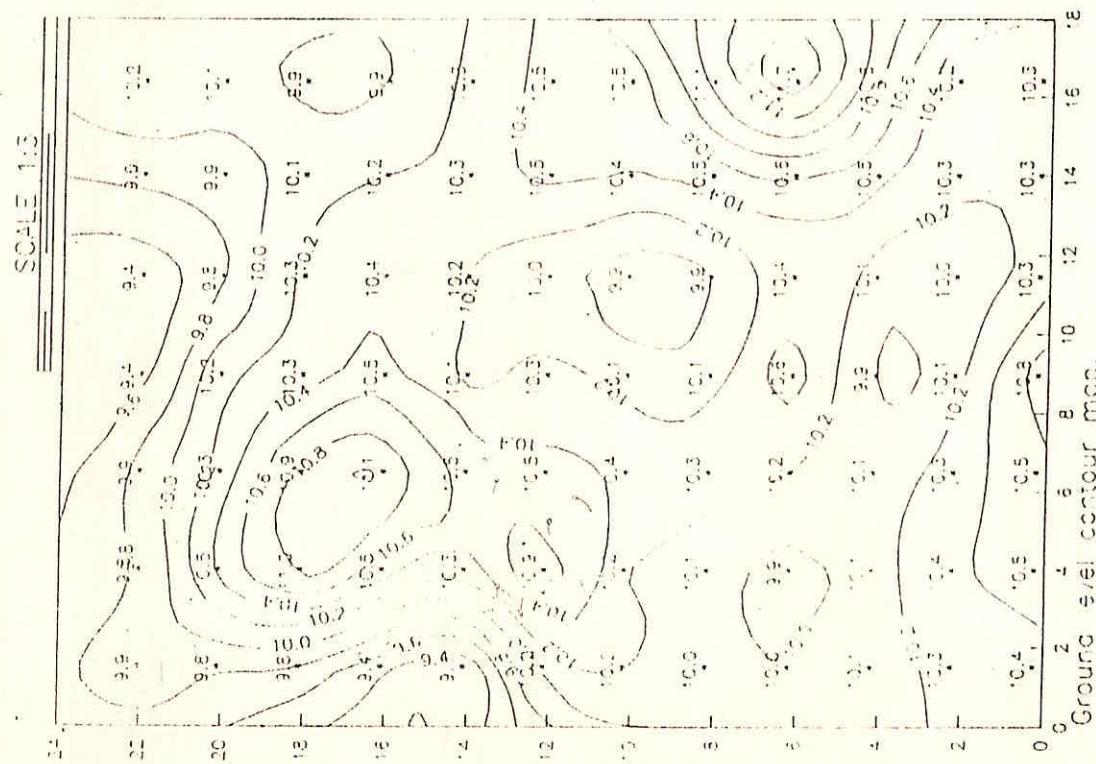
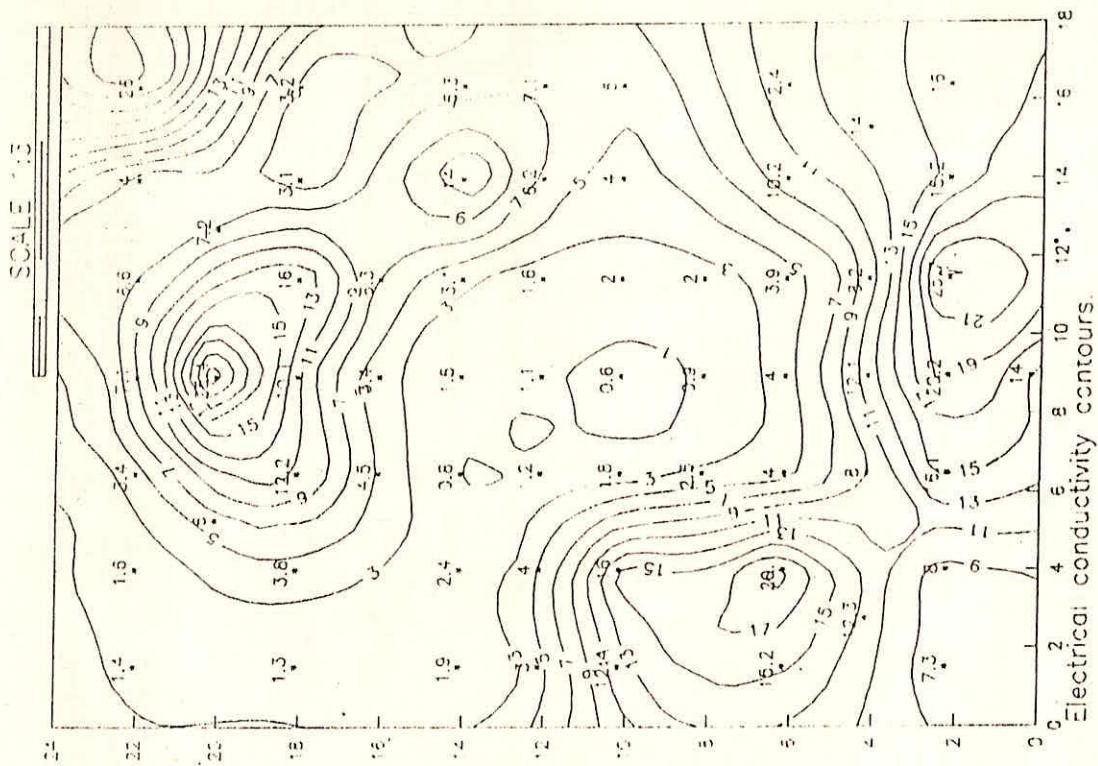


Fig. 5.4: Contour maps of ground level and electrical conductivity.

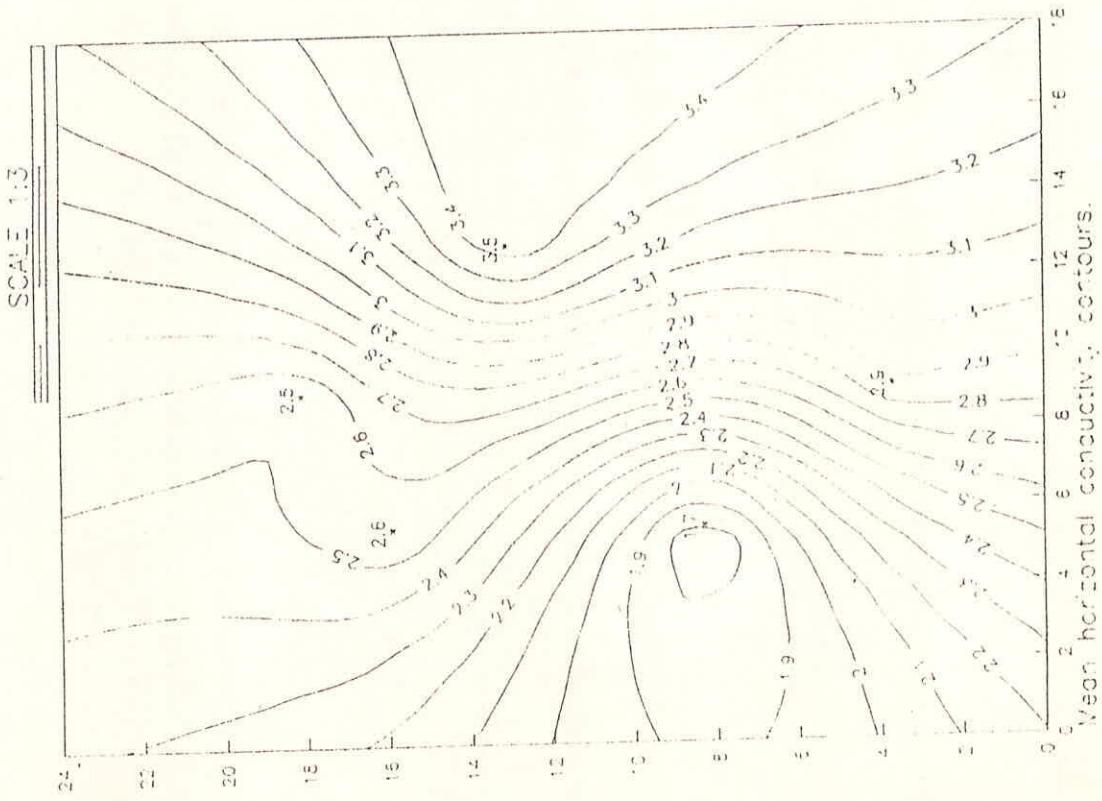


Fig. 5.5: Contour map of hydraulic conductivity.

Table 5.1: Node number its coordinates along with the interpolated values of depth to water table, water table by surfer and hand, ground level and electrical conductivity.

Sl. No.	Node No.	X axis, cm.	Y axis, cm.	Depth to water table, m	Water table by SURFER, m	Water table by HANH, m	Ground level by HAND, m	Elec. condt. mimh/cm
(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
1	1	1.5	22.05	3.2	6.75	6.7	9.9	1.4
2	2	4	22.05	2.6	7.15	7.3	9.9	1.6
3	3	6.5	22.05	2.7	7.18	7.2	9.9	3.4
4	4	8.95	22.05	2.2	7.25	7.2	9.4	7.1
5	5	11.45	22.05	1.6	7.85	7.9	9.5	6.6
6	6	14	22.05	1.5	8.37	8.4	9.9	4
7	7	16.4	22.05	1.4	8.83	8.9	10.3	26
8	8	1.5	20.1	3.15	6.67	6.54	9.69	1.3
9	9	4	20.1	3.3	7.15	7.01	10.31	4
10	10	6.5	20.1	3	7.25	7.07	10.07	11
11	11	8.95	20.1	2.5	7.08	7	9.5	25.4
12	12	11.45	20.1	2.05	7.7	7.55	9.6	13
13	13	14	20.1	1.7	8.2	8.15	9.85	6
14	14	16.4	20.1	1.5	8.57	8.605	10.105	13.2
15	15	1.5	18.1	3.1	6.65	6.5	9.6	1.3
16	16	4	18.1	3.5	7.5	7.6	11.1	3.8
17	17	6.5	18.1	3	7.93	8	11	12.2
18	18	8.95	18.1	2.4	7.87	7.9	10.3	10.1
19	19	11.45	18.1	2.3	8	8	10.3	16
20	20	14	18.1	1.8	8.3	8.3	10.1	3.1
21	21	16.4	18.1	1.5	8.45	8.4	9.9	3.2
22	22	1.5	16.1	2.25	7.2	7.19	9.44	1.3
23	23	4	16.1	2.6	7.9	7.95	10.55	3
24	24	6.5	16.1	2.5	8.55	8.6	11.1	4.5
25	25	8.95	16.1	2	8.5	8.4	10.4	3.4
26	26	11.45	16.1	1.9	8.5	8.5	10.4	6.3
27	27	14	16.1	1.61	8.6	8.61	10.22	8
28	28	16.4	16.1	1.5	8.45	8.3	9.8	5
29	29	1.5	14.1	1.3	7.8	7.8	9.1	1.9
30	30	4	14.1	2.1	8.2	8.2	10.3	2.4
31	31	6.5	14.1	1.4	9.1	9.1	10.5	0.8
32	32	8.95	14.1	0.7	9.4	9.5	10.2	3.1
33	33	11.45	14.1	1.1	9.1	9.1	10.2	1.5
34	34	14	14.1	1.3	8.95	9	10.3	14
35	35	16.4	14.1	1.7	8.57	8.5	10.2	5.3
36	36	1.5	12.15	2.2	7.95	8	10.2	3
37	37	4	12.15	2.3	8.55	8.6	10.9	4
38	38	6.5	12.15	1	9.5	9.6	10.6	1.2
39	39	8.95	12.15	0.4	9.9	10	10.4	1.1
40	40	11.45	12.15	0.6	9.4	9.4	10	1.6
41	41	14	12.15	1.4	9.1	9.1	10.5	6.2
42	42	16.4	12.15	1.6	8.86	8.9	10.5	7.1
43	43	1.5	10.2	2.5	7.73	7.7	10.2	12.4
44	44	4	10.2	1.8	8.4	8.4	10.2	16
45	45	6.5	10.2	0.9	9.45	9.5	10.4	1.8
46	46	8.95	10.2	0.6	9.5	9.8	10.4	0.6

Contd.

(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
47	47	11.45	10.2	0.5	9.37	9.4	9.9	2
48	48	14	10.2	1.2	9.17	9.2	10.4	4
49	49	16.4	10.2	1.5	8.96	9	10.5	5
50	50	1.5	8.2	2.6	7.44	7.39	9.99	15.2
51	51	4	8.2	2.1	8	8	10.1	15
52	52	6.5	8.2	1.4	8.9	8.9	10.3	2.5
53	53	8.95	8.2	0.8	9.27	9.6	10.4	0.9
54	54	11.45	8.2	0.8	9.1	9.1	9.9	2
55	55	14	8.2	1.6	8.9	8.9	10.5	6
56	56	16.4	8.2	2.3	8.77	8.9	11.2	8.2
57	57	1.5	6.2	2.9	7.1	7.1	10	15.2
58	58	4	6.2	2.5	7.4	7.4	9.9	20.1
59	59	6.5	6.2	1.9	8.3	8.3	10.2	4
60	60	8.95	6.2	1.5	9.05	9.1	10.6	4
61	61	11.45	6.2	1.7	8.7	8.7	10.4	3.9
62	62	14	6.2	2.1	8.5	8.5	10.6	10.2
53	53	16.4	6.2	3.2	8.5	8.5	11.7	12.4
64	64	1.5	4.2	3.36	6.75	6.8	10.16	11.5
65	65	4	4.2	3	7.1	7.194	10.2	12
66	66	6.5	4.2	2.2	7.87	7.9	10.1	8
67	67	8.95	4.2	1.6	8.3	8.3	9.9	12.1
68	68	11.45	4.2	1.9	8.2	8.2	10.1	8.2
69	69	14	4.2	2.35	8.1	8.05	10.4	12.6
70	70	16.4	4.2	2.7	8.1	8.06	10.76	14.5
71	71	1.5	2.25	3.7	6.55	6.6	10.3	7.3
72	72	4	2.25	3.5	6.9	6.9	10.4	8
73	73	6.5	2.25	2.9	7.4	7.4	10.3	16.1
74	74	8.95	2.25	2.4	7.7	7.7	10.1	20.2
75	75	11.45	2.25	2.6	7.65	7.6	10.2	15.2
76	76	14	2.25	2.6	7.7	7.7	10.2	16
77	77	16.4	2.25	2.5	6.5	6.36	10.21	6
78	78	1.5	0.25	3.85	6.75	6.71	10.41	9.5
79	79	4	0.25	3.7	7.03	7	10.5	13.5
80	80	6.5	0.25	3.5	7.15	7.1	10.7	14
81	81	8.95	0.25	3.6	7.35	7.25	10.2	20
82	82	11.45	0.25	2.95	7.4	7.14	9.99	17.2
83	83	14	0.25	2.85	7.5	7.25	10.03	18.5
84	84	16.4	0.25	2.78				

The same method is used to calculate hydraulic conductivity at other points. With these values the contours were drawn (Fig. 5.5) and mean horizontal hydraulic conductivity of aquifer at each node side is interpolated. The data of SGMP11.inp is shown in Appendix A.

5.5.2. Preparation of input file SGMP12A.INP

LSW3, LSW4 and LSW5 are taken equal to one and LSW6 may be zero or one depending the package running in normal or inverse mode. The initial water table and the water table at time step 1 are the interpolated water table data. The input to SGMP12A.INP is reported in Appendix A.

5.5.3. Preparation of input file SGMP12B.INP

The file contains the values of net recharge. Since the

problem does not give any value to net recharge, the net recharge is taken as zero at each nodal point. The data of SGMP12B.INP is reported in Appendix A.

5.5.4. Preparation of input file SGMP13.INP

This file contain the input data to have a desired format of output data and the data of SGMP13.INP is reported in Appendix A.

5.6. Package output in different runs

The package SGMP11 is operated in different modes to have different outputs and to understand the working of the package. The running of package in different modes is described in subsequent sections.

5.6.1. Normal mode run

Running the package in normal mode requires the values of LSW2=2 and LSW6=0. Running the package in this mode describes about the behaviour of the aquifer i.e. whether the water table in the aquifer is rising or falling. While running the package, the values of DL, DELQ and ERROR were selected as 0.02, 0.001 and 0.001 respectively. The value of DL=0.02 means, the calculated heads will be adjusted within 1.0 cm to the prescribed head. The value of DELQ=0.001, means that at each iteration $0.001 \times 10^6 \text{ m}^3$ of water will be added or subtracted in water balance. The value of ERROR=.001, means that the water balance at each node will be within 0.1 percent.

A positive value of net change in storage, in SGMP13.OUT water balance component, indicates that water level in the aquifer has lowered. It has happened only because the net recharge to the aquifer is prescribed as zero in SGMP12B.INP, however there would have been some recharge to the area. In a normal run the superiteration does not work and it will be zero at each time step. In package the, number of iteration works to adjust the water table and its value ranges within 2 to 50 and finally should reach to 2. The output files (SGMP11.OUT, SGMP12.OUT and SGMP13.OUT) are reported in Appendix B.

5.6.2. Inverse mode run

Running the package in inverse mode requires the values of LSW2=2 and LSW6=1. The package in this mode describes about the artificial flow at each node for which the water table elevations are calculated keeping the initial head same as final, which is the assumption in the problem. While running the package, the values of DL, DELQ and ERROR remains same as prescribed in normal run or slightly can be modified. In this run the values are taken same and also in subsequent runs no change is made.

In this run the change in head will be zero and an artificial recharge will be introduced at each node to keep the head same with in one percent error. Both iteration and superiteration works in this run. The superiteration works to adjust the water balance and finally comes to zero from 50. The number of iteration works to adjust the water table and its value should work within 2 to 50 and finally should reach to 2. The water balance component of output file SGMP13.OUT is reported in Appendix C.

5.6.3. Runes with drains in aquifer

In this run the condition is imposed to obtain the results by providing a drain at 1.0 m and 1.5 m depths in irrigated form. To impose this condition and to run the package the following changes are required in SGMP11.INP.

a) The artificial recharge obtained by inverse mode run in section 5.6.2 is considered as the actual net recharge to the irrigated form and the output of net recharge becomes the input of net recharge in SGMP11.INP.

b) Now the package is operated in normal mode by changing LSW6=0. Under this condition the artificial recharge at each node in SGMP13.OUT should be zero and there should be no change in storage.

c) To impose the condition of placing the drain at 1.0 and 1.5 meters depth in irrigated form, the prescribed value of UL will be as follows; $UL = SL - 1.0$ or $UL = SL - 1.5$ to all the nodes within the irrigated form. To make the UL and LL values effective the value of LSW2 will be equal to 1 in SGMP11.INP. On running the package the water balance component of SGMP13.OUT will result in artificial net recharge at each node where the drainage is required. The results of water balance component for both the cases are reported in Appendix D.

5.7. Discussion

The nodal and average net recharge to the irrigated area is reported in Table 5.2. The net recharge is calculated in mm/day by multiplying the net recharge by 10^9 and dividing it by area in m^2 and 30.5 number of days in a month.

The following conclusions can be drawn by this exercise.

a) The use of SURFER soft were introduces an error in plotting the contours. This may result an error in estimation of net recharge, especially when the hydraulic gradient in the area is high.

b) The average net recharge to the irrigated area, estimated by this package is based on bottom to top approach and is found as 2.18 mm/day. However, when it is calculated by traditional method it is 2.0 mm/day.

c) Since the average net recharge to the irrigated area is only 2.18 mm/day, there is no need to provide any drainage system to the area.

d) Even if the area has to be provided with drainage system to bring down the water level below 1.0 m to the land surface, the drainage will be designed considering 2.18 mm/day ne recharge. However, when the simulation of the drainage system is done by this package, which has basis of estimation by bottom to top approach, estimates the drainage should be designed considering 0.44 mm/day. This yields that the traditional method over estimates the drainage requirement and thus drain design. The similar results were drawn when the simulation was done for a drainage system to bring down the water level below 1.5 m. For this case the drainage requirement was only 0.99 mm/day.

Table 5.2: Estimated net recharge in irrigated area and drainage requirement for two depths at 1.0 and 1.5 m depths along with the nodal area.

Sl. No.	Node No.	Nodal area	Net Recharge@ no drain		Net Recharge# drain at 1.0 m		Net Recharge\$ drain at 1.5 m	
			m ²	mcm/month	cm/day	mcm/month	cm/day	mcm/month
1	17	49500	0.0060	3.97	0.0000	0.00	0.0000	0.00
2	18	49500	0.0026	1.72	0.0000	0.00	0.0000	0.00
3	19	50500	-0.0022	-1.42	0.0000	0.00	0.0000	0.00
4	24	49500	0.0062	4.10	0.0000	0.00	0.0000	0.00
5	25	49500	-0.0086	-5.69	0.0000	0.00	0.0000	0.00
6	26	50500	-0.0018	-1.16	0.0000	0.00	0.0000	0.00
7	31	48881	0.0026	1.74	0.0000	0.00	0.0000	0.00
8	32	48881	0.0116	7.78	0.0000	0.00	-0.0038	-2.54
9	33	49868	0.0012	0.78	0.0000	0.00	0.0000	0.00
10	38	48262	0.0082	5.57	0.0000	0.00	0.0000	0.00
11	39	48262	0.0142	9.64	-0.0080	-5.43	-0.0098	-6.65
12	40	49237	0.0018	1.19	-0.0008	-0.53	-0.0052	-3.46
13	45	48881	0.0072	4.82	0.0000	0.00	-0.0008	-0.53
14	46	48881	0.0044	2.95	0.0000	0.00	-0.0008	-0.53
15	47	49868	0.0024	1.57	-0.0070	-4.60	-0.0102	-6.70
16	52	49500	0.0000	0.00	0.0000	0.00	0.0000	0.00
17	53	49500	0.0104	6.88	0.0000	0.00	-0.0022	-1.45
18	54	50500	-0.0008	-0.51	0.0000	0.00	-0.0030	-1.94
19	59	49500	-0.0018	-1.19	0.0000	0.00	0.0000	0.00
20	60	49500	0.0106	7.02	0.0000	0.00	0.0000	0.00
21	61	50500	0.0000	0.00	0.0000	0.00	0.0000	0.00
22	66	48881	0.0024	1.60	0.0000	0.00	0.0000	0.00
23	67	48881	0.0018	1.20	0.0000	0.00	0.0000	0.00
24	68	49868	-0.0004	-0.26	0.0000	0.00	0.0000	0.00
Total, mm/day				52.30		-10.56		-23.79
Average of 24 nodes, mm/day				2.18		-0.44		-0.99

@: Net recharge when no drainage is provided in irrigated area.

#: Net recharge when drainage is provided at 1.0 m depth.

\$: Net recharge when drainage is provided at 1.5 m depth.

6. CASE STUDY II

6.1. A Case study of Bulandshehar District of Uttar Pradesh

The study area is the Bulandshar District of the Uttar Pradesh. This area is bounded by river Ganga and Yamuna on its two sides and falls in the command area of Upper Ganga Canal. The area has been provided by the canal irrigation system by three main canals as Mat, Upper Ganga Canal and Anupshar branch. The area has two drain rivers called Karwan and Kali Nadi. The study area has been shown in Figure 6.1, along with the available related details. Regarding availability of data, it will be discussed as and when it will come in picture. In order to prepare the input files the same steps are followed as discussed in the case study I of Hansi form in Haryana.

6.2. Time discretization

Since the groundwater data available is before and after the monsoon in a year i.e. for the month of June and October and the study period selected is of two years. Based on this information the following time discretization is done.

TMBAS = Month
DMTIM = 30.5
DELTA = 1
MINOR = 1
MAJOR = 2
LIST = 2

The time discretization should follow following relations:

$$\text{Total simulation period} = \text{DELTA} * \text{MINOR} * \text{MAJOR} * \text{LIST}$$
$$= 1 * 1 * 2 * 2$$

4 steps

6.3. Design of nodel net work

A regular grid system is imposed on the area such that a node should fall in the middle of river at each side of the area. The grid system is shown in Figure 6.2. It is done keeping in mind that the water balance problem can be extended and the river elevations can also be considered when needed. The only drawback with this grid system is that the nodel area could not be representative of the Block area. Therefore, a irregular grid system as reported in Figure 6.3 is designed for the area such that the nodal area should cover a Block or a part of a Block. This design has an advantage that the nodel area can be a representative of the Block area and the Block average data can be an input to that node. With the irregular grid system, the total number of nodes (N) comes to be 100 and number of sides (NSIDES) as 263.

6.4. Interpolation of data

The contour maps are drawn with the help of SURFER for water table of June 76, October 76, June 77, October 77 and June 78 with the available data and the contours are shown in Figure 6.4 to 6.8. The contour plots are also drawn for aquifer thickness, specific yield and ground level and are reported in Figure 6.9 to 6.11. The interpolated vale at each nodel point is reported in Table 6.1.

With the interpolated data the contour plots are again drawn to check for any unavoidable error in interpolation and feeding the data. The contour plots with the interpolated data is reported in Figure 6.12 to 6.19

6.5. Preparation of input files

It involves the preparation of SGMP11.INP, SGMP12A.INP, SGMP12B.INP and SGMP13.INP as per the structure and specified format.

6.5.1. Preparation of input file SGMP11.INP

So far we have decided for the data of N, NSIDES, TMBAS, DMTIM, DELTA, MINOR, MAJOR and LIST. The values of DL, ERROR and DELQ are estimated by trial and error while running the package and will be discussed later. The scale of the map on which the coordinates were measured is 1 inch = 4 mile and therefore, the input data to SCALE will be 253439 ($1 \text{ cm} = 4 \times 1.609 \times 10^6 / 2.539 = 253439 \text{ cm}$). Since the problem is of unsteady state case, the water table changing with time, the number of iteration is selected as 250 and LSW1=1. The aquifer is unconfined, the top level TL to which the water can rise is equal to surface level (SL). The depth of aquifer at each node is taken from the interpolated data.

The specific yield (AS) has been taken from interpolated data and mean storage coefficient (ASC) of the aquifer is as 0.001 because of unavailability of data. The hydraulic conductivity at each nodal point is also assumed as 3 since the transmissivity data is not available. The input file is reported in Appendix E.

6.5.2. Preparation of input file SGMP12A.INP

LSW3, LSW4 and LSW5 are taken equal to 3, 2, 2 respectively and LSW6 may be zero or one depending the package running in normal or inverse mode. The initial water table and the water table at each time step are the interpolated water table data in October and June. The input to SGMP12A.INP is reported in Appendix E.

6.5.3. Preparation of input file SGMP12B.INP

The file contains the values of net recharge. Since the problem does not give any value to net recharge, the net recharge is taken as zero at each nodal point. The data of SGMP12B.INP is reported in Appendix E.

6.5.4. Preparation of input file SGMP13.INP

This file contain the input data to have a desired format of output data and the data of SGMP13.INP is reported in Appendix E.

6.6. Package output in different runs

The package SGMP11 is operated in different modes to have different out puts and to find out the net recharge in monsoon and non-monsoon period. The running of package in different modes is described in subsequent sections.

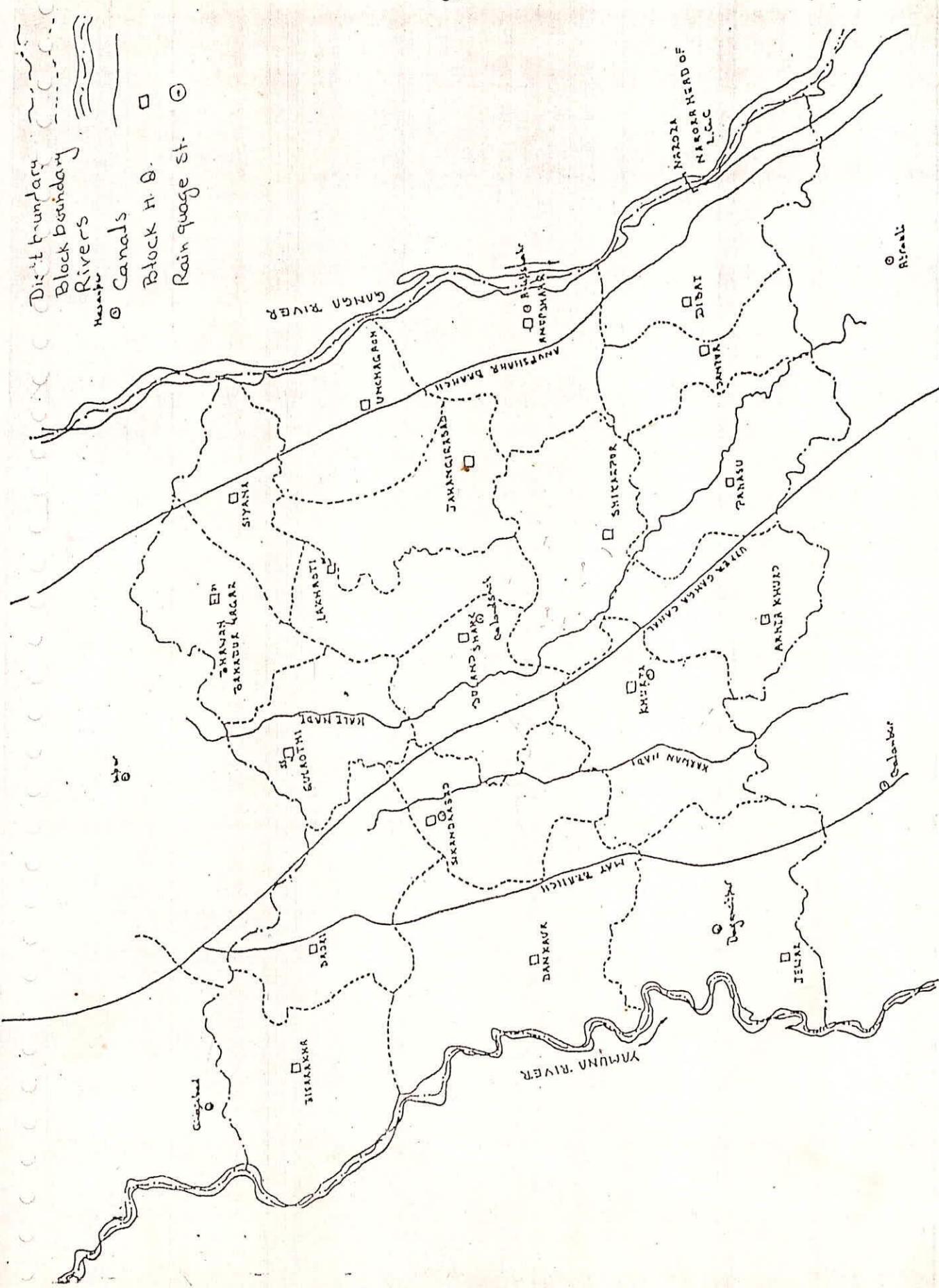
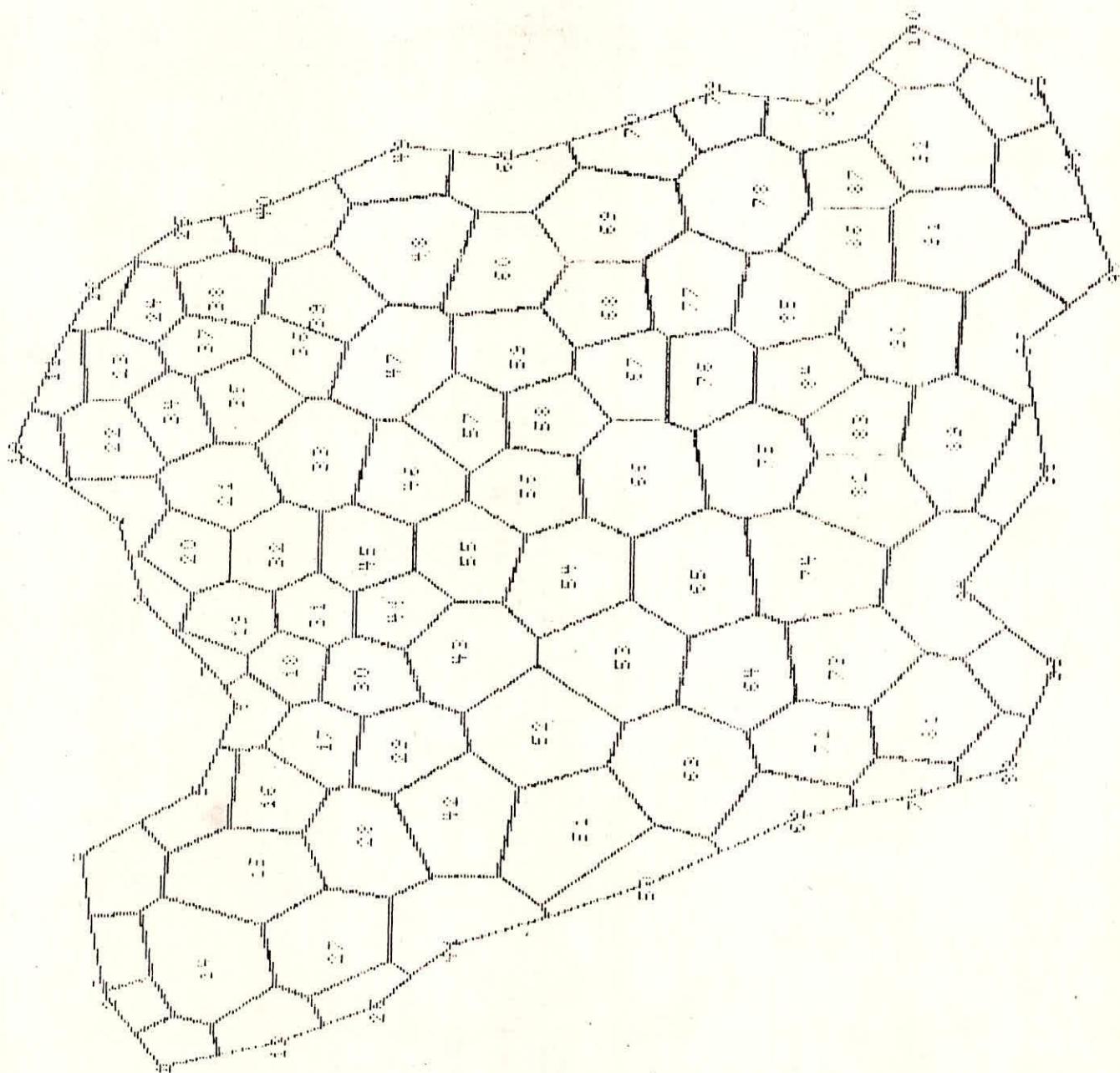


Fig. 6.1: Bulandshahr District of Uttar Pradesh, the study area.



Fig. 6.2: Regular grid system on the study area.



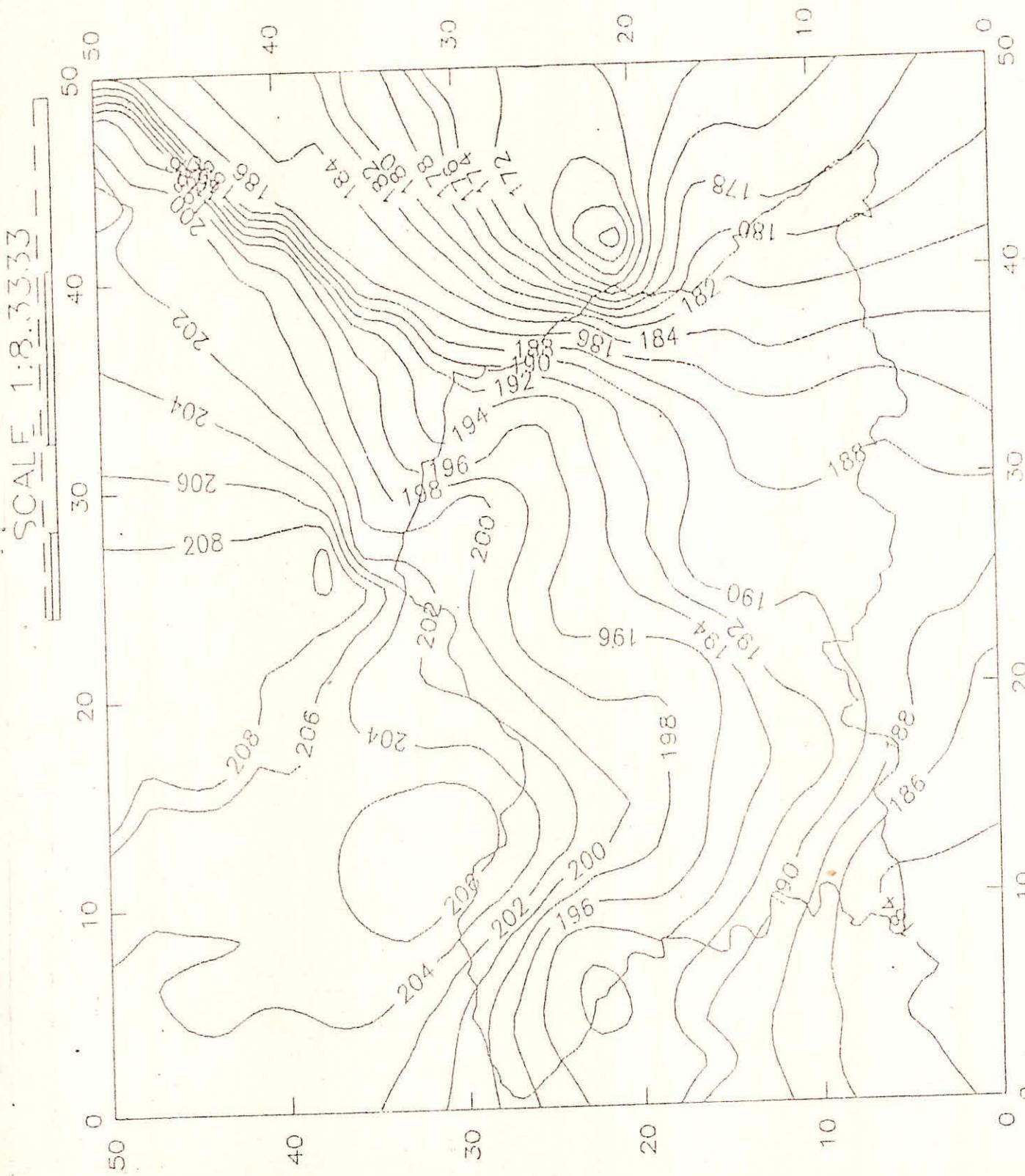


Fig. 6.4: Contour map of water table elevation in June 76, m.

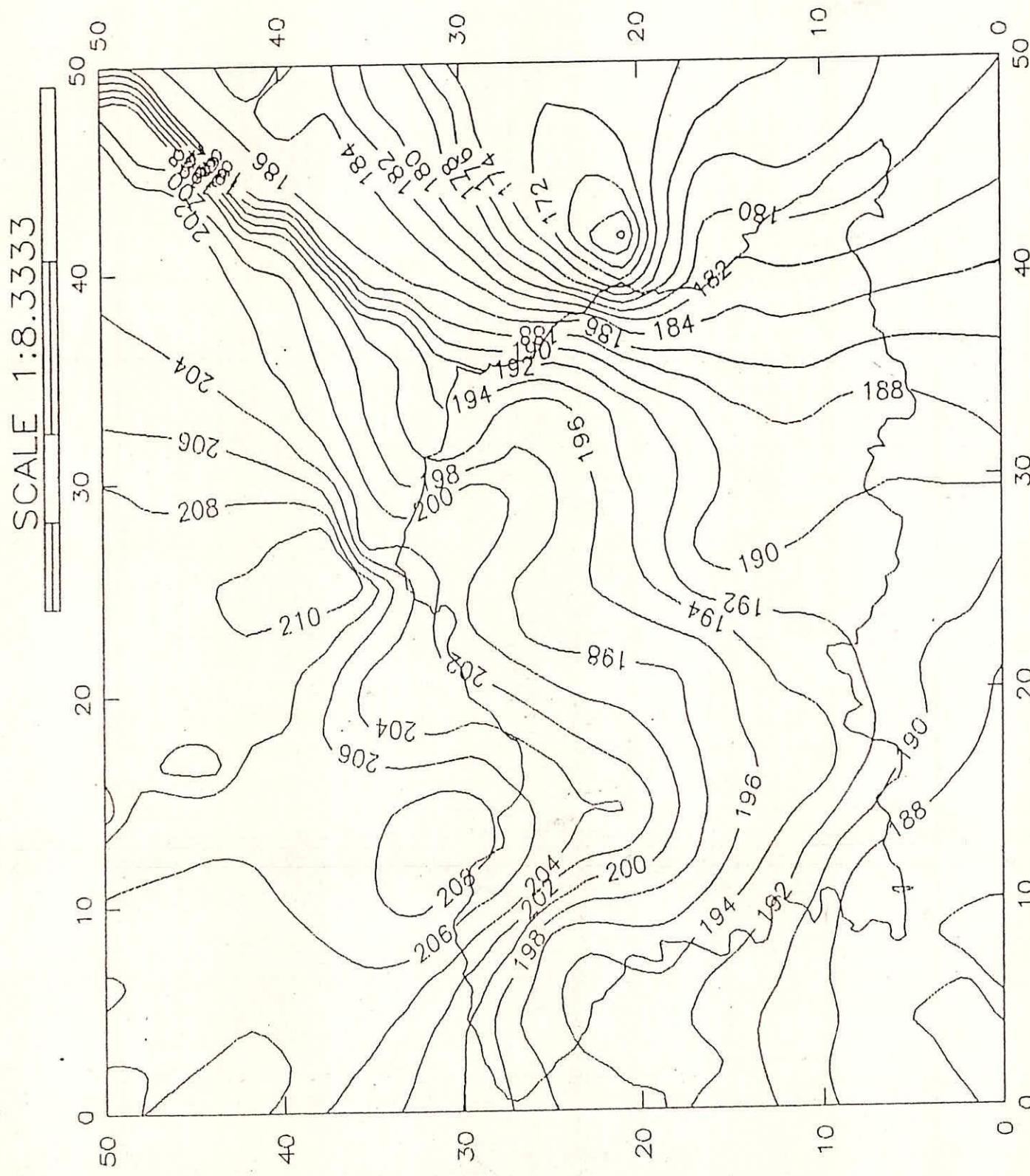
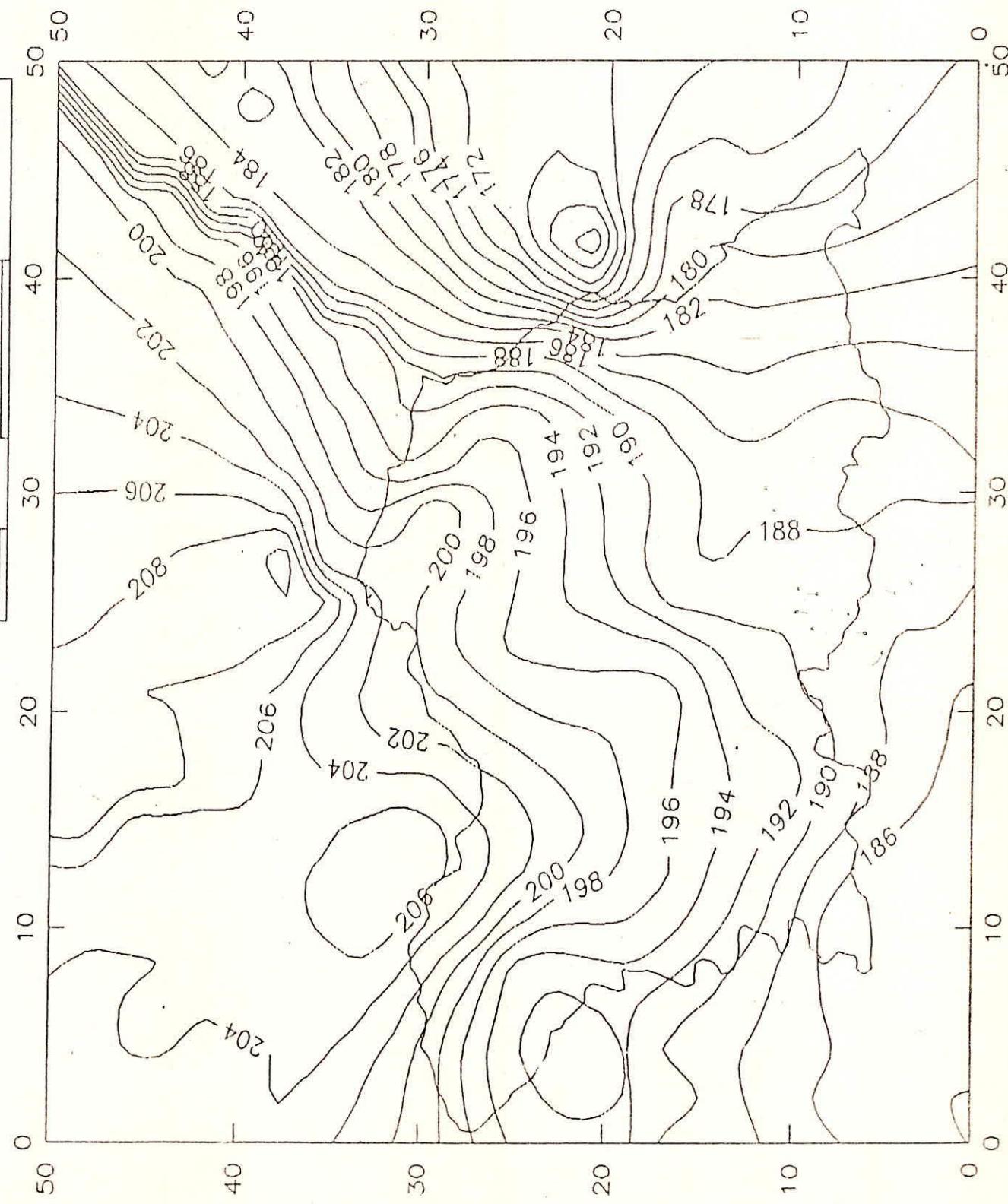


Fig. 6.5 : Contour map of water table elevation in October 76, m.

SCALE 1:8.3333



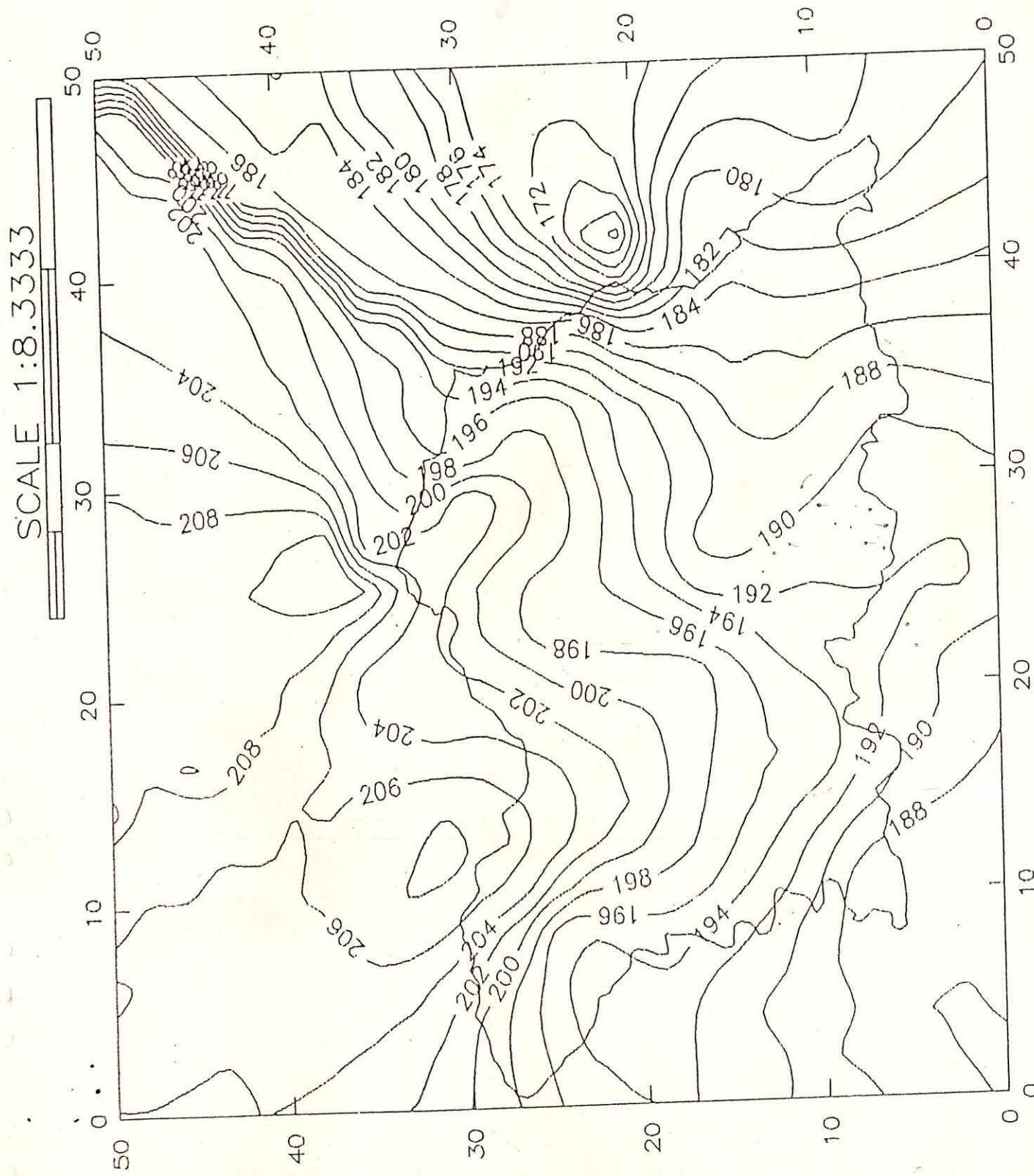


Fig. 6.7 : Contour map of water table elevation in October 77, m.

SCALE 1:8.3333

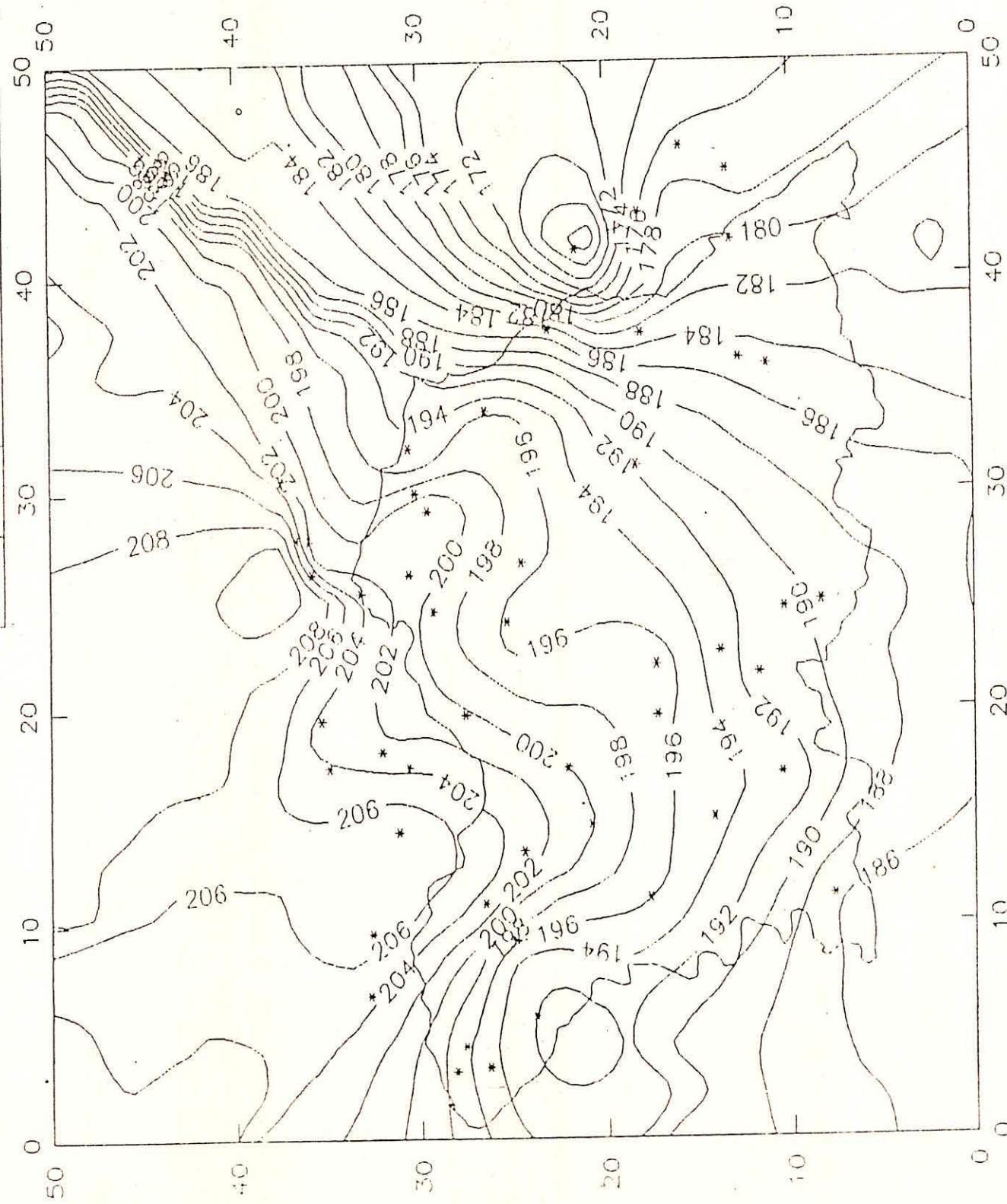


FIG. 6.B : Contour map of water table elevation in June 78, m.

SCALE 1:8.33333

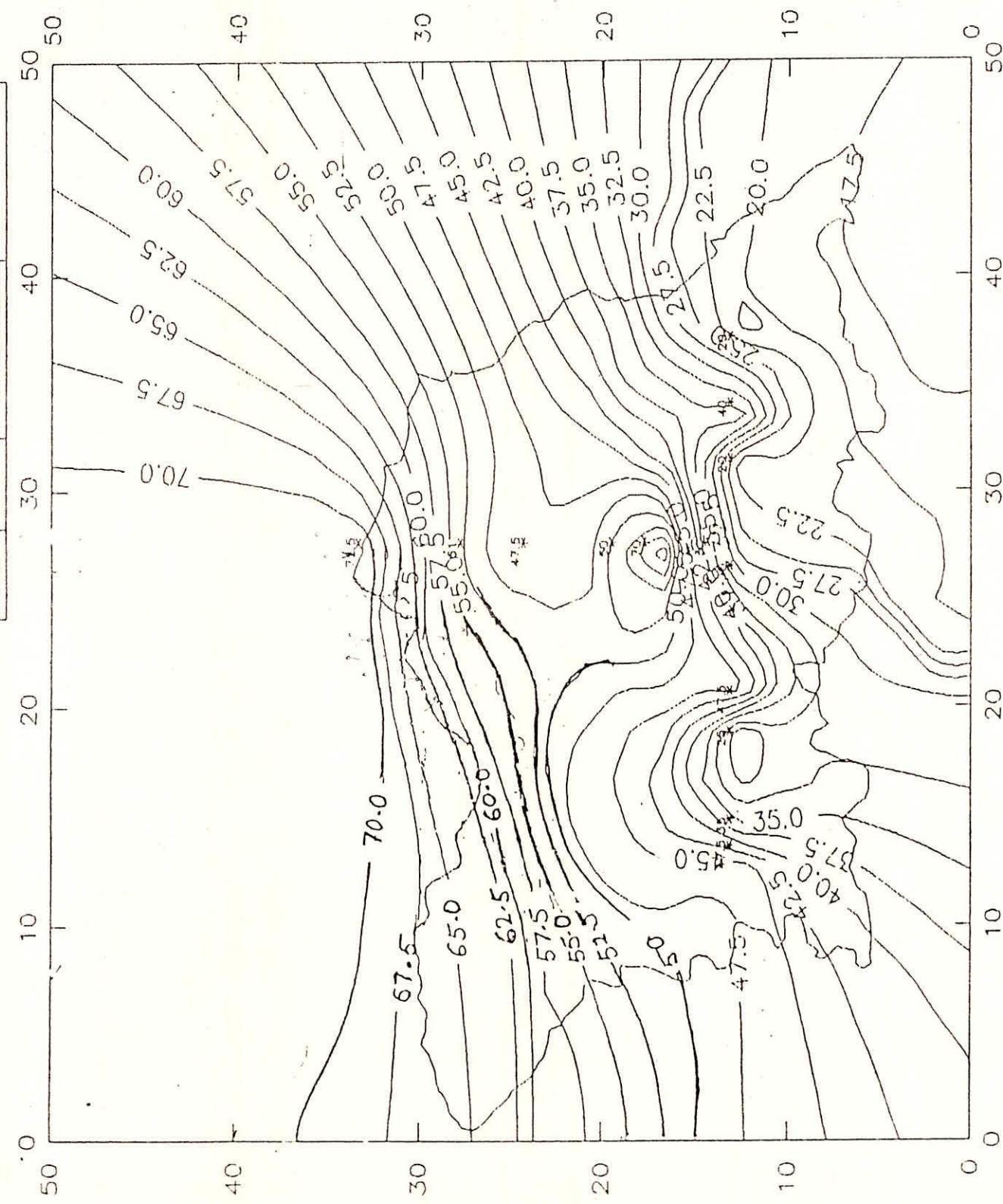


Fig. 6.9: Contour map of the aquifer thickness, m.

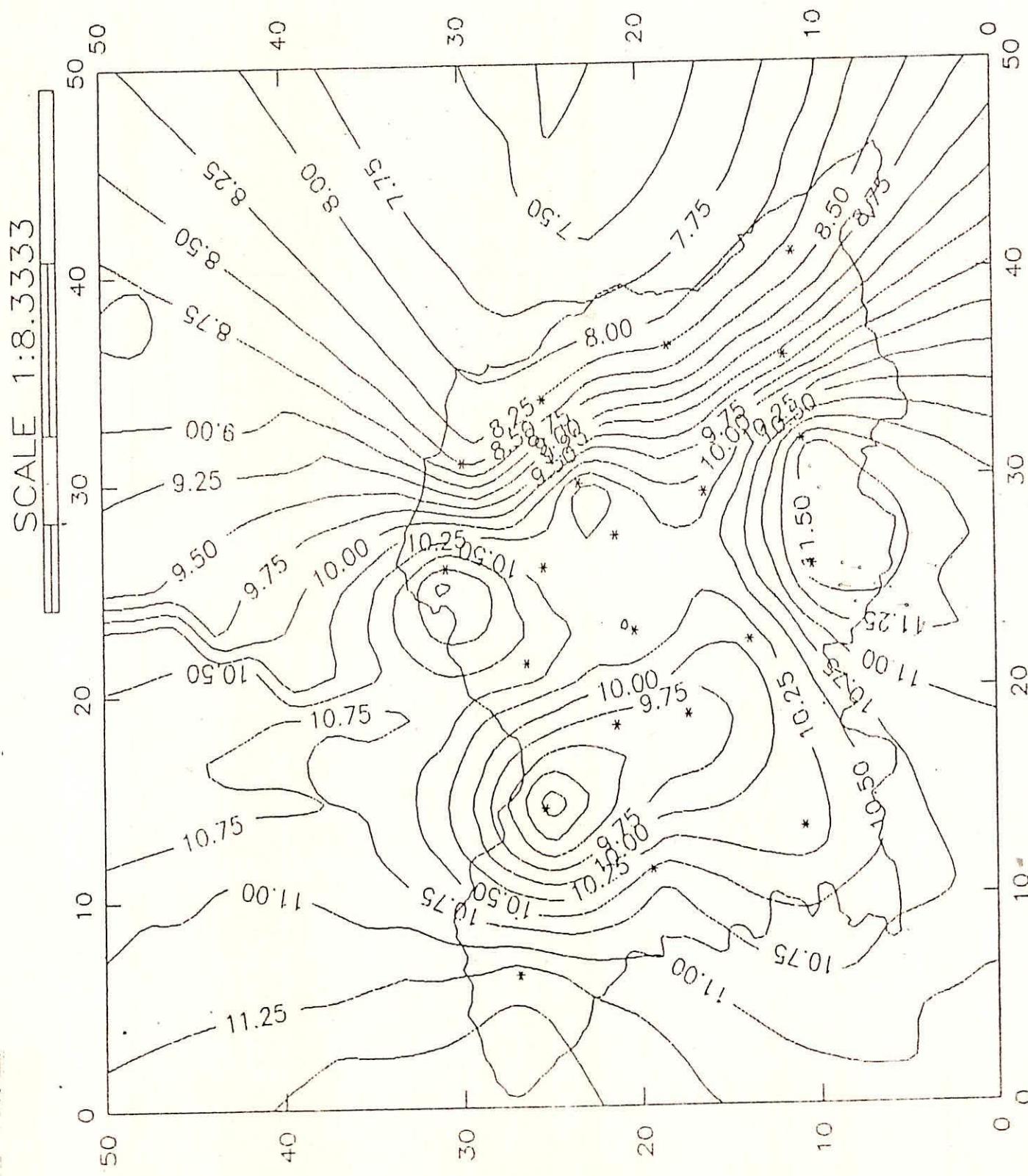


Fig. 6.10: Contour map of Block average specific yield, %.

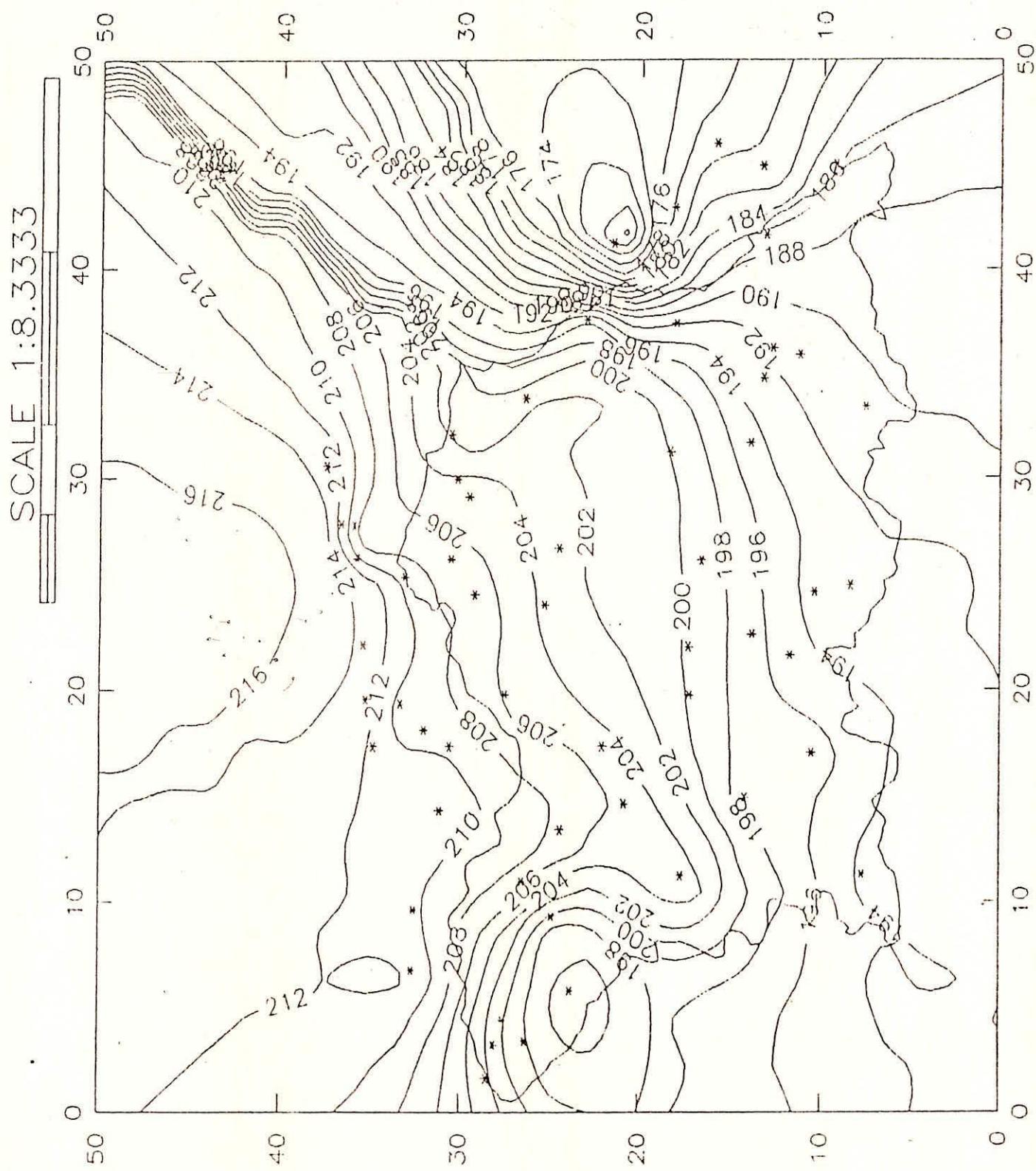


Fig. 6.11: Contour map of surface level, m.

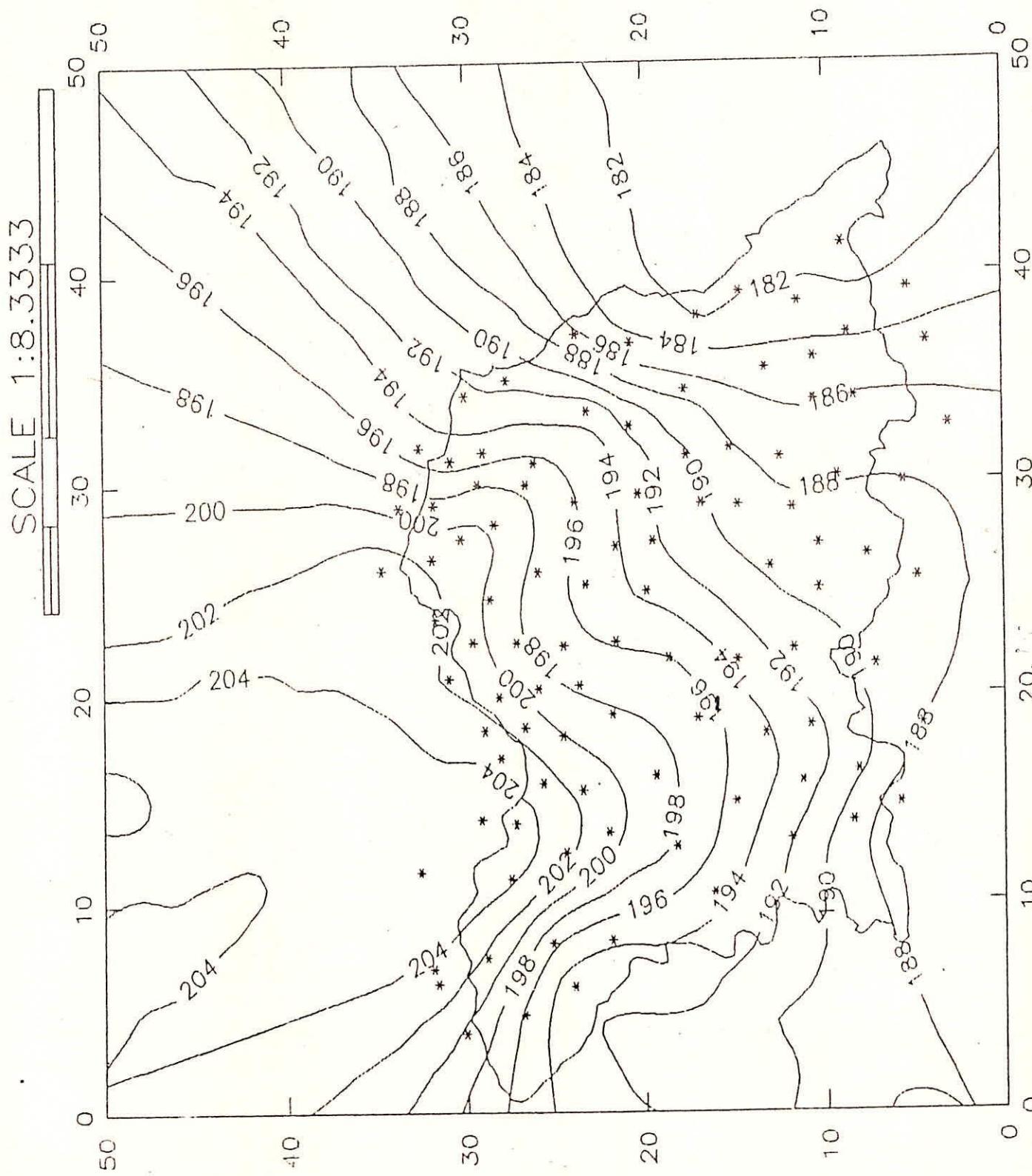


Fig. 6.12 : Contour map of water table elevation with interpolated

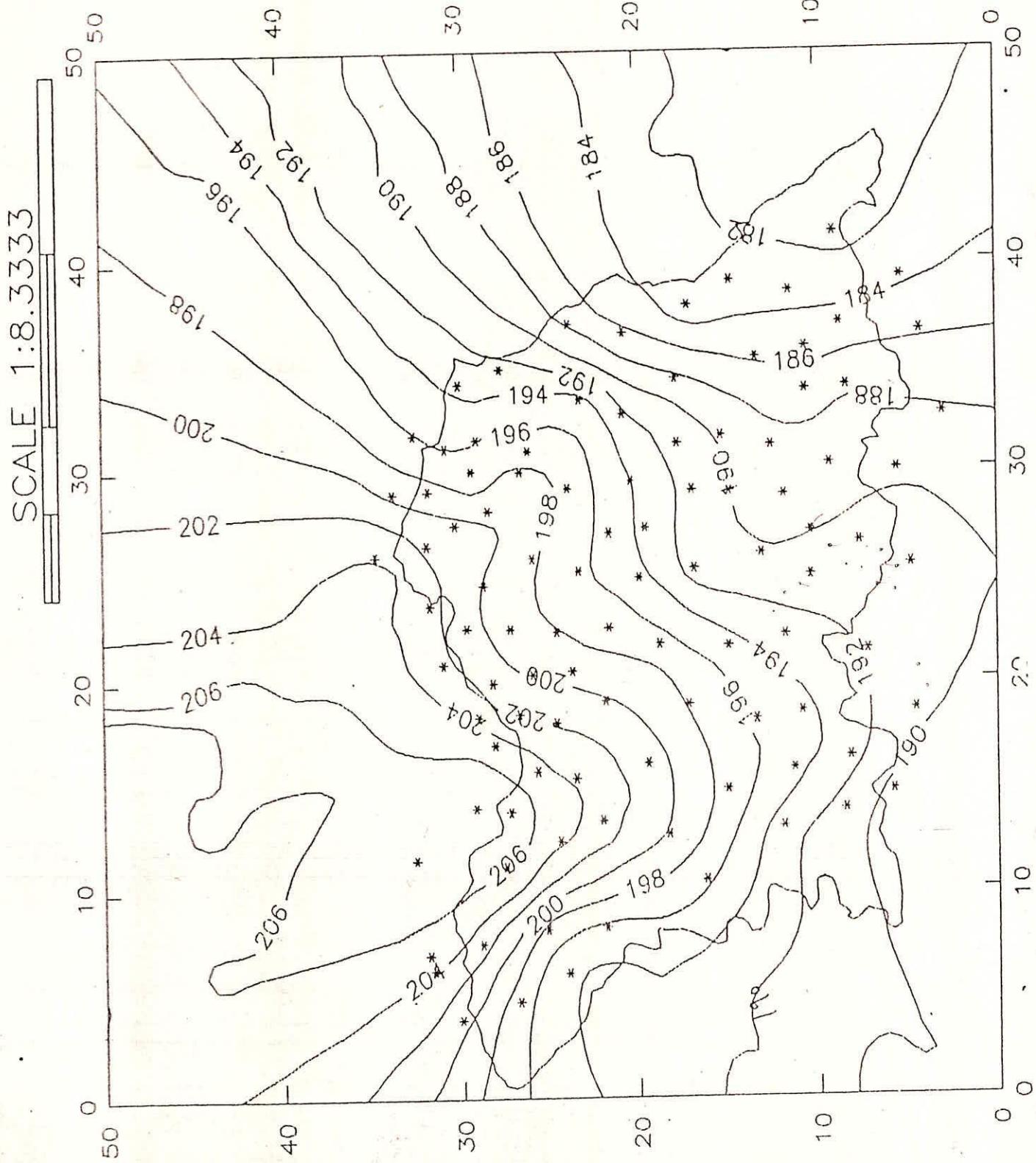


Fig. 6.13 : Contour map of water table elevation with interpolated values in October 76.

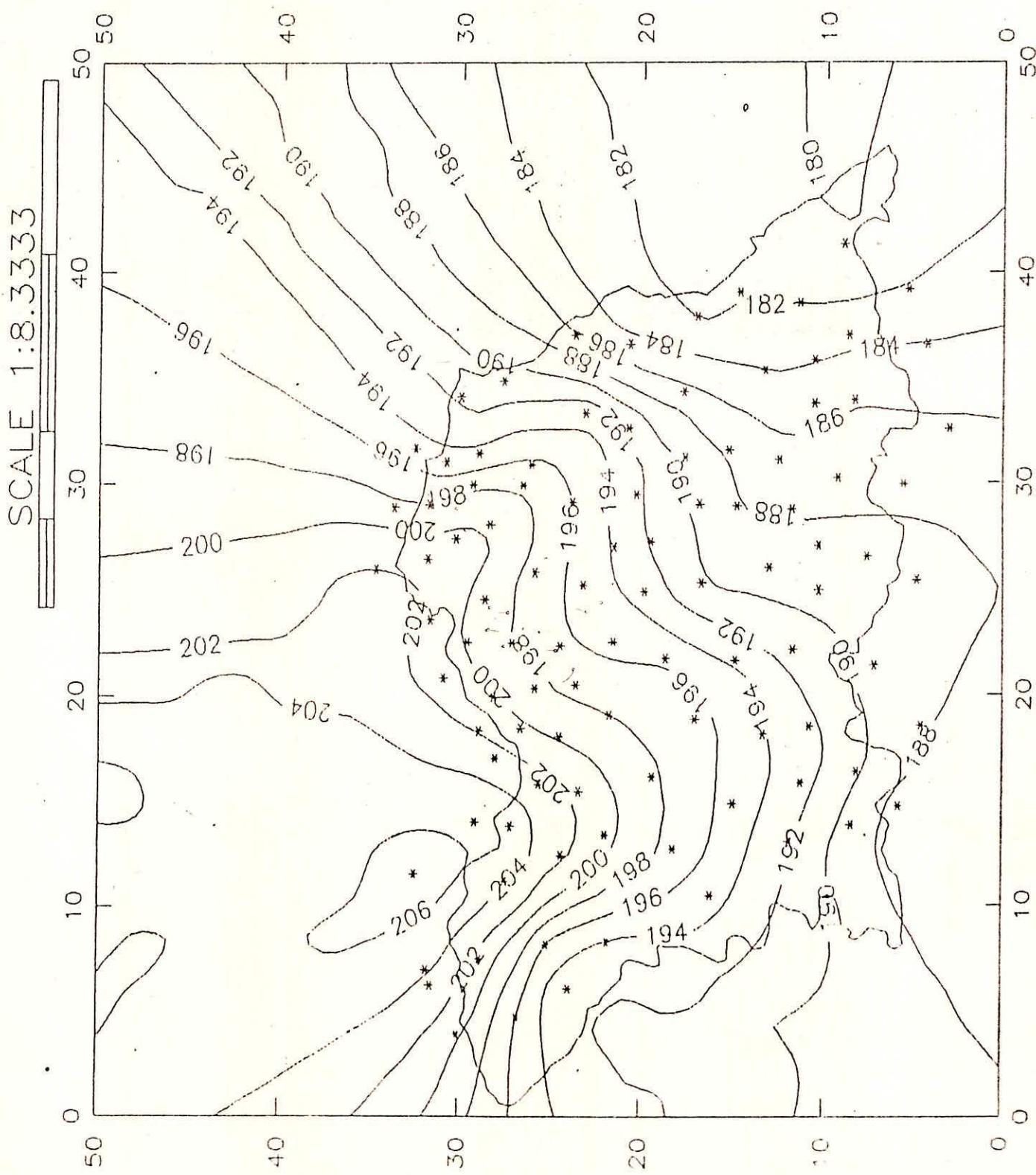


Fig. 6.14 : Contour map of water table elevation with interpolated values in June 77.

SCALE 1:8.3333

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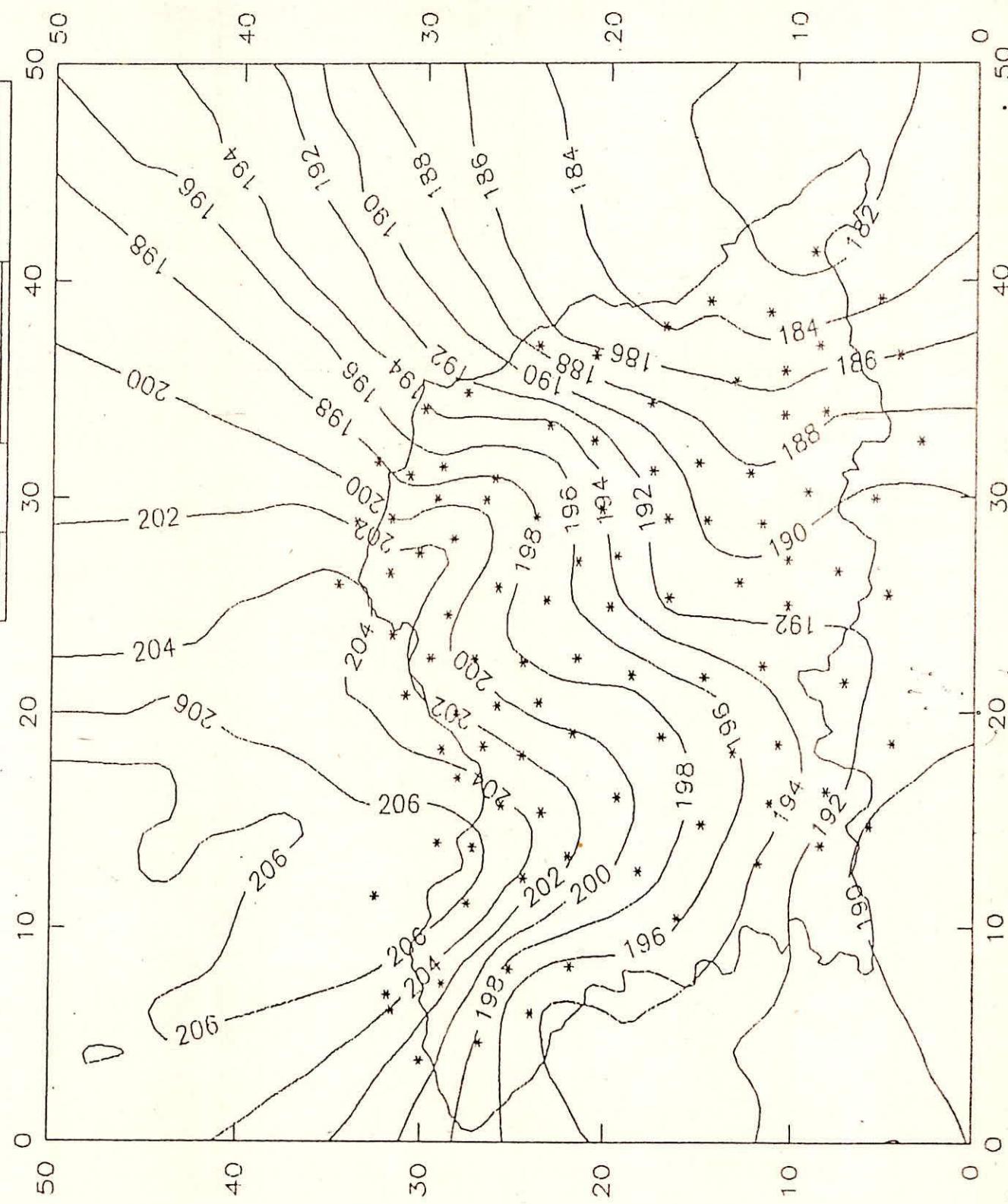
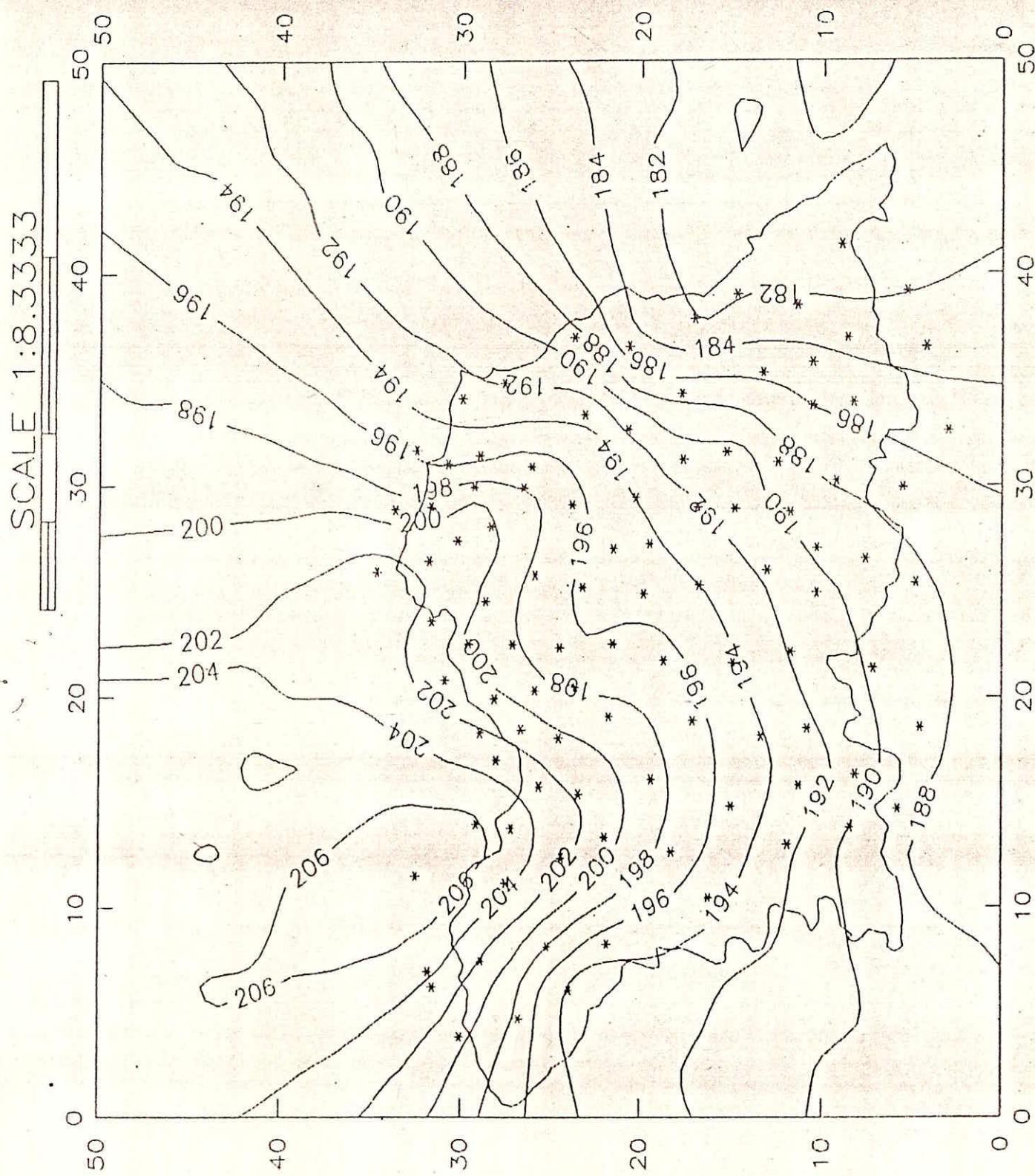


Fig.6.15 : Contour map of water table elevation with interpolated



J Fig. 6.16 : Contour map of water table elevation with interpolated values in June 78.

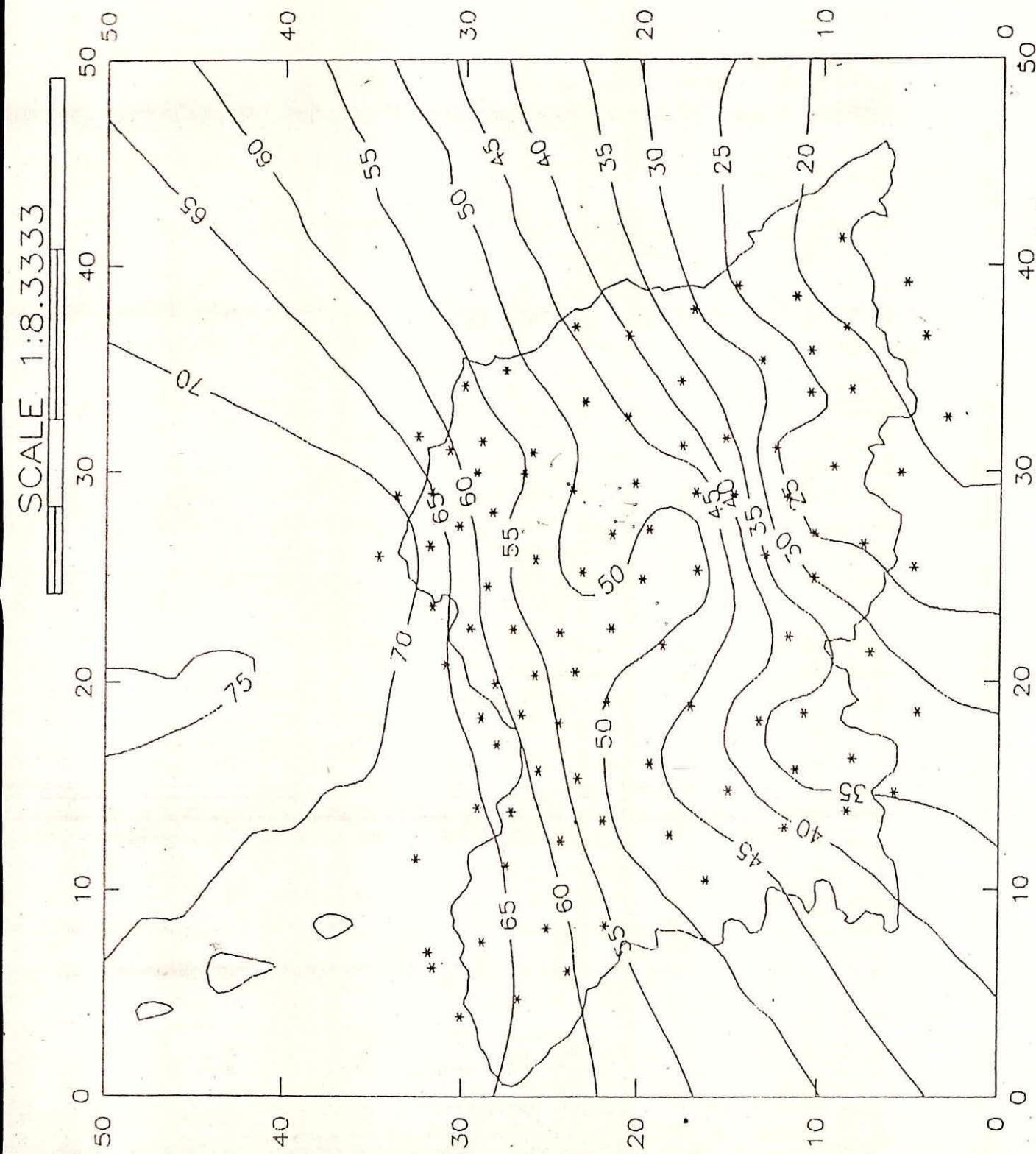


Fig. 6.17: Contour map of the aquifer thickness with interpolated values, m.

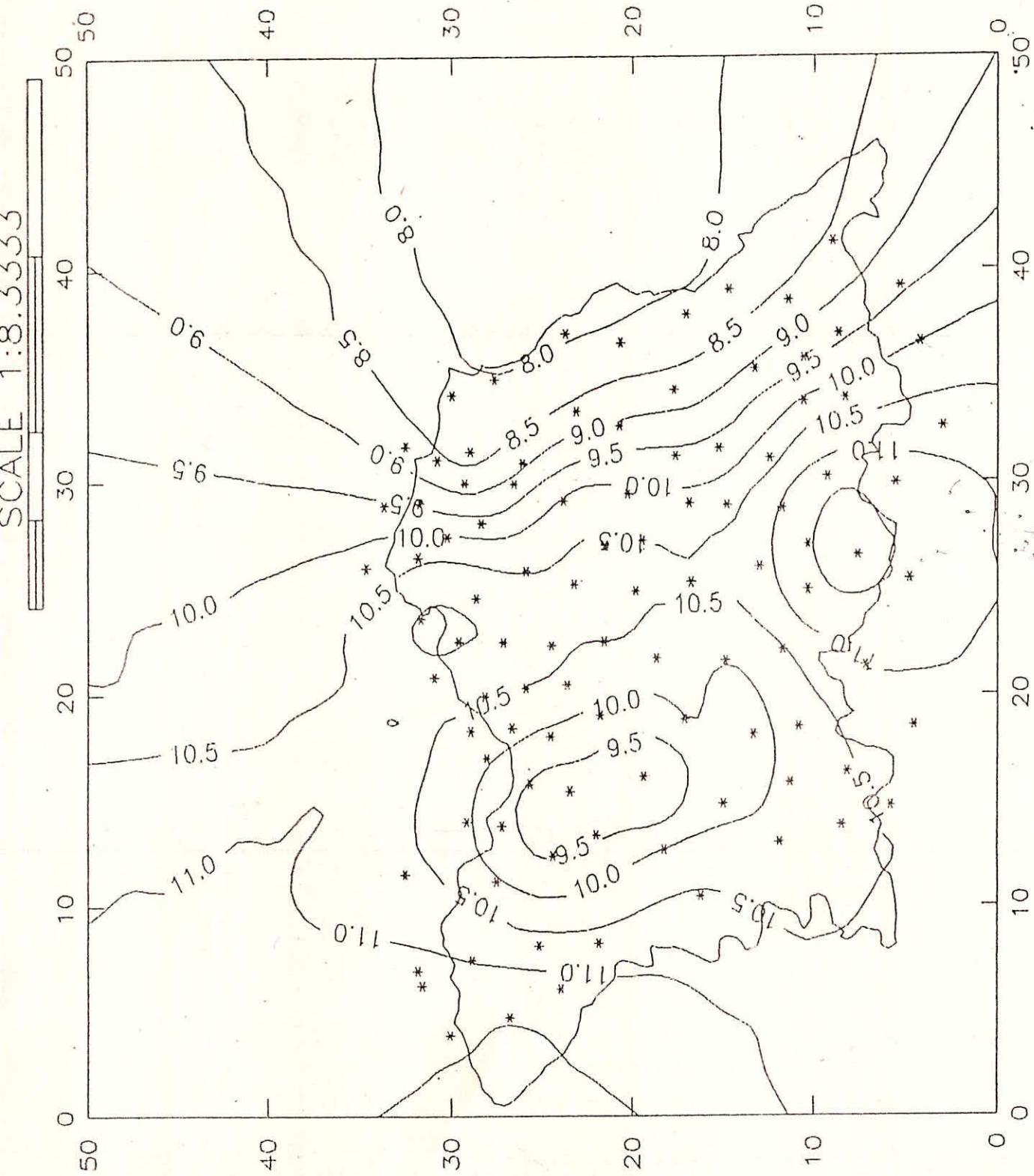


Fig. 6.18: Contour map of Block average specific yield with interpolated values, %.

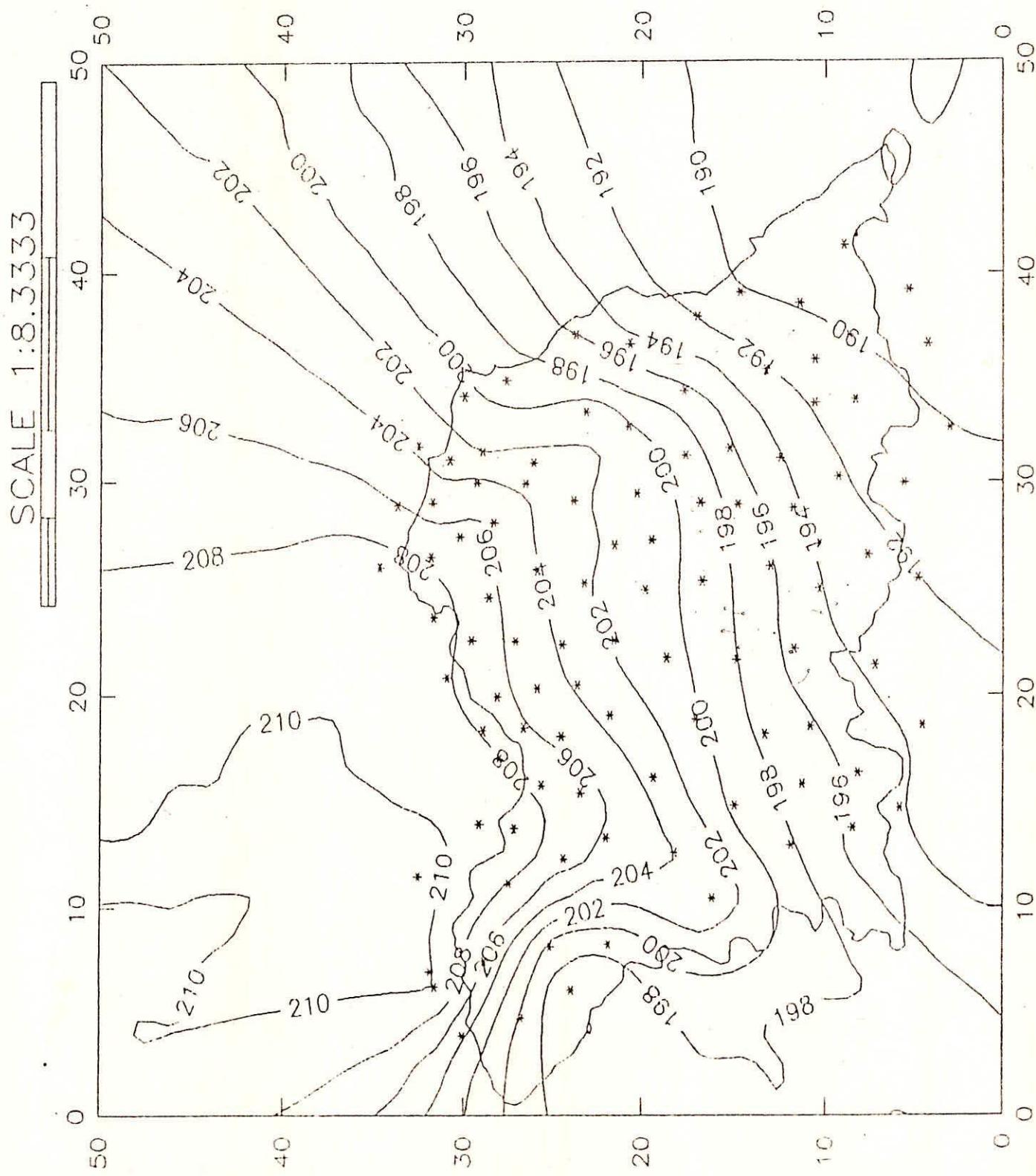


Fig. 6.19: Contour map of surface level with interpolated values,

Table 6.1: Nodal coordinates and interpolated values of water table in June 76, October 76, June 77, October 77, June 78, aquifer depth and specific yield.

Sl. No.	Grid No.	X axis, cm	Y axis, cm	Ground level, m	Water table in						Depth of aquifer, m	Specific yield,	
					June 76		Oct 76		June 77		Oct 77		
					(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)
1	1	3.5	29.7	203	199.2	200.1	200	200.5	200	67	11.45		
2	2	5.85	31.25	210	202.5	203.5	203	203	203	68	11.25		
3	3	6.6	31.5	210.1	203.6	204.5	204	204.8	204	68.5	11.2		
4	4	11.2	32.2	210.3	206	208	207	208	208	70	10.65		
5	5	13.65	28.8	210	206	208	206	208	206.5	65	10		
6	6	16.7	27.7	208	203.5	205.5	203.1	204.6	203.5	63	10		
7	7	18	28.6	208	203	204.1	202.1	203.8	202.5	63.5	10.25		
8	8	20.5	30.6	208	201.5	202.5	201	202.2	201	65	10.75		
9	9	23.25	31.35	208.7	202	202.2	201.2	202.5	200.5	66.5	11.2		
10	10	25.6	34.35	209.5	204	205	203	205	204	75	10.3		
11	11	28.5	33.35	206.1	199.7	199.7	198.8	202	199	70	9.7		
12	12	31.35	32.2	204	195	196	195	198.5	195.5	63	8.7		
13	13	4.35	26.4	199	195	196	195.1	197	197	64.5	11.55		
14	14	7.1	28.5	205	201	202	201	202	201	66	11.05		
15	15	10.85	27.15	207.8	204.2	206.1	204.2	205.6	204.2	64.5	10.05		
16	16	13.45	26.85	209	205	207	205	206.3	205	63.5	9.5		
17	17	15.45	25.3	207.7	203	205	203	204.8	203	58	9.25		
18	18	18.15	26.3	206	201.2	201	200	203	201	58	10		
19	19	19.6	27.8	206.2	201	200.4	200.2	202.2	200.5	60	10.5		
20	20	22.2	29.25	207.3	200.5	200.5	199.5	200.6	200	60.5	11		
21	21	24.2	28.3	206.2	199	199.2	198.5	199.5	199	57.5	11.1		
22	22	26.1	31.5	207.8	201.8	201.9	201.6	203.5	202	67.5	10.65		
23	23	28.65	31.4	205	199	199.2	198.6	200	200	65	9.5		
24	24	30.7	30.45	203	195.5	196	194.9	197	196	60	8.6		
25	25	33.75	29.65	200.5	192.5	193.5	191.5	194	193	56	8.15		
26	26	5.7	23.6	195.8	192	193.9	192	193.6	192	60	11.25		
27	27	7.8	24.8	198.1	195	195.9	194.5	195.8	197	63	10.8		
28	28	12.05	24.05	206.5	202	204	202	203.2	202	58	9.4		
29	29	15.1	23.1	207	201.5	203.9	201.5	203.6	202	52.5	9.25		
30	30	17.75	24.2	205.5	200	202.2	200	202.1	200.5	54	9.75		
31	31	20	25.55	205	199	200.7	198.7	200.8	199	56	10.5		
32	32	22.15	26.8	205.2	198.1	199	198	199	198	56	10.8		
33	33	25.45	25.55	204	197	197.9	197	198.2	197	50.5	10.6		
34	34	27.05	29.9	207	201	201.2	201	203	201.4	62	10.25		
35	35	27.7	28	205	200	200.4	200.1	202	200.5	55	9.8		
36	36	29.55	26.2	203.8	197.8	199	198	199.8	198.2	56	9.3		
37	37	29.6	28.95	205.7	199	196.6	198.6	200.5	200	57	8.85		
38	38	31.1	28.65	202	195.5	196.5	195	197.2	196	56	8.3		
39	39	30.55	25.75	203.2	197	198.2	197.1	199	197.5	55.5	9.1		
40	40	34.5	27.3	199	192	193	191.2	193	192	51.5	8		
41	41	7.9	21.5	198	193.8	195	193.2	195	194.5	55	10.8		
42	42	13.05	21.65	205.8	200.5	203	200.5	202	200.5	50	9.55		
43	43	18.75	21.45	203.5	198.5	200.5	198.2	200.5	199	48.5	9.8		
44	44	20.15	23.3	203.6	197.8	199.3	197.5	199.2	198	52.5	10.25		
45	45	22	24.15	203.6	196.5	197.8	196	197.8	196	52.5	10.55		
46	46	24.85	22.9	202	195	196.2	195	196.6	195.5	49	10.55		
47	47	28.75	23.5	202.3	196	197.2	195.8	198	196.5	48	10.4		
48	48	33	22.3	201.6	194	195	194	195.1	194	47	8.75		

Contd.

(1)	(2)	(3)	(4)	(5)	(6)	(7)	(8)	(9)	(10)	(11)	(12)
49	49	36.65	23.4	196	186	187.5	186	187	189	44	7.95
50	50	10.15	15.85	203.5	194.4	195.9	194.5	195.9	194.5	48	10.6
51	51	12.35	17.9	204.3	197.5	199.6	197.2	198.7	197	46.5	10.25
52	52	15.8	19.05	203.3	198.7	201	198.2	200.5	198.6	45	8.65
53	53	18.55	16.75	200	197	197.8	196.6	198.6	196.5	44	10
54	54	21.4	19.35	200.7	196.5	197.3	196.1	197.8	197	49.5	10.2
55	55	22.2	21.25	201.5	196	197.2	195.5	197.3	196.5	51.5	10.55
56	56	24.55	19.5	200.8	193.5	194.5	193	194.6	195.5	51.5	10.55
57	57	26.65	21.2	201.5	194	195.1	193.5	195.5	195	48.5	10.55
58	58	26.9	19.15	200.8	192	193	191.5	193.1	194.3	52	10.55
59	59	29.1	19.95	201	192.6	193.9	192	194	194	48.5	10.25
60	60	32.3	20.4	200.8	192	192.2	191.7	193.2	192	45	9.25
61	61	36.25	20.35	194	184	186	184	186	184	40	8.2
62	62	12.7	11.55	198	191.5	192.2	191.5	193.2	192	42.5	10.15
63	63	14.5	14.65	199.2	195	197	195	197	195	42.5	9.9
64	64	17.85	13	197.3	194.1	195.6	194	196.1	194	33.5	9.85
65	65	21.3	14.55	197.8	193.2	195	194	195.8	194	47	9.9
66	66	24.95	16.4	199	190	191	189.9	191.2	194	52.5	10.6
67	67	28.65	16.5	199	189.9	191	189.4	190.8	192	50	10.2
68	68	30.9	17.3	199	190.2	191.6	189.5	191.2	191	42.5	9.75
69	69	34	17.4	197	187.8	189	187	188.7	188	37.5	8.75
70	70	37.55	16.7	192	182	183	182	184	182	30	8.15
71	71	13.5	8.15	195	188.2	190.6	188.5	190.7	189	37.5	10.35
72	72	15.5	10.95	197	192.7	194.5	192.5	194.7	193	32.5	10.15
73	73	18.2	10.5	196.2	192.4	194.4	192.3	194.6	192.2	33.5	10.25
74	74	21.8	11.4	195	191	193	191	193.5	191.5	40	10.45
75	75	25.65	12.7	196	189.5	190	189	190.6	191.6	32.5	10.85
76	76	28.55	14.45	197	189	189.7	188	189.5	191	37.5	10.5
77	77	31.25	14.9	196	188.2	189.6	187.6	189	191.2	36	9.75
78	78	35	12.95	192.2	185	186	184.1	186.1	186	30	9.25
79	79	38.7	14.35	190	182	183	182	184	182	23.5	8.2
80	80	14.4	5.5	193.8	186.5	189	187	189.3	188	35	10.6
81	81	16.05	7.85	195.5	190.1	192.5	190	192.7	191	33	10.45
82	82	24.6	10	193.8	189.7	191.2	189.7	191.8	190.5	30	11.5
83	83	26.7	10	193.8	189	190.2	188.7	190.5	189.6	25	11.55
84	84	28.4	11.45	194	188.3	189	188	189.5	189.6	23	11.25
85	85	30.8	12.15	194	187.5	188.7	186.8	188.2	188.5	23.5	10.5
86	86	33.45	10.25	191.7	186	187.3	185.5	187.2	185	26.5	10.25
87	87	35.5	10.25	191	184.5	185.5	184.2	185.7	185	23	9.5
88	88	38.2	11.1	190	183.4	183.5	182.1	182.8	182.5	20	8.7
89	89	26.2	7.3	192.6	189	190.8	189	191.3	189	25	11.65
90	90	29.9	8.95	192	188	189	187.3	189.6	188	22	11.6
91	91	33.6	8	190.8	186.5	188	186.1	188.2	185.5	22	10.3
92	92	36.65	8.35	190	184	185	183.8	185.5	183.5	20	9.5
93	93	18.25	4.25	193.5	187.8	190.2	188	190.6	188.3	32	10.65
94	94	21.05	6.9	193.5	189.5	191.7	189.2	192.3	189.6	31.5	11
95	95	25.1	4.5	192	188.6	191	189	192.1	188.3	22.5	11.4
96	96	29.6	5.25	191	188.3	189.9	187.5	190.6	187.2	21	11.3
97	97	32.25	2.7	190	186.8	188.3	186.5	189.2	185.2	19	10.85
98	98	36.25	3.95	189.5	185	186	184.3	186.3	183.4	17.5	10.1
99	99	38.85	5	189	183	183.8	182.4	184	182.1	17.5	9.5
100	100	41	8.65	188	180.5	181.2	180	181.3	180.5	18.5	8.6

6.6.1. Normal mode run

Running the package in normal mode requires the values of LSW2=2 and LSW6=0. Running the package in this mode describes

about the behaviour of the aquifer i.e. whether the water table in the aquifer is rising or falling. While running the package, the values of DL, DELQ and ERROR were selected as 0.1, 0.05 and 0.08 respectively.

With no prescribed net recharge in SGMP12b.INP during the period of monsoon, a positive value of net change in storage, in SGMP13.OUT water balance component, indicates that water level in the aquifer has lowered and vice versa. It has happened only because the net recharge to the aquifer is prescribed as zero in SGMP12B.INP, however there would have been some recharge to the area.

6.6.2. Inverse mode run

Running the package in inverse mode requires the values of LSW2=2 and LSW6=1. The package in this mode describes about the artificial flow at each node for which the water table elevations are calculated keeping the initial head same as final, which is the assumption in the problem. While running the package, the values of DL, DELQ and ERROR remains same as prescribed in normal run.

In this run the change in head will be zero and an artificial recharge will be introduced at each node to keep the head same with in one percent error. Both iteration and superiteration works in this run. The water balance component of output file SGMP13.OUT is reported in Appendix F.

6.7. Discussion

The artificial nodal recharge in monsoon and non-monsoon period is given in Table 6.2. The following conclusions can be drawn by this exercise.

- a) The introduced artificial recharge should have been the net recharge in monsoon and non-monsoon period.
- b) During monsoon period the introduced artificial recharge is positive, indicates a recharge to the aquifer and during the non-monsoon period it is negative indicating a withdrawal from aquifer.

Table 6.2: Estimated net recharge in the study area for monsoon and non-monsoon period.

Sl. No.	Node No.	Net artificial recharge during monsoon and non-monsoon			
		monsoon mcm	non-monsoon mcm	monsoon mcm	non-monsoon mcm
1	14	8.4000	-8.4000	8.4000	-8.4000
2	15	14.6000	-14.0000	10.6000	-10.4000
3	16	7.8000	-7.6000	5.2000	-5.2000
4	17	7.4000	-7.2000	6.6000	-6.8000
5	18	4.8000	-6.0000	6.2000	-5.6000
6	19	4.8000	-8.4000	7.6000	-5.6000
7	20	.0000	-4.0000	4.2000	-2.0000
8	21	.8000	-3.6000	5.4000	-2.8000
9	22	.4000	-1.2000	8.6000	-6.8000
10	23	3.0000	-4.0000	4.0000	.0000
11	24	1.2000	-2.8000	4.8000	-2.4000
12	27	6.2000	-10.8000	9.6000	9.2000
13	28	12.8000	-12.4000	7.4000	-7.2000
14	29	11.6000	-11.6000	10.4000	-7.6000
15	30	8.4000	-8.4000	8.0000	-6.0000
16	31	5.4000	-6.4000	6.6000	-5.6000
17	32	4.2000	-4.8000	4.6000	-4.8000
18	33	5.6000	-5.6000	7.8000	-8.0000
19	34	.6000	-.4000	6.8000	-5.2000
20	35	1.8000	-1.2000	8.0000	-6.0000
21	36	4.0000	-3.2000	5.8000	-5.2000
22	37	-5.8000	4.8000	4.6000	-.8000
23	38	3.0000	-4.8000	6.8000	-3.6000
24	39	5.6000	-4.8000	8.8000	-6.8000
25	42	17.0000	-16.8000	10.0000	-10.0000
26	43	13.0000	-15.2000	15.4000	-9.6000
27	44	5.2000	-6.4000	6.0000	-4.4000
28	45	6.0000	-8.4000	8.2000	-8.4000
29	46	6.6000	-6.4000	8.4000	-6.0000
30	47	8.2000	-9.6000	15.6000	-10.4000
31	48	6.8000	-6.4000	7.6000	-7.2000
32	51	18.8000	-21.6000	13.6000	-15.2000
33	52	16.6000	-20.0000	16.6000	-13.6000
34	53	6.2000	-9.6000	16.2000	-16.8000
35	54	5.6000	-8.4000	12.2000	-5.2000
36	55	6.6000	-9.2000	9.8000	-4.4000
37	56	5.4000	-8.0000	10.4000	.0000
38	57	5.0000	-7.2000	8.8000	-2.4000
39	58	4.4000	-6.8000	8.8000	4.0000
40	59	7.4000	-10.8000	11.4000	.0000
41	60	1.0000	-2.8000	8.8000	-6.8000
42	63	17.6000	-18.0000	18.0000	-18.0000
43	64	9.8000	-10.4000	13.8000	-13.6000
45	65	14.0000	-7.6000	14.0000	-14.0000
46	66	7.8000	-9.2000	10.4000	14.8000
47	67	5.2000	-7.6000	6.8000	.8000
48	68	6.8000	-10.0000	8.0000	-1.2000
49	69	7.8000	-13.2000	11.4000	-4.4000
50	72	10.2000	-11.2000	12.6000	-9.6000
51	73	13.6000	-14.0000	15.6000	-16.4000

Contd.

52	74	16.2000	-16.4000	20.4000	-16.4000	
53	75	3.4000	-7.2000	11.6000	7.6000	
54	76	3.6000	-8.8000	7.8000	7.6000	
55	77	7.8000	-11.2000	7.8000	12.4000	
56	78	7.0000	-13.6000	14.4000	-4.4000	
57	81	16.8000	-17.6000	19.0000	-11.6000	
58	82	10.2000	-10.0000	14.0000	-8.4000	
59	83	5.8000	-7.2000	8.6000	-4.4000	
60	84	3.4000	-4.8000	7.2000	.4000	
61	85	7.0000	-10.8000	8.0000	-1.2000	
62	86	6.4000	-8.8000	8.4000	-6.0000	
63	87	3.6000	-4.8000	5.6000	-2.8000	
64	89	14.6000	-14.8000	19.0000	-18.8000	
65	90	8.0000	-14.0000	19.0000	-8.8000	
66	91	12.0000	-15.2000	16.8000	-22.0000	
67	92	6.4000	-7.6000	10.6000	-12.4000	
<hr/>		Total	481.3997	-584.3998	663.3997	-380.7998

APPENDIXES

APPENDIX A

1. Format of input file for normal run

2. Format of input file SGHP11.INP

'MONTHLY DATA OF HARYANA'
'HANDLING SGHP11.INP'

N	NSIDES							
84	149							
TMBS	DMTIM	T	DELTA	MINOR	MAJOR	LIST	DL	
'MONTH'	30.5	0	1	1	12	1	0.02	
SCALE	ERROR	COEFFA	PREDIC	DELQ	LSW1	LSW2	NITER	
10000	0.001	1	1	0.0002	1	2	50	
K	NTYPE	CO(X,1)	CO(Y,1)	SL(K)	TL(K)	BL(K)	UL(K)	LL(K)
1	1	1.5	22.05	9.9	9.9	-90.1	9.9	-90.1
2	1	4	22.05	9.9	9.9	-90.1	9.9	-90.1
3	1	6.5	22.05	9.9	9.9	-90.1	9.9	-90.1
4	1	8.95	22.05	9.4	9.4	-90.6	9.4	-90.6
5	1	11.45	22.05	9.5	9.5	-90.5	9.5	-90.5
6	1	14	22.05	9.9	9.9	-90.1	9.9	-90.1
7	1	16.4	22.05	10.3	10.3	-89.7	10.3	-89.7
8	1	1.5	20.1	9.69	9.69	-90.31	9.69	-90.31
9	3	4	20.1	10.31	10.31	-89.69	10.31	-89.69
10	3	6.5	20.1	10.07	10.07	-89.93	10.07	-89.93
11	3	8.95	20.1	9.5	9.5	-90.5	9.5	-90.5
12	3	11.45	20.1	9.6	9.6	-90.4	9.6	-90.4
13	3	14	20.1	9.85	9.85	-90.15	9.85	-90.15
14	1	16.4	20.1	10.105	10.105	-89.895	10.105	-89.895
15	1	1.5	18.1	9.6	9.6	-90.4	9.6	-90.4
16	3	4	18.1	11.1	11.1	-88.9	11.1	-88.9
17	3	6.5	18.1	11	11	-89	11	-89
18	3	8.95	18.1	10.3	10.3	-89.7	10.3	-89.7
19	3	11.45	18.1	10.3	10.3	-89.7	10.3	-89.7
20	3	14	18.1	10.1	10.1	-89.9	10.1	-89.9
21	1	16.4	18.1	9.9	9.9	-90.1	9.9	-90.1
22	1	1.5	16.1	9.44	9.44	-90.56	9.44	-90.56
23	3	4	16.1	10.55	10.55	-89.45	10.55	-89.45
24	3	6.5	16.1	11.1	11.1	-88.9	11.1	-88.9
25	3	8.95	16.1	10.4	10.4	-89.6	10.4	-89.6
26	3	11.45	16.1	10.4	10.4	-89.6	10.4	-89.6
27	3	14	16.1	10.22	10.22	-89.78	10.22	-89.78
28	1	16.4	16.1	9.8	9.8	-90.2	9.8	-90.2
29	1	1.5	14.1	9.1	9.1	-90.9	9.1	-90.9
30	3	4	14.1	10.3	10.3	-89.7	10.3	-89.7
31	3	6.5	14.1	10.5	10.5	-89.5	10.5	-89.5
32	3	8.95	14.1	10.2	10.2	-89.8	10.2	-89.8
33	3	11.45	14.1	10.2	10.2	-89.8	10.2	-89.8
34	3	14	14.1	10.3	10.3	-89.7	10.3	-89.7
35	1	16.4	14.1	10.2	10.2	-89.8	10.2	-89.8
36	1	1.5	12.15	10.2	10.2	-89.8	10.2	-89.8
37	3	4	12.15	10.9	10.9	-89.1	10.9	-89.1
38	3	6.5	12.15	10.6	10.6	-89.4	10.6	-89.4
39	3	8.95	12.15	10.4	10.4	-89.6	10.4	-89.6
40	3	11.45	12.15	10	10	-90	10	-90

Contd.

41	3	14	12.15	10.5	10.5	-89.5	10.5	-89.5
42	1	16.4	12.15	10.5	10.5	-89.5	10.5	-89.5
43	1	1.5	10.2	10.2	10.2	-89.8	10.2	-89.8
44	3	4	10.2	10.2	10.2	-89.8	10.2	-89.8
45	3	6.5	10.2	10.4	10.4	-89.6	10.4	-89.6
46	3	8.95	10.2	10.4	10.4	-89.6	10.4	-89.6
47	3	11.45	10.2	9.9	9.9	-90.1	9.9	-90.1
48	3	14	10.2	10.4	10.4	-89.6	10.4	-89.6
49	1	16.4	10.2	10.5	10.5	-89.5	10.5	-89.5
50	1	1.5	8.2	9.99	9.99	-90.01	9.99	-90.01
51	3	4	8.2	10.1	10.1	-89.9	10.1	-89.9
52	3	6.5	8.2	10.3	10.3	-89.7	10.3	-89.7
53	3	8.95	8.2	10.4	10.4	-89.6	10.4	-89.6
54	3	11.45	8.2	9.9	9.9	-90.1	9.9	-90.1
55	3	14	8.2	10.5	10.5	-89.5	10.5	-89.5
56	1	16.4	8.2	11.2	11.2	-88.8	11.2	-88.8
57	1	1.5	6.2	10	10	-90	10	-90
58	3	4	6.2	9.9	9.9	-90.1	9.9	-90.1
59	3	6.5	6.2	10.2	10.2	-89.8	10.2	-89.8
60	3	8.95	6.2	10.6	10.6	-89.4	10.6	-89.4
61	3	11.45	6.2	10.4	10.4	-89.6	10.4	-89.6
62	3	14	6.2	10.6	10.6	-89.4	10.6	-89.4
63	1	16.4	6.2	11.7	11.7	-88.3	11.7	-88.3
64	1	1.5	4.2	10.16	10.16	-89.84	10.16	-89.84
65	3	4	4.2	10.2	10.2	-89.8	10.2	-89.8
66	3	6.5	4.2	10.1	10.1	-89.9	10.1	-89.9
67	3	8.95	4.2	9.9	9.9	-90.1	9.9	-90.1
68	3	11.45	4.2	10.1	10.1	-89.9	10.1	-89.9
69	3	14	4.2	10.4	10.4	-89.6	10.4	-89.6
70	1	16.4	4.2	10.76	10.76	-89.24	10.76	-89.24
71	1	1.5	2.25	10.3	10.3	-89.7	10.3	-89.7
72	3	4	2.25	10.4	10.4	-89.6	10.4	-89.6
73	3	6.5	2.25	10.3	10.3	-89.7	10.3	-89.7
74	3	8.95	2.25	10.1	10.1	-89.9	10.1	-89.9
75	3	11.45	2.25	10	10	-90	10	-90
76	3	14	2.25	10.2	10.2	-89.8	10.2	-89.8
77	1	16.4	2.25	10.2	10.2	-89.8	10.2	-89.8
78	1	1.5	0.25	10.21	10.21	-89.79	10.21	-89.79
79	1	4	0.25	10.41	10.41	-89.59	10.41	-89.59
80	1	6.5	0.25	10.5	10.5	-89.5	10.5	-89.5
81	1	8.95	0.25	10.7	10.7	-89.3	10.7	-89.3
82	1	11.45	0.25	10.2	10.2	-89.8	10.2	-89.8
83	1	14	0.25	9.99	9.99	-90.01	9.99	-90.01
84	1	16.4	0.25	10.03	10.03	-89.97	10.03	-89.97

K	AS(K)	ASC(K)
1	0.2	0.001
2	0.2	0.001
3	0.2	0.001
4	0.2	0.001
5	0.2	0.001
6	0.2	0.001
7	0.2	0.001
8	0.2	0.001
9	0.2	0.001
10	0.2	0.001
11	0.2	0.001
12	0.2	0.001

Contd.

13	0.2	0.001
14	0.2	0.001
15	0.2	0.001
16	0.2	0.001
17	0.2	0.001
18	0.2	0.001
19	0.2	0.001
20	0.2	0.001
21	0.2	0.001
22	0.2	0.001
23	0.2	0.001
24	0.2	0.001
25	0.2	0.001
26	0.2	0.001
27	0.2	0.001
28	0.2	0.001
29	0.2	0.001
30	0.2	0.001
31	0.2	0.001
32	0.2	0.001
33	0.2	0.001
34	0.2	0.001
35	0.2	0.001
36	0.2	0.001
37	0.2	0.001
38	0.2	0.001
39	0.2	0.001
40	0.2	0.001
41	0.2	0.001
42	0.2	0.001
43	0.2	0.001
44	0.2	0.001
45	0.2	0.001
46	0.2	0.001
47	0.2	0.001
48	0.2	0.001
49	0.2	0.001
50	0.2	0.001
51	0.2	0.001
52	0.2	0.001
53	0.2	0.001
54	0.2	0.001
55	0.2	0.001
56	0.2	0.001
57	0.2	0.001
58	0.2	0.001
59	0.3	0.001
60	0.2	0.001
61	0.2	0.001
62	0.2	0.001
63	0.2	0.001
64	0.2	0.001
65	0.2	0.001
66	0.2	0.001
67	0.2	0.001
68	0.2	0.001
69	0.2	0.001
70	0.2	0.001

71	0.2	0.001
72	0.2	0.001
73	0.2	0.001
74	0.2	0.001
75	0.2	0.001
76	0.2	0.001
77	0.2	0.001
78	0.2	0.001
79	0.2	0.001
80	0.2	0.001
81	0.2	0.001
82	0.2	0.001
83	0.2	0.001
84	0.2	0.001

IB	IE	CONDU
1	2	2.4
2	3	2.45
3	4	2.55
4	5	2.7
5	6	2.85
6	7	3
1	8	2.35
2	9	2.45
3	10	2.5
4	11	2.6
5	12	2.8
6	13	3
7	14	3.1
8	9	2.4
9	10	2.45
10	11	2.55
11	12	2.7
12	13	2.95
13	14	3.1
8	15	2.3
9	16	2.45
10	17	2.5
11	18	2.6
12	19	2.8
13	20	3.05
14	21	3.2
15	16	2.35
16	17	2.5
17	18	2.55
18	19	2.75
19	20	3
20	21	3.2
15	22	2.3
16	23	2.45
17	24	2.55
18	25	2.6
19	26	2.95
20	27	3.2
21	28	3.3
22	23	2.35
23	24	2.6
24	25	2.6

Contd.

25	26	2.9
26	27	3.2
27	28	3.35
22	29	2.2
23	30	2.4
24	31	2.6
25	32	2.8
26	33	3.15
27	34	3.35
28	35	3.45
29	36	2.25
30	37	2.4
31	38	2.65
32	39	3
33	40	3.4
34	41	3.45
29	42	2.1
30	43	2.2
31	44	2.5
32	45	2.8
33	46	3.3
34	47	3.5
35	48	3.5
36	49	2.1
37	50	2.2
38	51	2.55
39	52	3
40	53	3.45
41	54	3.45
42	55	1.95
43	56	1.9
44	57	2
45	58	2.3
46	59	2.7
47	60	3.2
48	61	3.45
49	62	3.45
50	63	1.9
51	64	2
52	65	2.4
53	66	2.9
54	67	3.3
55	68	3.3
56	69	3.45
57	70	3.45
58	71	1.85
59	72	1.85
60	73	1.9
61	74	2.3
62	75	2.85
63	76	3.2
64	77	3.35
65	78	1.85
66	79	1.85
67	80	2.1

53	60	2.65
54	61	3.05
55	62	3.25
56	63	3.35
57	58	1.9
58	59	2
59	60	2.5
60	61	2.9
61	62	3.2
62	63	3.35
57	64	1.95
58	65	2.05
59	66	2.3
60	67	2.75
61	68	3.05
62	69	3.25
63	70	3.35
64	65	2.1
65	66	2.25
66	67	2.65
67	68	2.95
68	69	3.15
69	70	3.25
64	71	2.1
65	72	2.25
66	73	2.5
67	74	2.9
68	75	3.1
69	76	3.2
70	77	3.3
71	72	2.2
72	73	2.4
73	74	2.7
74	75	3
75	76	3.15
76	77	3.25
71	78	2.2
72	79	2.4
73	80	2.6
74	81	2.9
75	82	3.05
76	83	3.2
77	84	3.25
78	79	2.3
79	80	2.5
80	81	2.75
81	82	2.95
82	83	3.15
83	84	3.25

3. Format of input file SGMP12A.INP

LSW3	LSW4	LSW5	LSW6
1	1	1	0

H(K): initial water table elevations

6.7	7.3	7.2	7.2	7.9	8.4	8.9	6.54	7.01	7.07
7	7.55	8.15	8.605	6.5	7.6	8	7.9	8	8.3
8.4	7.19	7.95	8.6	8.4	8.5	8.61	8.3	7.8	8.2
9.1	9.5	9.1	9	8.5	8	8.6	9.6	10	9.4
9.1	8.9	7.7	8.4	9.5	9.8	9.4	9.2	9	7.39
8	8.9	9.6	9.1	8.9	8.9	7.1	7.4	8.3	9.1
8.7	8.5	8.5	6.8	7.2	7.9	8.3	8.2	8.05	8.06
6.6	6.9	7.4	7.7	7.8	7.6	7.7	6.36	6.71	7
7.1	7.25	7.14	7.25						

H(K): water table elevation time step 1

6.7	7.3	7.2	7.2	7.9	8.4	8.9	6.54	7.01	7.07
7	7.55	8.15	8.605	6.5	7.6	8	7.9	8	8.3
8.4	7.19	7.95	8.6	8.4	8.5	8.61	8.3	7.8	8.2
9.1	9.5	9.1	9	8.5	8	8.6	9.6	10	9.4
9.1	8.9	7.7	8.4	9.5	9.8	9.4	9.2	9	7.39
8	8.9	9.6	9.1	8.9	8.9	7.1	7.4	8.3	9.1
8.7	8.5	8.5	6.8	7.2	7.9	8.3	8.2	8.05	8.06
6.6	6.9	7.4	7.7	7.8	7.6	7.7	6.36	6.71	7
7.1	7.25	7.14	7.25						

4. Format of input file SGMP12A.INP

QNET(K): net recharge time step 1

0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0

5. Format of input file SGMP12A.INP

LSW7	LSW8	LSW9
2	2	2

APPENDIX B

1. Format of output files of normal run

2. Format of output file SGMP11.OUT

GROUNDWATER MODEL FOR MONTHLY DATA OF HARYANA
HANDLING SGMP11.INP

NODAL NETWORK INFORMATION AND HYDROGEOLOGICAL DATA

TOTAL NUMBER OF NODES IS 84

UNIT LENGTH : 1 METRE

UNIT AREA : 1,000,000 SQ. METRES

UNIT VOLUME : 1,000,000 CU. METRES

UNIT OF TIME FOR DELTA AND BOUNDARY CONDITIONS IS MONTH

BETWEEN	NODE	PERMEABILITY	NODE	EFFECTIVE POROSITY	STORAGE COEFF.	AREA
1 AND	8	2.35	1	.2000	.001000	.01
1 AND	2	2.40	2	.2000	.001000	.02
2 AND	9	2.45	3	.2000	.001000	.02
2 AND	3	2.45	4	.2000	.001000	.02
3 AND	10	2.50	5	.2000	.001000	.02
3 AND	4	2.55	6	.2000	.001000	.02
4 AND	11	2.60	7	.2000	.001000	.01
4 AND	5	2.70	8	.2000	.001000	.02
5 AND	12	2.80	9	.2000	.001000	.05
5 AND	6	2.85	10	.2000	.001000	.05
6 AND	13	3.00	11	.2000	.001000	.05
6 AND	7	3.00	12	.2000	.001000	.05
7 AND	14	3.10	13	.2000	.001000	.05
8 AND	15	2.30	14	.2000	.001000	.02
8 AND	9	2.40	15	.2000	.001000	.03
9 AND	10	2.45	16	.2000	.001000	.05
9 AND	16	2.45	17	.2000	.001000	.05
10 AND	11	2.55	18	.2000	.001000	.05
10 AND	17	2.50	19	.2000	.001000	.05
11 AND	12	2.70	20	.2000	.001000	.05
11 AND	18	2.60	21	.2000	.001000	.02
12 AND	13	2.95	22	.2000	.001000	.03
12 AND	19	2.80	23	.2000	.001000	.05
13 AND	14	3.10	24	.2000	.001000	.05
13 AND	20	3.05	25	.2000	.001000	.05
14 AND	21	3.20	26	.2000	.001000	.05
15 AND	22	2.30	27	.2000	.001000	.05
15 AND	16	2.35	28	.2000	.001000	.02
16 AND	17	2.50	29	.2000	.001000	.02
16 AND	23	2.45	30	.2000	.001000	.05
17 AND	18	2.55	31	.2000	.001000	.05

Contd.

17 AND 24	2.55	32	.2000	.001000	.05
18 AND 19	2.75	33	.2000	.001000	.05
18 AND 25	2.60	34	.2000	.001000	.05
19 AND 20	3.00	35	.2000	.001000	.02
19 AND 26	2.95	36	.2000	.001000	.02
20 AND 21	3.20	37	.2000	.001000	.05
20 AND 27	3.20	38	.2000	.001000	.05
21 AND 28	3.30	39	.2000	.001000	.05
22 AND 29	2.20	40	.2000	.001000	.05
22 AND 23	2.35	41	.2000	.001000	.05
23 AND 24	2.60	42	.2000	.001000	.02
23 AND 30	2.40	43	.2000	.001000	.02
24 AND 25	2.60	44	.2000	.001000	.05
24 AND 31	2.60	45	.2000	.001000	.05
25 AND 26	2.90	46	.2000	.001000	.05
25 AND 32	2.80	47	.2000	.001000	.05
26 AND 27	3.20	48	.2000	.001000	.05
26 AND 33	3.15	49	.2000	.001000	.02
27 AND 28	3.35	50	.2000	.001000	.03
27 AND 34	3.35	51	.2000	.001000	.05
28 AND 35	3.45	52	.2000	.001000	.05
29 AND 36	2.10	53	.2000	.001000	.05
29 AND 30	2.25	54	.2000	.001000	.05
30 AND 31	2.40	55	.2000	.001000	.05
30 AND 37	2.20	56	.2000	.001000	.02
31 AND 32	2.65	57	.2000	.001000	.03
31 AND 38	2.50	58	.2000	.001000	.05
32 AND 33	3.00	59	.2000	.001000	.05
32 AND 39	2.80	60	.2000	.001000	.05
33 AND 34	3.40	61	.2000	.001000	.05
33 AND 40	3.30	62	.2000	.001000	.05
34 AND 35	3.45	63	.2000	.001000	.02
34 AND 41	3.50	64	.2000	.001000	.02
35 AND 42	3.50	65	.2000	.001000	.05
36 AND 43	1.95	66	.2000	.001000	.05
36 AND 37	2.10	67	.2000	.001000	.05
37 AND 38	2.20	68	.2000	.001000	.05
37 AND 44	2.00	69	.2000	.001000	.05
38 AND 39	2.55	70	.2000	.001000	.02
38 AND 45	2.30	71	.2000	.001000	.02
39 AND 40	3.00	72	.2000	.001000	.05
39 AND 46	2.70	73	.2000	.001000	.05
40 AND 41	3.45	74	.2000	.001000	.05
40 AND 47	3.20	75	.2000	.001000	.05
41 AND 42	3.45	76	.2000	.001000	.05
41 AND 48	3.45	77	.2000	.001000	.02
42 AND 49	3.45	78	.2000	.001000	.01
43 AND 50	1.85	79	.2000	.001000	.03
43 AND 44	1.90	80	.2000	.001000	.02
44 AND 45	2.00	81	.2000	.001000	.02
44 AND 51	1.85	82	.2000	.001000	.03
45 AND 46	2.40	83	.2000	.001000	.02
45 AND 52	2.10	84	.2000	.001000	.01
46 AND 47	2.90				
46 AND 53	2.65				
47 AND 48	3.30				
47 AND 54	3.10				
48 AND 49	3.45				

48 AND	55	3.35
49 AND	56	3.45
50 AND	57	1.85
50 AND	51	1.95
51 AND	52	1.80
51 AND	58	1.85
52 AND	53	2.30
52 AND	59	2.10
53 AND	54	2.85
53 AND	60	2.65
54 AND	55	3.20
54 AND	61	3.05
55 AND	56	3.35
55 AND	62	3.25
56 AND	63	3.35
57 AND	64	1.95
57 AND	58	1.90
58 AND	59	2.00
58 AND	65	2.05
59 AND	60	2.50
59 AND	66	2.30
60 AND	61	2.90
60 AND	67	2.75
61 AND	62	3.20
61 AND	63	3.05
62 AND	63	3.35
62 AND	69	3.25
63 AND	70	3.35
64 AND	71	2.10
64 AND	65	2.10
65 AND	66	2.25
65 AND	72	2.25
66 AND	67	2.65
66 AND	73	2.50
67 AND	68	2.95
67 AND	74	2.90
68 AND	69	3.15
68 AND	75	3.10
69 AND	70	3.25
69 AND	76	3.20
70 AND	77	3.30
71 AND	78	2.20
71 AND	72	2.20
72 AND	73	2.40
72 AND	79	2.40
73 AND	74	2.70
73 AND	80	2.60
74 AND	75	3.00
74 AND	81	2.90
75 AND	76	3.15
75 AND	82	3.05
76 AND	77	3.25
76 AND	83	3.20
77 AND	84	3.25
78 AND	79	2.30
79 AND	80	2.50
80 AND	81	2.75
81 AND	82	2.95

Contd.

82 AND 83 3.15
83 AND 84 3.25

TOTAL AREA 3.25

NODE NO.	X-COORD. (CM)	Y-COORD. (CM)	SURFACE (K)	TOP (K)	BOTTOM (K)	THICKNESS (K)	UPPER LEVEL	LOWER LEVEL
1	1.50	22.05	9.90	9.90	-90.10	100.00	9.90	-90.10
2	4.00	22.05	9.90	9.90	-90.10	100.00	9.90	-90.10
3	6.50	22.05	9.90	9.90	-90.10	100.00	9.90	-90.10
4	8.95	22.05	9.40	9.40	-90.60	100.00	9.40	-90.60
5	11.45	22.05	9.50	9.50	-90.50	100.00	9.50	-90.50
6	14.00	22.05	9.90	9.90	-90.10	100.00	9.90	-90.10
7	16.40	22.05	10.30	10.30	-89.70	100.00	10.30	-89.70
8	1.50	20.10	9.69	9.69	-90.31	100.00	9.69	-90.31
9	4.00	20.10	10.31	10.31	-89.69	100.00	10.31	-89.69
10	6.50	20.10	10.07	10.07	-89.93	100.00	10.07	-89.93
11	8.95	20.10	9.50	9.50	-90.50	100.00	9.50	-90.50
12	11.45	20.10	9.60	9.60	-90.40	100.00	9.60	-90.40
13	14.00	20.10	9.85	9.85	-90.15	100.00	9.85	-90.15
14	16.40	20.10	10.10	10.10	-89.89	100.00	10.10	-89.89
15	1.50	18.10	9.60	9.60	-90.40	100.00	9.60	-90.40
16	4.00	18.10	11.10	11.10	-88.90	100.00	11.10	-88.90
17	6.50	18.10	11.00	11.00	-89.00	100.00	11.00	-89.00
18	8.95	18.10	10.30	10.30	-89.70	100.00	10.30	-89.70
19	11.45	18.10	10.30	10.30	-89.70	100.00	10.30	-89.70
20	14.00	18.10	10.10	10.10	-89.90	100.00	10.10	-89.90
21	16.40	18.10	9.90	9.90	-90.10	100.00	9.90	-90.10
22	1.50	16.10	9.44	9.44	-90.56	100.00	9.44	-90.56
23	4.00	16.10	10.55	10.55	-89.45	100.00	10.55	-89.45
24	6.50	16.10	11.10	11.10	-88.90	100.00	11.10	-88.90
25	8.95	16.10	10.40	10.40	-89.60	100.00	10.40	-89.60
26	11.45	16.10	10.40	10.40	-89.60	100.00	10.40	-89.60
27	14.00	16.10	10.22	10.22	-89.78	100.00	10.22	-89.78
28	16.40	16.10	9.80	9.80	-90.20	100.00	9.80	-90.20
29	1.50	14.10	9.10	9.10	-90.90	100.00	9.10	-90.90
30	4.00	14.10	10.30	10.30	-89.70	100.00	10.30	-89.70
31	6.50	14.10	10.50	10.50	-89.50	100.00	10.50	-89.50
32	8.95	14.10	10.20	10.20	-89.80	100.00	10.20	-89.80
33	11.45	14.10	10.20	10.20	-89.80	100.00	10.20	-89.80
34	14.00	14.10	10.30	10.30	-89.70	100.00	10.30	-89.70
35	16.40	14.10	10.20	10.20	-89.80	100.00	10.20	-89.80
36	1.50	12.15	10.20	10.20	-89.80	100.00	10.20	-89.80
37	4.00	12.15	10.90	10.90	-89.10	100.00	10.90	-89.10
38	6.50	12.15	10.60	10.60	-89.40	100.00	10.60	-89.40
39	8.95	12.15	10.40	10.40	-89.60	100.00	10.40	-89.60
40	11.45	12.15	10.00	10.00	-90.00	100.00	10.00	-90.00
41	14.00	12.15	10.50	10.50	-89.50	100.00	10.50	-89.50
42	16.40	12.15	10.50	10.50	-89.50	100.00	10.50	-89.50
43	1.50	10.20	10.20	10.20	-89.80	100.00	10.20	-89.80
44	4.00	10.20	10.20	10.20	-89.80	100.00	10.20	-89.80
45	6.50	10.20	10.40	10.40	-89.60	100.00	10.40	-89.60
46	8.95	10.20	10.40	10.40	-89.60	100.00	10.40	-89.60
47	11.45	10.20	9.90	9.90	-90.10	100.00	9.90	-90.10
48	14.00	10.20	10.40	10.40	-89.60	100.00	10.40	-89.60
49	16.40	10.20	10.50	10.50	-89.50	100.00	10.50	-89.50

Contd.

50	1.50	8.20	9.99	9.99	-90.01	100.00	9.99	-90.01
51	4.00	8.20	10.10	10.10	-89.90	100.00	10.10	-89.90
52	6.50	8.20	10.30	10.30	-89.70	100.00	10.30	-89.70
53	8.95	8.20	10.40	10.40	-89.60	100.00	10.40	-89.60
54	11.45	8.20	9.90	9.90	-90.10	100.00	9.90	-90.10
55	14.00	8.20	10.50	10.50	-89.50	100.00	10.50	-89.50
56	16.40	8.20	11.20	11.20	-88.80	100.00	11.20	-88.80
57	1.50	6.20	10.00	10.00	-90.00	100.00	10.00	-90.00
58	4.00	6.20	9.90	9.90	-90.10	100.00	9.90	-90.10
59	6.50	6.20	10.20	10.20	-89.80	100.00	10.20	-89.80
60	8.95	6.20	10.60	10.60	-89.40	100.00	10.60	-89.40
61	11.45	6.20	10.40	10.40	-89.60	100.00	10.40	-89.60
62	14.00	6.20	10.60	10.60	-89.40	100.00	10.60	-89.40
63	16.40	6.20	11.70	11.70	-88.30	100.00	11.70	-88.30
64	1.50	4.20	10.16	10.16	-89.84	100.00	10.16	-89.84
65	4.00	4.20	10.20	10.20	-89.80	100.00	10.20	-89.80
66	6.50	4.20	10.10	10.10	-89.90	100.00	10.10	-89.90
67	8.95	4.20	9.90	9.90	-90.10	100.00	9.90	-90.10
68	11.45	4.20	10.10	10.10	-89.90	100.00	10.10	-89.90
69	14.00	4.20	10.40	10.40	-89.60	100.00	10.40	-89.60
70	16.40	4.20	10.76	10.76	-89.24	100.00	10.76	-89.24
71	1.50	2.25	10.30	10.30	-89.70	100.00	10.30	-89.70
72	4.00	2.25	10.40	10.40	-89.60	100.00	10.40	-89.60
73	6.50	2.25	10.30	10.30	-89.70	100.00	10.30	-89.70
74	8.95	2.25	10.10	10.10	-89.90	100.00	10.10	-89.90
75	11.45	2.25	10.00	10.00	-90.00	100.00	10.00	-90.00
76	14.00	2.25	10.20	10.20	-89.80	100.00	10.20	-89.80
77	16.40	2.25	10.20	10.20	-89.80	100.00	10.20	-89.80
78	1.50	.25	10.21	10.21	-89.79	100.00	10.21	-89.79
79	4.00	.25	10.41	10.41	-89.59	100.00	10.41	-89.59
80	6.50	.25	10.50	10.50	-89.50	100.00	10.50	-89.50
81	8.95	.25	10.70	10.70	-89.30	100.00	10.70	-89.30
82	11.45	.25	10.20	10.20	-89.80	100.00	10.20	-89.80
83	14.00	.25	9.99	9.99	-90.01	100.00	9.99	-90.01
84	16.40	.25	10.03	10.03	-89.97	100.00	10.03	-89.97

MISCELLANEOUS CONSTANTS

LIST = 1	SCALE = 10000.00	LSW1 = 1
MAJOR = 12	ERROR = .00	LSW2 = 2
MINOR = 1	DELQ = .00	NITER = 50
DELTA = 1.000	COFFFA = 1.00	DG = .0
DHTIM = 30.500	PREDIC = 1.00	

3. Format of output file SGMP12.OUT

GROUNDWATER MODEL FOR MONTHLY DATA OF HARYANA
HANDLING SGMP11.INP

CALCULATION OF WATER BALANCES (RESULTS NOT PRINTED)

VALUES OF EXTERNAL SWITCHES

LSW3 = 1

LSW4 = 1

LSW5 = 1

LSWG = 0

FLOW ACROSS BOUNDARIES

NODE	INFLOW	OUTFLOW
9	.0157	-.0416
10	.0000	-.0212
11	.0000	-.0444
12	.0003	-.0110
13	.0332	.0000
16	.0000	-.0509
20	.0076	.0000
23	.0000	-.0256
27	.0003	-.0074
30	.0022	-.0063
34	.0047	-.0054
37	.0048	-.0050
41	.0256	-.0006
44	.0000	-.0121
48	.0322	-.0001
51	.0000	-.0170
55	.0332	.0000
58	.0000	-.0184
62	.0203	.0000
65	.0000	-.0219
69	.0107	-.0001
72	.0000	-.0443
73	.0000	-.0265
74	.0000	-.0399
75	.0000	-.0400
76	.0114	-.0621
TOTAL	.2022	-.5018

4. Format of output file SGMP13.OUT

GROUNDWATER MODEL FOR MONTHLY DATA OF HARYANA
HANDLING SGMP11.INP

PRINTING OF CALCULATED WATER LEVELS AND BALANCES

TOTAL NUMBER OF NODES IS 84

UNIT LENGTH : 1 METRE
UNIT AREA : 1,000,000 SQ .METRES
UNIT VOLUME : 1,000,000 CU. METRES

UNIT OF TIME FOR DELTA AND BOUNDARY CONDITIONS IS MONTH

VALUES OF EXTERNAL SWITCHES

LSW7 = 2
LSW8 = 2
LSW9 = 2

INITIAL WATER LEVEL ELEVATIONS

T = .0000

NODE	HINIT	NODE	HINIT	NODE	HINIT
1	6.7000	29	7.8000	57	7.1000
2	7.3000	30	8.2000	58	7.4000
3	7.2000	31	9.1000	59	8.3000
4	7.2000	32	9.5000	60	9.1000
5	7.9000	33	9.1000	61	8.7000
6	8.4000	34	9.0000	62	8.5000
7	8.9000	35	8.5000	63	8.5000
8	6.5400	36	8.0000	64	6.8000
9	7.0100	37	8.6000	65	7.2000
10	7.0700	38	9.6000	66	7.9000
11	7.0000	39	10.0000	67	8.3000
12	7.5500	40	9.4000	68	8.2000
13	8.1500	41	9.1000	69	8.0500
14	8.6050	42	8.9000	70	8.0600
15	6.5000	43	7.7000	71	6.6000
16	7.6000	44	8.4000	72	6.9000
17	8.0000	45	9.5000	73	7.4000
18	7.9000	46	9.8000	74	7.7000
19	8.0000	47	9.4000	75	7.8000
20	8.3000	48	9.2000	76	7.6000
21	8.4000	49	9.0000	77	7.7000
22	7.1900	50	7.3900	78	6.3600
23	7.9500	51	8.0000	79	6.7100
24	8.6000	52	8.9000	80	7.0000
25	8.4000	53	9.6000	81	7.1000
26	8.5000	54	9.1000	82	7.2500
27	8.6100	55	8.9000	83	7.1400
28	8.3000	56	8.9000	84	7.2500

Contd.

T = 1.0000 NO. OF ITERATIONS = 11 NO. OF SUBITERATIONS = 0

NODE	H	NODE	H	NODE	H
1	6.7000	29	7.8000	57	7.1000
2	7.3000	30	8.1984	58	7.5644
3	7.2000	31	8.7799	59	8.2329
4	7.2000	32	9.0169	60	8.6559
5	7.9000	33	8.9071	61	8.5829
6	8.4000	34	8.7829	62	8.4902
7	8.9000	35	8.5000	63	8.5000
8	6.5400	36	8.0000	64	6.8000
9	7.1580	37	8.4340	65	7.2535
10	7.3471	38	9.0839	66	7.7929
11	7.4522	39	9.3315	67	8.1120
12	7.8764	40	9.1336	68	8.1341
13	8.2927	41	8.9708	69	8.0699
14	8.6050	42	8.9000	70	8.0600
15	6.5000	43	7.7000	71	6.6000
16	7.4364	44	8.3218	72	6.9643
17	7.8413	45	9.0528	73	7.3713
18	7.9559	46	9.3143	74	7.6116
19	8.1458	47	9.1486	75	7.6993
20	8.3480	48	9.0132	76	7.6366
21	8.4000	49	9.0000	77	7.7000
22	7.1900	50	7.3900	78	6.3600
23	7.8421	51	7.9870	79	6.7100
24	8.3359	52	8.7058	80	7.0000
25	8.4597	53	9.0858	81	7.1000
26	8.5175	54	8.9436	82	7.2500
27	8.5252	55	8.8365	83	7.1400
28	8.3000	56	8.9000	84	7.2500

WATER BALANCE COMPONENTS

NODE NET RECHARGE CHANGE IN STORAGE SUBSURFACE FLOW ARTIFICIAL FLOW NODE
(ALL COMPONENTS ARE CALCULATED FOR THE LAST 1 TIME STEPS)

9	.0000	-.0015	.0015	.0000	9
10	.0000	-.0027	.0027	.0000	10
11	.0000	-.0044	.0044	.0000	11
12	.0000	-.0033	.0032	.0000	12
13	.0000	-.0014	.0014	.0000	13
16	.0000	.0016	-.0016	.0000	16
17	.0000	.0016	-.0016	.0000	17
18	.0000	-.0006	.0005	.0000	18
19	.0000	-.0015	.0015	.0000	19
20	.0000	-.0005	.0005	.0000	20
23	.0000	.0011	-.0011	.0000	23
24	.0000	.0026	-.0026	.0000	24
25	.0000	-.0006	.0006	.0000	25
26	.0000	-.0002	.0002	.0000	26
27	.0000	.0008	-.0008	.0000	27
30	.0009	.0000	-.0000	.0000	30
31	.0000	.0031	-.0031	.0000	31
32	.0000	.0047	-.0047	.0000	32
33	.0000	.0019	-.0019	.0000	33
34	.0000	.0021	-.0021	.0000	34

Contd.

37	.0000	.0016	-.0016	.0000	37
38	.0000	.0050	-.0050	.0000	38
39	.0000	.0065	-.0065	.0000	39
40	.0000	.0026	-.0026	.0000	40
41	.0000	.0012	-.0012	.0000	41
44	.0000	.0008	-.0008	.0000	44
45	.0000	.0044	-.0044	.0000	45
46	.0000	.0047	-.0048	.0000	46
47	.0000	.0025	-.0025	.0000	47
48	.0000	.0018	-.0018	.0000	48
51	.0000	.0001	-.0001	.0000	51
52	.0000	.0019	-.0019	.0000	52
53	.0000	.0051	-.0051	.0000	53
54	.0000	.0016	-.0016	.0000	54
55	.0000	.0006	-.0006	.0000	55
58	.0000	-.0016	.0016	.0000	58
59	.0000	.0007	-.0007	.0000	59
60	.0000	.0044	-.0044	.0000	60
61	.0000	.0012	-.0012	.0000	61
62	.0000	.0001	-.0001	.0000	62
65	.0000	-.0005	.0005	.0000	65
66	.0000	.0010	-.0010	.0000	66
67	.0000	.0018	-.0018	.0000	67
68	.0000	.0007	-.0007	.0000	68
69	.0000	-.0002	.0002	.0000	69
72	.0000	-.0006	.0006	.0000	72
73	.0000	.0003	-.0003	.0000	73
74	.0000	.0009	-.0009	.0000	74
75	.0000	.0010	-.0010	.0000	75
76	.0000	-.0004	.0004	.0000	76
TOTAL	.0000	.0523	-.0525	.0000	

T = 2.0000 NO. OF ITERATIONS = 8 NO. OF SUBITERATIONS = 0

NODE	H	NODE	H	NODE	H
1	6.7000	29	7.8000	57	7.1000
2	7.3000	30	8.1096	58	7.5888
3	7.2000	31	8.5311	59	8.1156
4	7.2000	32	8.7376	60	8.4101
5	7.9000	33	8.7441	61	8.4523
6	8.4000	34	8.6773	62	8.4445
7	8.9000	35	8.5000	63	8.5000
8	6.5400	36	8.0000	64	6.8000
9	7.1913	37	8.2963	65	7.2647
10	7.4253	38	8.7489	66	7.7116
11	7.5924	39	8.9506	67	7.9727
12	7.9896	40	8.9137	68	8.0521
13	8.3431	41	8.8579	69	8.0491
14	8.6050	42	8.9000	70	8.0600
15	6.5000	43	7.7000	71	6.6000
16	7.3723	44	8.2132	72	6.9785
17	7.7765	45	8.7413	73	7.3370
18	7.9812	46	8.9599	74	7.5450
19	8.1999	47	8.9301	75	7.6470
20	8.3662	48	8.8975	76	7.6310
21	8.4000	49	9.0000	77	7.7000

Contd.

22	7.1900	50	7.3900	78	6.3600
23	7.7547	51	7.9375	79	6.7100
24	8.1743	52	9.5004	80	7.0000
25	8.3802	53	8.7682	81	7.1000
26	8.4738	54	8.7672	82	7.2500
27	8.4798	55	8.7579	83	7.1400
28	8.3000	56	8.9000	84	7.2500

WATER BALANCE COMPONENTS

NODE NET RECHARGE CHANGE IN STORAGE SUBSURFACE FLOW ARTIFICIAL FLOW NODE
 (ALL COMPCNENTS ARE CALCULATED FOR THE LAST 1 TIME STEPS)

9	.0000	-.0003	.0003	.0000	9
10	.0000	-.0008	.0008	.0000	10
11	.0000	-.0014	.0014	.0000	11
12	.0000	-.0011	.0011	.0000	12
13	.0000	-.0005	.0005	.0000	13
16	.0000	.0006	-.0006	.0000	16
17	.0000	.0006	-.0006	.0000	17
18	.0000	-.0003	.0003	.0000	18
19	.0000	-.0005	.0006	.0000	19
20	.0000	-.0002	.0002	.0000	20
23	.0000	.0009	-.0009	.0000	23
24	.0000	.0016	-.0016	.0000	24
25	.0000	.0008	-.0008	.0000	25
26	.0000	.0004	-.0004	.0000	26
27	.0000	.0004	-.0004	.0000	27
30	.0000	.0009	-.0009	.0000	30
31	.0000	.0024	-.0024	.0000	31
32	.0000	.0027	-.0027	.0000	32
33	.0000	.0016	-.0016	.0000	33
34	.0000	.0010	-.0010	.0000	34
37	.0000	.0013	-.0013	.0000	37
38	.0000	.0032	-.0032	.0000	38
39	.0000	.0037	-.0037	.0000	39
40	.0000	.0022	-.0022	.0000	40
41	.0000	.0011	-.0011	.0000	41
44	.0000	.0011	-.0011	.0000	44
45	.0000	.0030	-.0030	.0000	45
46	.0000	.0035	-.0035	.0000	46
47	.0000	.0022	-.0022	.0000	47
48	.0000	.0011	-.0011	.0000	48
51	.0000	.0005	-.0005	.0000	51
52	.0000	.0020	-.0020	.0000	52
53	.0000	.0031	-.0031	.0000	53
54	.0000	.0018	-.0018	.0000	54
55	.0000	.0008	-.0008	.0000	55
58	.0000	-.0002	.0002	.0000	58
59	.0000	.0012	-.0012	.0000	59
60	.0000	.0024	-.0024	.0000	60
61	.0000	.0013	-.0013	.0000	61
62	.0000	.0005	-.0005	.0000	62
65	.0000	-.0001	.0001	.0000	65
66	.0000	.0008	-.0008	.0000	66
67	.0000	.0014	-.0014	.0000	67
68	.0000	.0008	-.0008	.0000	68
69	.0000	.0002	-.0002	.0000	69

Contd.

72	.0000	-.0001	.0001	.0000	72
73	.0000	.0003	-.0003	.0000	73
74	.0000	.0007	-.0007	.0000	74
75	.0000	.0005	-.0005	.0000	75
76	.0000	.0001	-.0001	.0000	76
TOTAL	.0000	.0493	-.0490	.0000	

T = 3.0000 NO. OF ITERATIONS = 8 NO. OF SUBITERATIONS = 0

NODE	H	NODE	H	NODE	H
1	6.7000	29	7.8000	57	7.1000
2	7.3000	30	8.0184	58	7.5630
3	7.2000	31	8.3485	59	8.0000
4	7.2000	32	8.5475	60	8.2433
5	7.9000	33	8.6156	61	8.3377
6	8.4900	34	8.6074	62	8.3931
7	8.9000	35	8.5000	63	8.5000
8	6.5400	36	8.0000	64	6.8000
9	7.1930	37	8.1823	65	7.2507
10	7.4396	38	8.5175	66	7.6397
11	7.6295	39	8.7021	67	7.8689
12	8.0233	40	8.7462	68	7.9773
13	8.3582	41	8.7723	69	8.0177
14	8.6050	42	8.9000	70	8.0600
15	6.5000	43	7.7000	71	6.6000
16	7.3337	44	8.1098	72	6.9741
17	7.7296	45	8.5138	73	7.3034
18	7.9679	46	8.7088	74	7.4940
19	8.2031	47	8.7573	75	7.6074
20	8.3653	48	8.8106	76	7.6162
21	8.4000	49	9.0000	77	7.7000
22	7.1900	50	7.3900	78	6.3600
23	7.6854	51	7.8716	79	6.7100
24	8.0580	52	8.3244	80	7.0000
25	8.2866	53	8.5477	81	7.1000
26	8.4139	54	8.6168	82	7.2500
27	8.4443	55	8.6860	83	7.1400
28	8.3000	56	8.9000	84	7.2500

WATER BALANCE COMPONENTS

NODE NET RECHARGE CHANGE IN STORAGE SUBSURFACE FLOW ARTIFICIAL FLOW NODE
(ALL COMPONENTS ARE CALCULATED FOR THE LAST 1 TIME STEPS)

9	.0000	-.0000	-.0000	.0000	9
10	.0000	-.0001	.0001	.0000	10
11	.0000	-.0004	.0004	.0000	11
12	.0000	-.0003	.0003	.0000	12
13	.0000	-.0001	.0002	.0000	13
16	.0000	.0004	-.0004	.0000	16
17	.0000	.0005	-.0005	.0000	17
18	.0000	.0001	-.0001	.0000	18
19	.0000	-.0000	.0000	.0000	19
20	.0000	.0000	.0000	.0000	20
23	.0000	.0007	-.0007	.0000	23
24	.0000	.0012	-.0011	.0000	24

Contd.

25	.0000	.0009	-.0009	.0000	25
26	.0000	.0006	-.0006	.0000	26
27	.0000	.0004	-.0003	.0000	27
30	.0000	.0009	-.0009	.0000	30
31	.0000	.0018	-.0018	.0000	31
32	.0000	.0019	-.0018	.0000	32
33	.0000	.0013	-.0013	.0000	33
34	.0000	.0007	-.0007	.0000	34
37	.0000	.0011	-.0011	.0000	37
38	.0000	.0022	-.0022	.0000	38
39	.0000	.0024	-.0024	.0000	39
40	.0000	.0016	-.0016	.0000	40
41	.0000	.0008	-.0008	.0000	41
44	.0000	.0010	-.0010	.0000	44
45	.0000	.0022	-.0022	.0000	45
46	.0000	.0025	-.0024	.0000	46
47	.0000	.0017	-.0017	.0000	47
48	.0000	.0008	-.0008	.0000	48
51	.0000	.0007	-.0007	.0000	51
52	.0000	.0017	-.0017	.0000	52
53	.0000	.0022	-.0022	.0000	53
54	.0000	.0015	-.0015	.0000	54
55	.0000	.0007	-.0007	.0000	55
58	.0000	.0003	-.0003	.0000	58
59	.0000	.0011	-.0011	.0000	59
60	.0000	.0017	-.0016	.0000	60
61	.0000	.0012	-.0012	.0000	61
62	.0000	.0005	-.0005	.0000	62
65	.0000	.0001	-.0001	.0000	65
66	.0000	.0007	-.0007	.0000	66
67	.0000	.0010	-.0010	.0000	67
68	.0000	.0007	-.0007	.0000	68
69	.0000	.0003	-.0003	.0000	69
72	.0000	.0000	-.0000	.0000	72
73	.0000	.0003	-.0003	.0000	73
74	.0000	.0005	-.0005	.0000	74
75	.0000	.0004	-.0004	.0000	75
76	.0000	.0001	-.0001	.0000	76
TOTAL	.0000	.0425	-.0423	.0000	

T = 4.0000 NO. OF ITERATIONS = 8 NO. OF SUBITERATIONS = 0

NODE	H	NODE	H	NODE	H
1	6.7000	29	7.8000	57	7.1000
2	7.3000	30	7.9415	58	7.5220
3	7.2000	31	8.2124	59	7.8989
4	7.2000	32	8.4077	60	8.1191
5	7.9000	33	8.5144	61	8.2436
6	8.4000	34	8.5546	62	8.3468
7	8.9000	35	8.5000	63	8.5000
8	6.5400	36	8.0000	64	6.8000
9	7.1854	37	8.0903	65	7.2264
10	7.4323	38	8.3499	66	7.5763
11	7.6303	39	8.5267	67	7.7884
12	8.0265	40	8.6194	68	7.9144
13	8.3593	41	8.7071	69	7.9870

Contd.

14	8.6050	42	8.9000	70	8.0600
15	6.5000	43	7.7000	71	6.6000
16	7.3030	44	8.0209	72	6.9629
17	7.6863	45	8.3434	73	7.2727
18	7.9361	46	8.5268	74	7.4537
19	8.1837	47	8.6244	75	7.5750
20	8.3547	48	8.7433	76	7.6009
21	8.4000	49	9.0000	77	7.7000
22	7.1900	50	7.3900	78	6.3600
23	7.6297	51	7.8048	79	6.7100
24	7.9676	52	8.1816	80	7.0000
25	8.2016	53	8.3855	81	7.1000
26	8.3539	54	8.4969	82	7.2500
27	8.4127	55	8.6262	83	7.1400
28	8.3000	56	8.9000	84	7.2500

WATER BALANCE COMPONENTS

NODE NET RECHARGE CHANGE IN STORAGE SUBSURFACE FLOW ARTIFICIAL FLOW NODE
 (ALL COMPONENTS ARE CALCULATED FOR THE LAST 1 TIME STEPS)

9	.0000	.0001	-.0001	.0000	9
10	.0000	.0001	-.0001	.0000	10
11	.0000	.0000	.0000	.0000	11
12	.0000	-.0000	.0000	.0000	12
13	.0000	-.0000	.0000	.0000	13
16	.0000	.0003	-.0003	.0000	16
17	.0000	.0004	-.0004	.0000	17
18	.0000	.0003	-.0003	.0000	18
19	.0000	.0002	-.0002	.0000	19
20	.0000	.0001	-.0001	.0000	20
23	.0000	.0006	-.0005	.0000	23
24	.0000	.0009	-.0009	.0000	24
25	.0000	.0008	-.0008	.0000	25
26	.0000	.0006	-.0006	.0000	26
27	.0000	.0003	-.0003	.0000	27
30	.0000	.0008	-.0008	.0000	30
31	.0000	.0013	-.0013	.0000	31
32	.0000	.0014	-.0014	.0000	32
33	.0000	.0010	-.0010	.0000	33
34	.0000	.0005	-.0005	.0000	34
37	.0000	.0009	-.0009	.0000	37
38	.0000	.0016	-.0016	.0000	38
39	.0000	.0017	-.0017	.0000	39
40	.0000	.0012	-.0012	.0000	40
41	.0000	.0006	-.0006	.0000	41
44	.0000	.0009	-.0009	.0000	44
45	.0000	.0017	-.0017	.0000	45
46	.0000	.0018	-.0018	.0000	46
47	.0000	.0013	-.0013	.0000	47
48	.0000	.0007	-.0007	.0000	48
51	.0000	.0007	-.0007	.0000	51
52	.0000	.0014	-.0014	.0000	52
53	.0000	.0016	-.0016	.0000	53
54	.0000	.0012	-.0012	.0000	54
55	.0000	.0006	-.0006	.0000	55
58	.0000	.0004	-.0004	.0000	58
59	.0000	.0010	-.0010	.0000	59

Contd.

60	.0000	.0012	-.0012	.0000	60
61	.0000	.0009	-.0009	.0000	61
62	.0000	.0005	-.0005	.0000	62
65	.0000	.0002	-.0002	.0000	65
66	.0000	.0006	-.0006	.0000	66
67	.0000	.0008	-.0008	.0000	67
68	.0000	.0006	-.0006	.0000	68
69	.0000	.0003	-.0003	.0000	69
72	.0000	.0001	-.0001	.0000	72
73	.0000	.0003	-.0003	.0000	73
74	.0000	.0004	-.0004	.0000	74
75	.0000	.0003	-.0003	.0000	75
76	.0000	.0002	-.0002	.0000	76
TOTAL	.0000	.0354	-.0352	.0000	

T = 5.0000 NO. OF ITERATIONS = 7 NO. OF SUBITERATIONS = 0

NODE	H	NODE	H	NODE	H
1	6.7000	29	7.8000	57	7.1000
2	7.3000	30	7.8793	58	7.4795
3	7.2000	31	8.1081	59	7.8140
4	7.2000	32	8.3004	60	8.0229
5	7.9000	33	8.4339	61	8.1683
6	8.4000	34	8.5127	62	8.3082
7	8.9000	35	8.5000	63	8.5000
8	6.5400	36	8.0000	64	6.8000
9	7.1753	37	8.0169	65	7.1998
10	7.4182	38	8.2241	66	7.5218
11	7.6186	39	8.3966	67	7.7244
12	8.0185	40	8.5222	68	7.8631
13	8.3550	41	8.6566	69	7.9605
14	8.6050	42	8.9000	70	8.0600
15	6.5000	43	7.7000	71	6.6000
16	7.2764	44	7.9474	72	6.9501
17	7.6461	45	8.2133	73	7.2459
18	7.8988	46	8.3908	74	7.4212
19	8.1566	47	8.5223	75	7.5485
20	8.3406	48	8.6909	76	7.5873
21	8.4000	49	9.0000	77	7.7000
22	7.1900	50	7.3900	78	6.3600
23	7.5843	51	7.7444	79	6.7100
24	7.8945	52	8.0674	80	7.0000
25	8.1285	53	8.2624	81	7.1000
26	8.2996	54	8.4030	82	7.2500
27	8.3845	55	8.5781	83	7.1400
28	8.3000	56	8.9000	84	7.2500

WATER BALANCE COMPONENTS

NODE NET RECHARGE CHANGE IN STORAGE SUBSURFACE FLOW ARTIFICIAL FLOW NODE
(ALL COMPONENTS ARE CALCULATED FOR THE LAST 1 TIME STEPS)

9	.0000	.0001	-.0001	.0000	9
10	.0000	.0001	-.0001	.0000	10
11	.0000	.0001	-.0001	.0000	11
12	.0000	.0001	-.0001	.0000	12

Contd.

13	.0000	.0000	-.0000	.0000	13
16	.0000	.0003	-.0003	.0000	16
17	.0000	.0004	-.0004	.0000	17
18	.0000	.0004	-.0004	.0000	18
19	.0000	.0003	-.0003	.0000	19
20	.0000	.0001	-.0001	.0000	20
23	.0000	.0005	-.0004	.0000	23
24	.0000	.0007	-.0007	.0000	24
25	.0000	.0007	-.0007	.0000	25
26	.0000	.0005	-.0005	.0000	26
27	.0000	.0003	-.0003	.0000	27
30	.0000	.0006	-.0006	.0000	30
31	.0000	.0010	-.0010	.0000	31
32	.0000	.0010	-.0010	.0000	32
33	.0000	.0008	-.0008	.0000	33
34	.0000	.0004	-.0004	.0000	34
37	.0000	.0007	-.0007	.0000	37
38	.0000	.0012	-.0012	.0000	38
39	.0000	.0013	-.0012	.0000	39
40	.0000	.0010	-.0009	.0000	40
41	.0000	.0005	-.0005	.0000	41
44	.0000	.0007	-.0007	.0000	44
45	.0000	.0013	-.0013	.0000	45
46	.0000	.0013	-.0013	.0000	46
47	.0000	.0010	-.0010	.0000	47
48	.0000	.0005	-.0005	.0000	48
51	.0000	.0006	-.0006	.0000	51
52	.0000	.0011	-.0011	.0000	52
53	.0000	.0012	-.0012	.0000	53
54	.0000	.0009	-.0009	.0000	54
55	.0000	.0005	-.0005	.0000	55
58	.0000	.0004	-.0004	.0000	58
59	.0000	.0008	-.0008	.0000	59
60	.0000	.0010	-.0009	.0000	60
61	.0000	.0008	-.0008	.0000	61
62	.0000	.0004	-.0004	.0000	62
65	.0000	.0003	-.0003	.0000	65
66	.0000	.0005	-.0005	.0000	66
67	.0000	.0006	-.0006	.0000	67
68	.0000	.0005	-.0005	.0000	68
69	.0000	.0003	-.0003	.0000	69
72	.0000	.0001	-.0001	.0000	72
73	.0000	.0003	-.0003	.0000	73
74	.0000	.0003	-.0003	.0000	74
75	.0000	.0003	-.0003	.0000	75
76	.0000	.0001	-.0001	.0000	76
TOTAL	.0000	.0291	-.0288	.0000	

T = 6.0000 NO. OF ITERATIONS = 7 NO. OF SUBITERATIONS = 0

NODE	H	NODE	H	NODE	H
1	6.7000	29	7.8000	57	7.1000
2	7.3000	30	7.8299	58	7.4410
3	7.2000	31	8.0272	59	7.7444
4	7.2000	32	8.2168	60	7.9474
5	7.9000	33	8.3700	61	8.1087

Contd.

6	8.4000	34	8.4792	62	8.2771
7	8.9000	35	8.5000	63	8.5000
8	6.5400	36	8.0000	64	6.8000
9	7.1655	37	7.9589	65	7.1749
10	7.4030	38	8.1281	66	7.4762
11	7.6037	39	8.2977	67	7.6734
12	8.0071	40	8.4471	68	7.8217
13	8.3490	41	8.6173	69	7.9387
14	8.6050	42	8.9000	70	8.0600
15	6.5000	43	7.7000	71	6.6000
16	7.2538	44	7.8880	72	6.9378
17	7.6105	45	8.1129	73	7.2230
18	7.8631	46	8.2874	74	7.3949
19	8.1291	47	8.4435	75	7.5269
20	8.3263	48	8.6501	76	7.5759
21	8.4000	49	9.0000	77	7.7000
22	7.1900	50	7.3900	78	6.3600
23	7.5474	51	7.6931	79	6.7100
24	7.8354	52	7.9768	80	7.0000
25	8.0676	53	8.1675	81	7.1000
26	8.2530	54	8.3298	82	7.2500
27	8.3603	55	8.5401	83	7.1400
28	8.3000	56	8.9000	84	7.2500

WATER BALANCE COMPONENTS

NODE NET RECHARGE CHANGE IN STORAGE SUBSURFACE FLOW ARTIFICIAL FLOW NODE
 (ALL COMPONENTS ARE CALCULATED FOR THE LAST 1 TIME STEPS)

9	.0000	.0001	-.0001	.0000	9
10	.0000	.0001	-.0001	.0000	10
11	.0000	.0001	-.0001	.0000	11
12	.0000	.0001	-.0001	.0000	12
13	.0000	.0001	-.0001	.0000	13
16	.0000	.0002	-.0002	.0000	16
17	.0000	.0004	-.0003	.0000	17
18	.0000	.0004	-.0003	.0000	18
19	.0000	.0003	-.0003	.0000	19
20	.0000	.0001	-.0001	.0000	20
23	.0000	.0004	-.0004	.0000	23
24	.0000	.0006	-.0006	.0000	24
25	.0000	.0006	-.0006	.0000	25
26	.0000	.0005	-.0005	.0000	26
27	.0000	.0002	-.0002	.0000	27
30	.0000	.0005	-.0005	.0000	30
31	.0000	.0008	-.0008	.0000	31
32	.0000	.0008	-.0008	.0000	32
33	.0000	.0006	-.0006	.0000	33
34	.0000	.0003	-.0003	.0000	34
37	.0000	.0006	-.0006	.0000	37
38	.0000	.0009	-.0009	.0000	38
39	.0000	.0010	-.0009	.0000	39
40	.0000	.0007	-.0007	.0000	40
41	.0000	.0004	-.0004	.0000	41
44	.0000	.0006	-.0006	.0000	44
45	.0000	.0010	-.0010	.0000	45
46	.0000	.0010	-.0010	.0000	46
47	.0000	.0008	-.0008	.0000	47

Contd.

48	.0000	.0004	-.0004	.0000	18
51	.0000	.0005	-.0005	.0000	51
52	.0000	.0009	-.0009	.0000	52
53	.0000	.0009	-.0009	.0000	53
54	.0000	.0007	-.0007	.0000	54
55	.0000	.0004	-.0004	.0000	55
58	.0000	.0004	-.0004	.0000	58
59	.0000	.0007	-.0007	.0000	59
60	.0000	.0007	-.0007	.0000	60
61	.0000	.0006	-.0006	.0000	61
62	.0000	.0003	-.0003	.0000	62
65	.0000	.0002	-.0002	.0000	65
66	.0000	.0004	-.0004	.0000	66
67	.0000	.0005	-.0005	.0000	67
68	.0000	.0004	-.0004	.0000	68
69	.0000	.0002	-.0002	.0000	69
72	.0000	.0001	-.0001	.0000	72
73	.0000	.0002	-.0002	.0000	73
74	.0000	.0003	-.0003	.0000	74
75	.0000	.0002	-.0002	.0000	75
76	.0000	.0001	-.0001	.0000	76
TOTAL	.0000	.0235	-.0233	.0000	

T = 7.0000 NO. OF ITERATIONS = 6 NO. OF SUBITERATIONS = 0

NODE	H	NODE	H	NODE	H
1	6.7000	29	7.8000	57	7.1000
2	7.3000	30	7.7904	58	7.4080
3	7.2000	31	7.9632	59	7.6879
4	7.2000	32	8.1505	60	7.8876
5	7.9000	33	8.3187	61	8.0614
6	8.4000	34	8.4522	62	8.2522
7	8.9000	35	8.5000	63	8.5000
8	6.5400	36	8.0000	64	6.8000
9	7.1565	37	7.9129	65	7.1530
10	7.3887	38	8.0531	66	7.4385
11	7.5890	39	8.2209	67	7.6324
12	7.9956	40	8.3882	68	7.7886
13	8.3429	41	8.5862	69	7.9211
14	8.6050	42	8.9000	70	8.0600
15	6.5000	43	7.7000	71	6.6000
16	7.2344	44	7.8402	72	6.9268
17	7.5796	45	8.0344	73	7.2039
18	7.8312	46	8.2073	74	7.3737
19	8.1041	47	8.3821	75	7.5096
20	8.3132	48	8.6180	76	7.5667
21	8.4000	49	9.0000	77	7.7000
22	7.1900	50	7.3900	78	6.3600
23	7.5173	51	7.6506	79	6.7100
24	7.7872	52	7.9047	80	7.0000
25	8.0170	53	8.0933	81	7.1000
26	8.2137	54	8.2724	82	7.2500
27	8.3398	55	8.5101	83	7.1400
28	8.3000	56	8.9000	84	7.2500

WATER BALANCE COMPONENTS

Contd.

NODE NET RECHARGE CHANGE IN STORAGE SUBSURFACE FLOW ARTIFICIAL FLOW NODE
 (ALL COMPONENTS ARE CALCULATED FOR THE LAST 1 TIME STBPS)

9	.0000	.0001	-.0001	.0000	9
10	.0000	.0001	-.0001	.0000	10
11	.0000	.0001	-.0001	.0000	11
12	.0000	.0001	-.0001	.0000	12
13	.0000	.0001	-.0001	.0000	13
16	.0000	.0002	-.0002	.0000	16
17	.0000	.0003	-.0003	.0000	17
18	.0000	.0003	-.0003	.0000	18
19	.0000	.0003	-.0002	.0000	19
20	.0000	.0001	-.0001	.0000	20
23	.0000	.0003	-.0003	.0000	23
24	.0000	.0005	-.0005	.0000	24
25	.0000	.0005	-.0005	.0000	25
26	.0000	.0004	-.0004	.0000	26
27	.0000	.0002	-.0002	.0000	27
30	.0000	.0004	-.0004	.0000	30
31	.0000	.0006	-.0006	.0000	31
32	.0000	.0006	-.0006	.0000	32
33	.0000	.0005	-.0005	.0000	33
34	.0000	.0003	-.0003	.0000	34
37	.0000	.0004	-.0004	.0000	37
38	.0000	.0007	-.0007	.0000	38
39	.0000	.0007	-.0007	.0000	39
40	.0000	.0006	-.0006	.0000	40
41	.0000	.0003	-.0003	.0000	41
44	.0000	.0005	-.0005	.0000	44
45	.0000	.0008	-.0008	.0000	45
46	.0000	.0008	-.0008	.0000	46
47	.0000	.0006	-.0006	.0000	47
48	.0000	.0003	-.0003	.0000	48
51	.0000	.0004	-.0004	.0000	51
52	.0000	.0007	-.0007	.0000	52
53	.0000	.0007	-.0007	.0000	53
54	.0000	.0006	-.0006	.0000	54
55	.0000	.0003	-.0003	.0000	55
58	.0000	.0003	-.0003	.0000	58
59	.0000	.0006	-.0006	.0000	59
60	.0000	.0006	-.0006	.0000	60
61	.0000	.0005	-.0005	.0000	61
62	.0000	.0002	-.0002	.0000	62
65	.0000	.0002	-.0002	.0000	65
66	.0000	.0004	-.0004	.0000	66
67	.0000	.0004	-.0004	.0000	67
68	.0000	.0003	-.0003	.0000	68
69	.0000	.0002	-.0002	.0000	69
72	.0000	.0001	-.0001	.0000	72
73	.0000	.0002	-.0002	.0000	73
74	.0000	.0002	-.0002	.0000	74
75	.0000	.0002	-.0002	.0000	75
76	.0000	.0001	-.0001	.0000	76
TOTAL	.0000	.0190	-.0187	.0000	

T = 8.0000 NO. OF ITERATIONS = 6 NO. OF SUBITERATIONS = 0

Contd.

NODE	H	NODE	H	NODE	H
1	6.7000	29	7.8000	57	7.1000
2	7.3000	30	7.7591	58	7.3807
3	7.2000	31	7.9128	59	7.6425
4	7.2000	32	8.0981	60	7.8403
5	7.9000	33	8.2780	61	8.0242
6	8.4000	34	8.4307	62	8.2326
7	8.9000	35	8.5000	63	8.5000
8	6.5400	36	8.0000	64	6.8000
9	7.1488	37	7.8766	65	7.1347
10	7.3763	38	7.9946	66	7.4078
11	7.5759	39	8.1609	67	7.5998
12	7.9853	40	8.3420	68	7.7624
13	8.3374	41	8.5618	69	7.9071
14	8.6050	42	8.9000	70	8.0600
15	6.5000	43	7.7000	71	6.6000
16	7.2184	44	7.8023	72	6.9175
17	7.5538	45	7.9729	73	7.1883
18	7.8040	46	8.1449	74	7.3567
19	8.0825	47	8.3342	75	7.4957
20	8.3020	48	8.5929	76	7.5593
21	8.4000	49	9.0000	77	7.7000
22	7.1900	50	7.3900	78	6.3600
23	7.4931	51	7.6161	79	6.7100
24	7.7482	52	7.8476	80	7.0000
25	7.9760	53	8.0353	81	7.1000
26	8.1816	54	8.2275	82	7.2500
27	8.3229	55	8.4866	83	7.1400
28	8.3000	56	8.9000	84	7.2500

WATER BALANCE COMPONENTS

NODE NET RECHARGE CHANGE IN STORAGE SUBSURFACE FLOW ARTIFICIAL FLOW NODE
 (ALL COMPONENTS ARE CALCULATED FOR THE LAST 1 TIME STBPS)

9	.0000	.0001	-.0001	.0000	9
10	.0000	.0001	-.0001	.0000	10
11	.0000	.0001	-.0001	.0000	11
12	.0000	.0001	-.0001	.0000	12
13	.0000	.0001	-.0001	.0000	13
16	.0000	.0002	-.0002	.0000	16
17	.0000	.0003	-.0002	.0000	17
18	.0000	.0003	-.0003	.0000	18
19	.0000	.0002	-.0002	.0000	19
20	.0000	.0001	-.0001	.0000	20
23	.0000	.0002	-.0002	.0000	23
24	.0000	.0004	-.0004	.0000	24
25	.0000	.0004	-.0004	.0000	25
26	.0000	.0003	-.0003	.0000	26
27	.0000	.0002	-.0002	.0000	27
30	.0000	.0003	-.0003	.0000	30
31	.0000	.0005	-.0005	.0000	31
32	.0000	.0005	-.0005	.0000	32
33	.0000	.0004	-.0004	.0000	33
34	.0000	.0002	-.0002	.0000	34
37	.0000	.0004	-.0003	.0000	37
38	.0000	.0006	-.0006	.0000	38

Contd.

39	.0000	.0006	-.0006	.0000	39
40	.0000	.0005	-.0004	.0000	40
41	.0000	.0002	-.0002	.0000	41
44	.0000	.0004	-.0004	.0000	44
45	.0000	.0006	-.0006	.0000	45
46	.0000	.0006	-.0006	.0000	46
47	.0000	.0005	-.0005	.0000	47
48	.0000	.0002	-.0002	.0000	48
51	.0000	.0003	-.0003	.0000	51
52	.0000	.0006	-.0006	.0000	52
53	.0000	.0006	-.0006	.0000	53
54	.0000	.0005	-.0004	.0000	54
55	.0000	.0002	-.0002	.0000	55
58	.0000	.0003	-.0003	.0000	58
59	.0000	.0004	-.0004	.0000	59
60	.0000	.0005	-.0005	.0000	60
61	.0000	.0004	-.0004	.0000	61
62	.0000	.0002	-.0002	.0000	62
65	.0000	.0002	-.0002	.0000	65
66	.0000	.0003	-.0003	.0000	66
67	.0000	.0003	-.0003	.0000	67
68	.0000	.0003	-.0003	.0000	68
69	.0000	.0001	-.0001	.0000	69
72	.0000	.0001	-.0001	.0000	72
73	.0000	.0002	-.0002	.0000	73
74	.0000	.0002	-.0002	.0000	74
75	.0000	.0001	-.0001	.0000	75
76	.0000	.0001	-.0001	.0000	76
TOTAL	.0000	.0152	-.0149	.0000	

T = 9.0000 NO. OF ITERATIONS = 6 NO. OF SUBITERATIONS = 0

NODE	H	NODE	H	NODE	H
1	6.7000	29	7.8000	57	7.1000
2	7.3000	30	7.7343	58	7.3584
3	7.2000	31	7.8728	59	7.6062
4	7.2000	32	8.0565	60	7.8029
5	7.9000	33	8.2456	61	7.9948
6	8.4000	34	8.4135	62	8.2171
7	8.9000	35	8.5000	63	8.5000
8	6.5400	36	8.0000	64	6.8000
9	7.1424	37	7.8479	65	7.1195
10	7.3658	38	7.9486	66	7.3830
11	7.5648	39	8.1139	67	7.5737
12	7.9764	40	8.3057	68	7.7415
13	8.3328	41	8.5426	69	7.8960
14	8.6050	42	8.9000	70	8.0600
15	6.5000	43	7.7000	71	6.6000
16	7.2052	44	7.7722	72	6.9098
17	7.5326	45	7.9246	73	7.1756
18	7.7816	46	8.0961	74	7.3430
19	8.0646	47	8.2967	75	7.4846
20	8.2926	48	8.5732	76	7.5535
21	8.4000	49	9.0000	77	7.7000
22	7.1900	50	7.3900	78	6.3600
23	7.4736	51	7.5884	79	6.7100

Contd.

24	7.7169	52	7.8024	80	7.0000
25	7.9428	53	7.9897	81	7.1000
26	8.1555	54	8.1922	82	7.2500
27	8.3092	55	8.4681	83	7.1400
28	8.3000	56	8.9000	84	7.2500

WATER BALANCE COMPONENTS

NODE NET RECHARGE CHANGE IN STORAGE SUBSURFACE FLOW ARTIFICIAL FLOW NODE
 (ALL COMPONENTS ARE CALCULATED FOR THE LAST 1 TIME STEPS)

9	.0000	.0001	-.0001	.0000	9
10	.0000	.0001	-.0001	.0000	10
11	.0000	.0001	-.0001	.0000	11
12	.0000	.0001	-.0001	.0000	12
13	.0000	.0000	-.0000	.0000	13
16	.0000	.0001	-.0001	.0000	16
17	.0000	.0002	-.0002	.0000	17
18	.0000	.0002	-.0002	.0000	18
19	.0000	.0002	-.0002	.0000	19
20	.0000	.0001	-.0001	.0000	20
23	.0000	.0002	-.0002	.0000	23
24	.0000	.0003	-.0003	.0000	24
25	.0000	.0003	-.0003	.0000	25
26	.0000	.0003	-.0003	.0000	26
27	.0000	.0001	-.0001	.0000	27
30	.0000	.0002	-.0002	.0000	30
31	.0000	.0004	-.0004	.0000	31
32	.0000	.0004	-.0004	.0000	32
33	.0000	.0003	-.0003	.0000	33
34	.0000	.0002	-.0002	.0000	34
37	.0000	.0003	-.0003	.0000	37
38	.0000	.0004	-.0004	.0000	38
39	.0000	.0005	-.0004	.0000	39
40	.0000	.0004	-.0004	.0000	40
41	.0000	.0002	-.0002	.0000	41
44	.0000	.0003	-.0003	.0000	44
45	.0000	.0005	-.0005	.0000	45
46	.0000	.0005	-.0005	.0000	46
47	.0000	.0004	-.0004	.0000	47
48	.0000	.0002	-.0002	.0000	48
51	.0000	.0003	-.0003	.0000	51
52	.0000	.0004	-.0004	.0000	52
53	.0000	.0005	-.0004	.0000	53
54	.0000	.0004	-.0004	.0000	54
55	.0000	.0002	-.0002	.0000	55
58	.0000	.0002	-.0002	.0000	58
59	.0000	.0004	-.0004	.0000	59
60	.0000	.0004	-.0004	.0000	60
61	.0000	.0003	-.0003	.0000	61
62	.0000	.0002	-.0002	.0000	62
65	.0000	.0001	-.0001	.0000	65
66	.0000	.0002	-.0002	.0000	66
67	.0000	.0003	-.0003	.0000	67
68	.0000	.0002	-.0002	.0000	68
69	.0000	.0001	-.0001	.0000	69
72	.0000	.0001	-.0001	.0000	72
73	.0000	.0001	-.0001	.0000	73

Contd.

74	.0000	.0001	-.0001	.0000	74
75	.0000	.0001	-.0001	.0000	75
76	.0000	.0001	-.0001	.0000	76
TOTAL	.0000	.0121	-.0119	.0000	

T = 10.0000 NO. OF ITERATIONS = 5 NO. OF SUBITERATIONS = 0

NODE	H	NODE	H	NODE	H
1	6.7000	29	7.8000	57	7.1000
2	7.3000	30	7.7143	58	7.3403
3	7.2000	31	7.8407	59	7.5770
4	7.2000	32	8.0231	60	7.7730
5	7.9000	33	8.2194	61	7.9714
6	8.4000	34	8.3996	62	8.2048
7	8.9000	35	8.5000	63	8.5000
8	6.5400	36	8.0000	64	6.8000
9	7.1370	37	7.8250	65	7.1072
10	7.3571	38	7.9119	66	7.3631
11	7.5555	39	8.0763	67	7.5529
12	7.9690	40	8.2766	68	7.7249
13	8.3288	41	8.5272	69	7.8871
14	8.6050	42	8.9000	70	8.0600
15	6.5000	43	7.7000	71	6.6000
16	7.1944	44	7.7481	72	6.9035
17	7.5151	45	7.8862	73	7.1653
18	7.7629	46	8.0572	74	7.3321
19	8.0498	47	8.2668	75	7.4758
20	8.2847	48	8.5576	76	7.5488
21	8.4000	49	9.0000	77	7.7000
22	7.1900	50	7.3900	78	6.3600
23	7.4577	51	7.5661	79	6.7100
24	7.6913	52	7.7664	80	7.0000
25	7.9158	53	7.9534	81	7.1000
26	8.1342	54	8.1642	82	7.2500
27	8.2980	55	8.4534	83	7.1400
28	8.3000	56	8.9000	84	7.2500

WATER BALANCE COMPONENTS

NODE NET RECHARGE CHANGE IN STORAGE SUBSURFACE FLOW ARTIFICIAL FLOW NODE
 (ALL COMPONENTS ARE CALCULATED FOR THE LAST 1 TIME STEPS)

9	.0000	.0001	-.0000	.0000	9
10	.0000	.0001	-.0001	.0000	10
11	.0000	.0001	-.0001	.0000	11
12	.0000	.0001	-.0001	.0000	12
13	.0000	.0000	-.0000	.0000	13
16	.0000	.0001	-.0001	.0000	16
17	.0000	.0002	-.0002	.0000	17
18	.0000	.0002	-.0002	.0000	18
19	.0000	.0002	-.0001	.0000	19
20	.0000	.0001	-.0001	.0000	20
23	.0000	.0002	-.0002	.0000	23
24	.0000	.0003	-.0002	.0000	24
25	.0000	.0003	-.0003	.0000	25
26	.0000	.0002	-.0002	.0000	26

Contd.

27	.0000	.0001	-.0001	.0000	27
30	.0000	.0002	-.0002	.0000	30
31	.0000	.0003	-.0003	.0000	31
32	.0000	.0003	-.0003	.0000	32
33	.0000	.0003	-.0003	.0000	33
34	.0000	.0001	-.0001	.0000	34
37	.0000	.0002	-.0002	.0000	37
38	.0000	.0004	-.0003	.0000	38
39	.0000	.0004	-.0004	.0000	39
40	.0000	.0003	-.0003	.0000	40
41	.0000	.0001	-.0001	.0000	41
44	.0000	.0002	-.0002	.0000	44
45	.0000	.0004	-.0004	.0000	45
46	.0000	.0004	-.0004	.0000	46
47	.0000	.0003	-.0003	.0000	47
48	.0000	.0002	-.0002	.0000	48
51	.0000	.0002	-.0002	.0000	51
52	.0000	.0004	-.0003	.0000	52
53	.0000	.0004	-.0004	.0000	53
54	.0000	.0003	-.0003	.0000	54
55	.0000	.0001	-.0001	.0000	55
58	.0000	.0002	-.0002	.0000	58
59	.0000	.0003	-.0003	.0000	59
60	.0000	.0003	-.0003	.0000	60
61	.0000	.0002	-.0002	.0000	61
62	.0000	.0001	-.0001	.0000	62
65	.0000	.0001	-.0001	.0000	65
66	.0000	.0002	-.0002	.0000	66
67	.0000	.0002	-.0002	.0000	67
68	.0000	.0002	-.0002	.0000	68
69	.0000	.0001	-.0001	.0000	69
72	.0000	.0001	-.0001	.0000	72
73	.0000	.0001	-.0001	.0000	73
74	.0000	.0001	-.0001	.0000	74
75	.0000	.0001	-.0001	.0000	75
76	.0000	.0000	-.0000	.0000	76

TOTAL .0000 .0098 -.0095 .0000

T = 11.0000 NO. OF ITERATIONS = 5 NO. OF SUBITERATIONS = 0

NODE	H	NODE	H	NODE	H
1	6.7000	29	7.8000	57	7.1000
2	7.3000	30	7.6984	58	7.3268
3	7.2000	31	7.8152	59	7.5538
4	7.2000	32	7.9965	60	7.7494
5	7.9000	33	8.1986	61	7.9530
6	8.4000	34	8.3886	62	8.1951
7	8.9000	35	8.5000	63	8.5000
8	6.5400	36	8.0000	64	6.8000
9	7.1327	37	7.8068	65	7.0973
10	7.3500	38	7.8829	66	7.3471
11	7.5478	39	8.0466	67	7.5363
12	7.9629	40	8.2537	68	7.7117
13	8.3256	41	8.5150	69	7.8801
14	8.6050	42	8.9000	70	8.0600
15	6.5000	43	7.7000	71	6.6000

Contd.

16	7.1857	44	7.7290	72	6.8984
17	7.5009	45	7.8558	73	7.1571
18	7.7479	46	8.0266	74	7.3233
19	8.0377	47	8.2433	75	7.4688
20	8.2784	48	8.5452	76	7.5451
21	8.4000	49	9.0000	77	7.7000
22	7.1900	50	7.3900	78	6.3600
23	7.4450	51	7.5484	79	6.7100
24	7.6709	52	7.7378	80	7.0000
25	7.8941	53	7.9247	81	7.1000
26	8.1171	54	8.1421	82	7.2500
27	8.2890	55	8.4419	83	7.1400
28	8.3000	56	8.9000	84	7.2500

WATER BALANCE COMPONENTS

NODE NET RECHARGE CHANGE IN STORAGE SUBSURFACE FLOW ARTIFICIAL FLOW NODE
 (ALL COMPONENTS ARE CALCULATED FOR THE LAST 1 TIME STEPS)

9	.0000	.0000	-.0000	.0000	9
10	.0000	.0001	-.0001	.0000	10
11	.0000	.0001	-.0001	.0000	11
12	.0000	.0001	-.0001	.0000	12
13	.0000	.0000	-.0000	.0000	13
16	.0000	.0001	-.0001	.0000	16
17	.0000	.0001	-.0001	.0000	17
18	.0000	.0001	-.0001	.0000	18
19	.0000	.0001	-.0001	.0000	19
20	.0000	.0001	-.0001	.0000	20
23	.0000	.0001	-.0001	.0000	23
24	.0000	.0002	-.0002	.0000	24
25	.0000	.0002	-.0002	.0000	25
26	.0000	.0002	-.0002	.0000	26
27	.0000	.0001	-.0001	.0000	27
30	.0000	.0002	-.0001	.0000	30
31	.0000	.0002	-.0002	.0000	31
32	.0000	.0003	-.0002	.0000	32
33	.0000	.0002	-.0002	.0000	33
34	.0000	.0001	-.0001	.0000	34
37	.0000	.0002	-.0002	.0000	37
38	.0000	.0003	-.0003	.0000	38
39	.0000	.0003	-.0003	.0000	39
40	.0000	.0002	-.0002	.0000	40
41	.0000	.0001	-.0001	.0000	41
44	.0000	.0002	-.0002	.0000	44
45	.0000	.0003	-.0003	.0000	45
46	.0000	.0003	-.0003	.0000	46
47	.0000	.0002	-.0002	.0000	47
48	.0003	.0001	-.0001	.0000	48
51	.0000	.0002	-.0002	.0000	51
52	.0000	.0003	-.0003	.0000	52
53	.0000	.0003	-.0003	.0000	53
54	.0000	.0002	-.0002	.0000	54
55	.0000	.0001	-.0001	.0000	55
58	.0000	.0001	-.0001	.0000	58
59	.0009	.0002	-.0002	.0000	59
60	.0000	.0002	-.0002	.0000	60
61	.0000	.0002	-.0002	.0000	61

Contd.

62	.0000	.0001	-.0001	.0000	62
65	.0000	.0001	-.0001	.0000	65
66	.0000	.0002	-.0002	.0000	66
67	.0000	.0002	-.0002	.0000	67
68	.0000	.0001	-.0001	.0000	68
69	.0000	.0001	-.0001	.0000	69
72	.0000	.0001	-.0000	.0000	72
73	.0000	.0001	-.0001	.0000	73
74	.0000	.0001	-.0001	.0000	74
75	.0000	.0001	-.0001	.0000	75
76	.0000	.0000	-.0000	.0000	76
TOTAL	.0000	.0078	-.0075	.0000	

T = 12.0000 NO. OF ITERATIONS = 5 NO. OF SUBITERATIONS = 0

NODE	H	NODE	H	NODE	H
1	6.7000	29	7.8000	57	7.1000
2	7.3000	30	7.6858	58	7.3143
3	7.2000	31	7.7949	59	7.5354
4	7.2000	32	7.9755	60	7.7307
5	7.9000	33	8.1822	61	7.9384
6	8.4000	34	8.3798	62	8.1875
7	8.9000	35	8.5000	63	8.5000
8	6.5400	36	8.0000	64	6.8000
9	7.1292	37	7.7924	65	7.0893
10	7.3442	38	7.8599	66	7.3344
11	7.5417	39	8.0232	67	7.5232
12	7.9580	40	8.2356	68	7.7013
13	8.3230	41	8.5054	69	7.8746
14	8.6050	42	8.9000	70	8.0600
15	6.5000	43	7.7000	71	6.6000
16	7.1787	44	7.7139	72	6.8944
17	7.4896	45	7.8318	73	7.1505
18	7.7358	46	8.0025	74	7.3164
19	8.0280	47	8.2247	75	7.4633
20	8.2733	48	8.5355	76	7.5421
21	8.4000	49	9.0000	77	7.7000
22	7.1900	50	7.3900	78	6.3600
23	7.4348	51	7.5343	79	6.7100
24	7.6547	52	7.7152	80	7.0000
25	7.8769	53	7.9022	81	7.1000
26	8.1035	54	8.1247	82	7.2500
27	8.2818	55	8.4328	83	7.1400
28	8.3000	56	8.9000	84	7.2500

WATER BALANCE COMPONENTS

NODE NET RECHARGE CHANGE IN STORAGE SUBSURFACE FLOW ARTIFICIAL FLOW NODE
(ALL COMPONENTS ARE CALCULATED FOR THE LAST 1 TIME STEPS)

9	.0000	.0000	-.0000	.0000	9
10	.0000	.0001	-.0001	.0000	10
11	.0000	.0001	-.0001	.0000	11
12	.0000	.0000	-.0000	.0000	12
13	.0000	.0000	-.0000	.0000	13
16	.0000	.0001	-.0001	.0000	16

Contd.

17	.0000	.0001	-.0001	.0000	17
18	.0000	.0001	-.0001	.0000	18
19	.0000	.0001	-.0001	.0000	19
20	.0000	.0001	-.0000	.0000	20
23	.0000	.0001	-.0001	.0000	23
24	.0000	.0002	-.0002	.0000	24
25	.0000	.0002	-.0002	.0000	25
26	.0000	.0001	-.0001	.0000	26
27	.0000	.0001	-.0001	.0000	27
30	.0000	.0001	-.0001	.0000	30
31	.0000	.0002	-.0002	.0000	31
32	.0000	.0002	-.0002	.0000	32
33	.0000	.0002	-.0002	.0000	33
34	.0000	.0001	-.0001	.0000	34
37	.0000	.0001	-.0001	.0000	37
38	.0000	.0002	-.0002	.0000	38
39	.0000	.0002	-.0002	.0000	39
40	.0000	.0002	-.0002	.0000	40
41	.0000	.0001	-.0001	.0000	41
44	.0000	.0001	-.0001	.0000	44
45	.0000	.0002	-.0002	.0000	45
46	.0000	.0002	-.0002	.0000	46
47	.0000	.0002	-.0002	.0000	47
48	.0000	.0001	-.0001	.0000	48
51	.0000	.0001	-.0001	.0000	51
52	.0000	.0002	-.0002	.0000	52
53	.0000	.0002	-.0002	.0000	53
54	.0000	.0002	-.0002	.0000	54
55	.0000	.0001	-.0001	.0000	55
58	.0000	.0001	-.0001	.0000	58
59	.0000	.0002	-.0002	.0000	59
60	.0000	.0002	-.0002	.0000	60
61	.0000	.0001	-.0001	.0000	61
62	.0000	.0001	-.0001	.0000	62
65	.0000	.0001	-.0001	.0000	65
66	.0000	.0001	-.0001	.0000	66
67	.0000	.0001	-.0001	.0000	67
68	.0000	.0001	-.0001	.0000	68
69	.0000	.0001	-.0001	.0000	69
72	.0000	.0000	-.0000	.0000	72
73	.0000	.0001	-.0001	.0000	73
74	.0000	.0001	-.0001	.0000	74
75	.0000	.0001	-.0001	.0000	75
76	.0000	.0000	-.0000	.0000	76
TOTAL	.0000	.0062	-.0060	.0000	

APPENDIX C

1. Format of output files for inverse run

2. Format of output file SGWP13.OUT

GROUNDWATER MODEL FOR MONTHLY DATA OF HARYANA
HANDLING SGWP11.INP

PRINTING OF CALCULATED WATER LEVELS AND BALANCES

TOTAL NUMBER OF NODES IS 84

UNIT LENGTH : 1 METRE

UNIT AREA : 1,000,000 SQ .METRES

UNIT VOLUME : 1,000,000 CU. METRES

UNIT OF TIME FOR DELTA AND BOUNDARY CONDITIONS IS MONTH

VALUES OF EXTERNAL SWITCHES

LSW7 = 2

LSW8 = 2

LSW9 = 2

INITIAL WATER LEVEL ELEVATIONS

T = .0000

NODE	HINIT	NODE	HINIT	NODE	HINIT
1	6.7000	29	7.8000	57	7.1000
2	7.3000	30	8.2000	58	7.4000
3	7.2000	31	9.1000	59	8.3000
4	7.2000	32	9.5000	60	9.1000
5	7.9000	33	9.1000	61	8.7000
6	8.4000	34	9.0000	62	8.5000
7	8.9000	35	8.5000	63	8.5000
8	6.5400	36	8.0000	64	6.8000
9	7.0100	37	8.6000	65	7.2000
10	7.0700	38	9.6000	66	7.9000
11	7.0000	39	10.0000	67	8.3000
12	7.5500	40	9.4000	68	8.2000
13	8.1500	41	9.1000	69	8.0500
14	8.6050	42	8.9000	70	8.0600
15	6.5000	43	7.7000	71	6.6000
16	7.6000	44	8.4000	72	6.9000
17	8.0000	45	9.5000	73	7.4000
18	7.9000	46	9.8000	74	7.7000
19	8.0000	47	9.4000	75	7.8000
20	8.3000	48	9.2000	76	7.6000
21	8.4000	49	9.0000	77	7.7000
22	7.1900	50	7.3900	78	6.3600
23	7.9500	51	8.0000	79	6.7100
24	8.6000	52	8.9000	80	7.0000
25	8.4000	53	9.6000	81	7.1000
26	8.5000	54	9.1000	82	7.2500

Contd.

27	8.6100	55	8.9000	83	7.1400
28	8.3000	56	8.9900	84	7.2500

T = 1.0000	NO. OF ITERATIONS = 2	NO. OF SUBITERATIONS = 50
T = 2.0000	NO. OF ITERATIONS = 2	NO. OF SUBITERATIONS = 24
T = 3.0000	NO. OF ITERATIONS = 4	NO. OF SUBITERATIONS = 5
T = 4.0000	NO. OF ITERATIONS = 4	NO. OF SUBITERATIONS = 1
T = 5.0000	NO. OF ITERATIONS = 3	NO. OF SUBITERATIONS = 0
T = 6.0000	NO. OF ITERATIONS = 2	NO. OF SUBITERATIONS = 0
T = 7.0000	NO. OF ITERATIONS = 2	NO. OF SUBITERATIONS = 1
T = 8.0000	NO. OF ITERATIONS = 2	NO. OF SUBITERATIONS = 0
T = 9.0000	NO. OF ITERATIONS = 2	NO. OF SUBITERATIONS = 0
T = 10.0000	NO. OF ITERATIONS = 2	NO. OF SUBITERATIONS = 0
T = 11.0000	NO. OF ITERATIONS = 2	NO. OF SUBITERATIONS = 0
T = 12.0000	NO. OF ITERATIONS = 2	NO. OF SUBITERATIONS = 0

NODE	H	NODE	H	NODE	H
1	6.7000	29	7.8000	57	7.1000
2	7.3000	30	8.1985	58	7.4057
3	7.2000	31	9.1018	59	8.3038
4	7.2000	32	9.5028	60	9.0948
5	7.9000	33	9.0974	61	8.7087
6	8.4000	34	8.9944	62	8.5083
7	8.9000	35	8.5000	63	8.5000
8	6.5400	36	8.0000	64	6.8000
9	7.0082	37	8.5940	65	7.2004
10	7.0707	38	9.5919	66	7.9022
11	6.9984	39	9.9977	67	8.3059
12	7.5185	40	9.3952	68	8.2069
13	8.1475	41	9.1070	69	8.0543
14	8.6050	42	8.9000	70	8.0600
15	6.5000	43	7.7000	71	6.6000
16	7.6009	44	8.3974	72	6.9062
17	8.0084	45	9.4934	73	7.4050
18	7.9049	46	9.7968	74	7.7069
19	8.0023	47	9.4004	75	7.7928
20	8.2985	48	9.1945	76	7.6071
21	8.4000	49	9.0000	77	7.7000
22	7.1900	50	7.3900	78	6.3600
23	7.9595	51	7.9936	79	6.7100
24	8.6094	52	8.9020	80	7.0000
25	8.4051	53	9.5998	81	7.1000
26	8.5006	54	9.1057	82	7.2500
27	8.6167	55	8.9076	83	7.1400
28	8.3000	56	8.9000	84	7.2500

WATER BALANCE COMPONENTS

NODE NET RECHARGE CHANGE IN STORAGE SUBSURFACE FLOW ARTIFICIAL FLOW NODE
 (ALL COMPONENTS ARE CALCULATED FOR THE LAST 1 TIME STEPS)

9	.0000	.0000	.0058	-.0058	9
10	.0000	.0000	.0090	-.0090	10

Contd.

11	.0000	.0000	.0146	-.0146	11
12	.0000	.0000	.0092	-.0092	12
13	.0000	.0000	.0040	-.0040	13
16	.0000	.0000	-.0058	.0058	16
17	.0000	.0000	-.0060	.0060	17
18	.0000	.0000	-.0026	.0026	18
19	.0000	.0000	.0022	-.0022	19
20	.0000	.0000	.0008	-.0008	20
23	.0000	.0000	-.0014	.0014	23
24	.0000	.0000	-.0062	.0062	24
25	.0000	.0000	.0086	-.0086	25
26	.0000	.0000	.0018	-.0018	26
27	.0000	.0000	-.0026	.0026	27
30	.0000	.0000	.0042	-.0042	30
31	.0000	.0000	-.0026	.0026	31
32	.0000	.0000	-.0116	.0116	32
33	.0000	.0000	-.0012	.0012	33
34	.0000	.0000	-.0066	.0066	34
37	.0000	.0000	-.0026	.0026	37
38	.0000	.0000	-.0082	.0082	38
39	.0000	.0000	-.0142	.0142	39
40	.0000	.0000	-.0018	.0018	40
41	.0000	.0000	.0002	-.0002	41
44	.0000	.0000	.0008	-.0008	44
45	.0000	.0000	-.0072	.0072	45
46	.0000	.0000	-.0044	.0044	46
47	.0000	.0000	-.0024	.0024	47
48	.0000	.0000	-.0048	.0048	48
51	.0000	.0000	.0000	.0000	51
52	.0000	.0000	.0000	.0000	52
53	.0000	.0000	-.0104	.0104	53
54	.0000	.0000	.0008	-.0008	54
55	.0000	.0000	.0002	-.0002	55
58	.0000	.0000	.0054	-.0054	58
59	.0000	.0000	.0018	-.0018	59
60	.0000	.0000	-.0106	.0106	60
61	.0000	.0000	.0000	.0000	61
62	.0000	.0000	.0008	-.0008	62
65	.0000	.0000	.0008	-.0008	65
66	.0000	.0000	-.0024	.0024	66
67	.0000	.0000	-.0018	.0018	67
68	.0000	.0000	.0004	-.0004	68
69	.0000	.0000	.0012	-.0012	69
72	.0000	.0000	.0020	-.0020	72
73	.0000	.0000	.0000	.0000	73
74	.0000	.0000	-.0012	.0012	74
75	.0000	.0000	-.0032	.0032	75
76	.0000	.0000	.0020	-.0020	76
TOTAL	.0000	-.0001	-.0452	.0452	

APPENDIX D

1. Format of output files for drain runs

2. Water balance component of SGMP13.OUT for drain at 1.0 m

WATER BALANCE COMPONENTS

NODE	NET RECHARGE	CHANGE IN STORAGE	SUBSURFACE FLOW	ARTIFICIAL FLOW	NODE
(ALL COMPONENTS ARE CALCULATED FOR THE LAST 1 TIME STEPS)					
9	-.0058	.0000	.0058	.0000	9
10	-.0090	.0000	.0090	.0000	10
11	-.0146	.0000	.0146	.0000	11
12	-.0092	.0000	.0092	.0000	12
13	-.0040	.0000	.0040	.0000	13
16	.0058	.0000	-.0058	.0000	16
17	.0060	.0000	-.0060	.0000	17
18	.0026	.0000	-.0026	.0000	18
19	-.0022	.0000	.0022	.0000	19
20	-.0008	.0000	.0008	.0000	20
23	.0014	.0000	-.0014	.0000	23
24	.0062	.0000	-.0062	.0000	24
25	-.0086	.0000	.0086	.0000	25
26	-.0018	.0000	.0018	.0000	26
27	.0026	.0000	-.0026	.0000	27
30	-.0042	.0000	.0042	.0000	30
31	.0026	.0000	-.0026	.0000	31
32	.0116	.0000	-.0116	.0000	32
33	.0012	.0000	-.0012	.0000	33
34	.0066	.0000	-.0066	.0000	34
37	.0026	.0000	-.0026	.0000	37
38	.0082	.0000	-.0082	.0000	38
39	.0142	-.0000	-.0062	-.0080	39
40	.0018	.0000	-.0010	-.0008	40
41	-.0002	.0000	.0002	.0000	41
44	-.0008	.0000	.0008	.0000	44
45	.0072	.0000	-.0072	.0000	45
46	.0044	.0000	-.0044	.0000	46
47	.0024	.0000	.0046	-.0070	47
48	.0048	.0000	-.0048	.0000	48
51	.0000	.0000	.0000	.0000	51
52	.0000	.0000	.0000	.0000	52
53	.0104	.0000	-.0104	.0000	53
54	-.0008	.0000	.0008	.0000	54
55	-.0002	.0000	.0002	.0000	55
58	-.0054	.0000	.0054	.0000	58
59	-.0018	.0000	.0018	.0000	59
60	.0106	.0000	-.0106	.0000	60
61	.0000	.0000	.0000	.0000	61
62	-.0008	.0000	.0008	.0000	62
65	-.0008	.0000	.0008	.0000	65
66	.0024	.0000	-.0024	.0000	66
67	.0018	.0000	-.0018	.0000	67
68	-.0004	.0000	.0004	.0000	68
69	-.0012	.0000	.0012	.0000	69
72	-.0020	.0000	.0020	.0000	72
73	.0000	.0000	.0000	.0000	73
74	.0012	.0000	-.0012	.0000	74
75	.0032	.0000	-.0032	.0000	75
76	-.0020	.0000	.0020	.0000	76
TOTAL	.0452	.0002	-.0294	-.0158	

3. Water balance component of SGMP13.OUT for drain at 1.5 m

WATER BALANCE COMPONENTS

NODE NET RECHARGE CHANGE IN STORAGE SUBSURFACE FLOW ARTIFICIAL FLOW NODE
 (ALL COMPONENTS ARE CALCULATED FOR THE LAST 1 TIME STEPS)

9	-.0058	.0000	.0058	.0000	9
10	-.0000	.0000	.0090	.0000	10
11	-.0146	.0000	.0146	.0000	11
12	-.0092	.0000	.0092	.0000	12
13	-.0040	.0000	.0040	.0000	13
16	.0058	.0000	-.0058	.0000	16
17	.0060	.0000	-.0060	.0000	17
18	.0026	.0000	-.0026	.0000	18
19	-.0022	.0000	.0022	.0000	19
20	-.0008	.0000	.0008	.0000	20
23	.0014	.0000	-.0014	.0000	23
24	.0052	.0000	-.0062	.0000	24
25	-.0086	.0000	.0086	.0000	25
26	-.0018	.0000	.0018	.0000	26
27	.0026	.0000	-.0026	.0000	27
30	-.0042	.0000	.0042	.0000	30
31	.0026	.0000	-.0026	.0000	31
32	.0116	.0000	-.0078	-.0038	32
33	.0012	.0000	-.0012	.0000	33
34	.0066	.0000	-.0066	.0000	34
37	.0026	.0000	-.0026	.0000	37
38	.0082	.0000	-.0082	.0000	38
39	.0142	.0000	-.0044	-.0098	39
40	.0018	.0000	.0034	-.0052	40
41	-.0002	.0000	.0002	.0000	41
44	-.0008	.0000	.0008	.0000	44
45	.0072	.0000	-.0064	-.0008	45
46	.0044	.0000	-.0036	-.0008	46
47	.0024	.0000	.0078	-.0102	47
48	.0048	.0000	-.0048	.0000	48
51	.0000	.0000	.0000	.0000	51
52	.0000	.0000	.0000	.0000	52
53	.0104	.0000	-.0082	-.0022	53
54	-.0008	.0000	.0038	-.0030	54
55	-.0002	.0000	.0002	.0000	55
58	-.0054	.0000	.0054	.0000	58
59	-.0018	.0000	.0018	.0000	59
60	.0106	.0000	-.0106	.0000	60
61	.0000	.0000	.0000	.0000	61
62	-.0008	.0000	.0008	.0000	62
65	-.0008	.0000	.0008	.0000	65
66	.0024	.0000	-.0024	.0000	66
67	.0018	.0000	-.0018	.0000	67
68	-.0004	.0000	.0004	.0000	68
69	-.0012	.0000	.0012	.0000	69
72	-.0020	.0000	.0020	.0000	72
73	.0000	.0000	.0000	.0000	73
74	.0012	.0000	-.0012	.0000	74
75	.0032	.0000	-.0032	.0000	75
76	-.0020	.0000	.0020	.0000	76
TOTAL	.0452	.0003	-.0096	-.0358	

APPENDIX E

1. Format of input files for normal run

2. Format of input file SGMP11.INP

'**Numerical Groundwater Modelling of Bulandshahar'**

'**With monthly data from June 1976 to Oct. 1977'**

N	NSIDES
100	263

THDAS 'MONTH'	DHTIM 30.5	T 0	DELTA 1	MINOR 1	MAJOR 2	LIST 2	DL .1	
SCALE 253433	ERROR 0.08	COEFFA 0.1	PREDIC 1	DELQ .05	LSW1 1	LSW2 1	NITER 250	
K	RTYPE	CO(X,1)	CO(Y,1)	SL(K)	TL(K)	BL(K)	UL(K)	LL(K)
1	1	3.5	29.7	203	203	136	203	136
2	1	5.85	31.25	210	210	142	210	142
3	1	6.6	31.5	210.1	210.1	141.6	210.1	141.6
4	1	11.2	32.2	210.3	210.3	140.3	210.3	140.3
5	1	13.65	28.8	210	210	145	210	145
6	1	16.7	27.7	208	208	145	208	145
7	1	18	28.6	208	208	144.5	208	144.5
8	1	20.5	30.6	208	208	143	208	143
9	1	23.25	31.35	208.7	208.7	142.2	208.7	142.2
10	1	25.6	34.35	209.5	209.5	134.5	209.5	134.5
11	1	28.5	33.35	206.1	206.1	136.1	206.1	136.1
12	1	31.35	32.2	204	204	141	204	141
13	1	4.35	26.4	199	199	134.5	199	134.5
14	3	7.1	28.5	205	205	139	205	139
15	3	10.85	27.15	207.8	207.8	143.3	207.8	143.3
16	3	13.45	26.85	209	209	145.5	209	145.5
17	3	15.45	25.3	207.7	207.7	149.7	207.7	149.7
18	3	18.15	26.3	206	206	148	206	148
19	3	19.6	27.8	206.2	206.2	146.2	206.2	146.2
20	3	22.2	29.25	207.3	207.3	146.8	207.3	146.8
21	3	24.2	28.3	206.2	206.2	148.7	206.2	148.7
22	3	26.1	31.5	207.8	207.8	140.3	207.8	140.3
23	3	28.65	31.4	205	205	140	205	140
24	3	30.7	30.45	203	203	143	203	143
25	1	33.75	29.65	200.5	200.5	144.5	200.5	144.5
26	1	5.7	23.6	195.8	195.8	135.8	195.8	135.8
27	3	7.8	24.8	198.1	198.1	135.1	198.1	135.1
28	3	12.05	24.05	206.5	206.5	148.5	206.5	148.5
29	3	15.1	23.1	207	207	154.5	207	154.5
30	3	17.75	24.2	205.5	205.5	151.5	205.5	151.5
31	3	20	25.55	205	205	149	205	149
32	3	22.15	26.8	205.2	205.2	149.2	205.2	149.2
33	3	25.45	25.55	204	204	153.5	204	153.5
34	3	27.05	29.9	207	207	145	207	145
35	3	27.7	28	205	205	150	205	150
36	3	29.55	26.2	203.8	203.8	147.8	203.8	147.8
37	3	29.6	28.95	205.7	205.7	148.7	205.7	148.7
38	3	31.1	28.65	202	202	146	202	146

Contd.

39	3	30.55	25.75	203.2	203.2	147.7	203.2	147.7
40	1	34.5	27.3	199	199	147.5	199	147.5
41	1	7.9	21.5	198	198	143	198	143
42	3	13.05	21.65	205.8	205.8	155.8	205.8	155.8
43	3	18.75	21.45	203.5	203.5	155	203.5	155
44	3	20.15	23.3	203.6	203.6	151.1	203.6	151.1
45	3	22	24.15	203.6	203.6	151.1	203.6	151.1
46	3	24.85	22.9	202	202	153	202	153
47	3	28.75	23.5	202.3	202.3	154.3	202.3	154.3
48	3	33	22.8	201.6	201.6	154.6	201.6	154.6
49	1	36.65	23.4	196	196	152	196	152
50	1	10.15	15.85	203.5	203.5	155.5	203.5	155.5
51	3	12.35	17.9	204.3	204.3	157.8	204.3	157.8
52	3	15.8	19.05	203.3	203.3	158.3	203.3	158.3
53	3	18.55	16.75	200	200	156	200	156
54	3	21.4	18.35	200.7	200.7	151.2	200.7	151.2
55	3	22.2	21.25	201.5	201.5	150	201.5	150
56	3	24.55	19.5	200.8	200.8	149.3	200.8	149.3
57	3	26.65	21.2	201.5	201.5	153	201.5	153
58	3	26.9	19.15	200.8	200.8	148.8	200.8	148.8
59	3	29.1	19.95	201	201	152.5	201	152.5
60	3	32.3	20.4	200.8	200.8	155.8	200.8	155.8
61	1	36.25	20.35	194	194	154	194	154
62	1	12.7	11.55	198	198	155.5	198	155.5
63	3	14.5	14.65	199.2	199.2	156.7	199.2	156.7
64	3	17.85	13	197.3	197.3	163.8	197.3	163.8
65	3	21.3	14.55	197.8	197.8	150.8	197.8	150.8
66	3	24.95	16.4	199	199	146.5	199	146.5
67	3	28.65	16.5	199	199	149	199	149
68	3	30.9	17.3	199	199	156.5	199	156.5
69	3	34	17.4	197	197	159.5	197	159.5
70	1	37.55	16.7	192	192	162	192	162
71	1	13.5	8.15	195	195	157.5	195	157.5
72	3	15.5	10.95	197	197	164.5	197	164.5
73	3	18.2	10.5	196.2	196.2	162.7	196.2	162.7
74	3	21.8	11.4	195	195	155	195	155
75	3	25.65	12.7	196	196	163.5	196	163.5
76	3	28.55	14.45	197	197	159.5	197	159.5
77	3	31.25	14.9	196	196	160	196	160
78	3	35	12.95	192.2	192.2	162.2	192.2	162.2
79	1	38.7	14.35	190	190	166.5	190	166.5
80	1	14.4	5.5	193.8	193.8	158.8	193.8	158.8
81	3	16.05	7.85	195.5	195.5	162.5	195.5	162.5
82	3	24.6	10	193.8	193.8	163.8	193.8	163.8
83	3	26.7	10	193.8	193.8	168.8	193.8	168.8
84	3	28.4	11.45	194	194	171	194	171
85	3	30.8	12.15	194	194	171.5	194	171.5
86	3	33.45	10.25	191.7	191.7	166.2	191.7	166.2
87	3	35.5	10.25	191	191	168	191	168
88	1	38.2	11.1	190	190	170	190	170
89	3	26.2	7.3	192.6	192.6	167.6	192.6	167.6
90	3	29.9	8.95	192	192	170	192	170
91	3	33.6	8	190.8	190.8	168.8	190.8	168.8
92	3	36.65	8.35	190	190	170	190	170
93	1	18.25	4.25	193.5	193.5	161.5	193.5	161.5
94	1	21.05	6.9	193.5	193.5	162	193.5	162
95	1	25.1	4.5	192	192	169.5	192	169.5
96	1	29.6	5.25	191	191	170	191	170

Contd.

97	1	32.25	2.7	190	190	171	190	171
98	1	36.25	3.95	189.5	189.5	172	189.5	172
99	1	38.85	5	189	189	171.5	189	171.5
100	1	41	8.65	188	188	169.5	188	169.5

K	AS(K)	ASC(K)
1	0.1145	0.001
2	0.1125	0.001
3	0.112	0.001
4	0.1065	0.001
5	0.1	0.001
6	0.1	0.001
7	0.1025	0.001
8	0.1075	0.001
9	0.112	0.001
10	0.103	0.001
11	0.097	0.001
12	0.087	0.001
13	0.1155	0.001
14	0.1105	0.001
15	0.1005	0.001
16	0.095	0.001
17	0.0925	0.001
18	0.1	0.001
19	0.105	0.001
20	0.11	0.001
21	0.111	0.001
22	0.1065	0.001
23	0.095	0.001
24	0.086	0.001
25	0.0815	0.001
26	0.1125	0.001
27	0.108	0.001
28	0.094	0.001
29	0.0925	0.001
30	0.0975	0.001
31	0.105	0.001
32	0.108	0.001
33	0.106	0.001
34	0.1025	0.001
35	0.098	0.001
36	0.093	0.001
37	0.0885	0.001
38	0.083	0.001
39	0.091	0.001
40	0.08	0.001
41	0.108	0.001
42	0.0955	0.001
43	0.098	0.001
44	0.1025	0.001
45	0.1055	0.001
46	0.1055	0.001
47	0.104	0.001
48	0.0875	0.001
49	0.0795	0.001
50	0.106	0.001
51	0.1025	0.001
52	0.0865	0.001

Contd.

53	0.1	0.001
54	0.102	0.001
55	0.1055	0.001
56	0.1055	0.001
57	0.1055	0.001
58	0.1055	0.001
59	0.1025	0.001
60	0.0925	0.001
61	0.082	0.001
62	0.1015	0.001
63	0.099	0.001
64	0.0985	0.001
65	0.099	0.001
66	0.106	0.001
67	0.102	0.001
68	0.0975	0.001
69	0.0875	0.001
70	0.0815	0.001
71	0.1035	0.001
72	0.1015	0.001
73	0.1025	0.001
74	0.1045	0.001
75	0.1085	0.001
76	0.105	0.001
77	0.0975	0.001
78	0.0925	0.001
79	0.082	0.001
80	0.106	0.001
81	0.1045	0.001
82	0.115	0.001
83	0.1155	0.001
84	0.1125	0.001
85	0.105	0.001
86	0.1025	0.001
87	0.095	0.001
88	0.087	0.001
89	0.1165	0.001
90	0.116	0.001
91	0.103	0.001
92	0.095	0.001
93	0.1065	0.001
94	0.11	0.001
95	0.114	0.001
96	0.113	0.001
97	0.1085	0.001
98	0.101	0.001
99	0.095	0.001
100	0.086	0.001

IB	IE	CONDU
1	2	3
1	14	3
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2	14	3
3	4	3
3	14	3
4	5	3

Contd.

4	14	3
4	15	3
5	6	3
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22	23	3
22	34	3
23	24	3
23	34	3

Contd.

23	37	3
24	25	3
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41	51	3
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43	53	3
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44	45	3
44	55	3

Contd.

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58	67	3
59	60	3
59	67	3
59	68	3
60	61	3
60	68	3
60	69	3
61	69	3
61	70	3
62	63	3
62	72	3
62	71	3
63	64	3
63	72	3
64	65	3
64	72	3
64	73	3
64	74	3
65	66	3
65	74	3
65	75	3

Contd.

66	67	3
66	75	3
66	76	3
67	68	3
67	76	3
67	77	3
68	69	3
68	77	3
69	70	3
69	77	3
69	78	3
70	78	3
70	79	3
71	72	3
71	81	3
71	80	3
72	73	3
72	81	3
73	74	3
73	81	3
73	94	3
74	75	3
74	94	3
74	82	3
75	76	3
75	82	3
75	83	3
75	84	3
76	77	3
76	84	3
76	85	3
77	78	3
77	85	3
78	79	3
78	85	3
78	86	3
78	87	3
78	88	3
79	88	3
80	81	3
80	93	3
81	94	3
81	93	3
82	83	3
82	94	3
82	89	3
83	84	3
83	89	3
83	90	3
84	85	3
84	90	3
85	86	3
85	90	3
86	87	3
86	90	3
86	91	3
87	88	3
87	91	3

Contd.

87	92	3
88	92	3
88	100	3
89	94	3
89	95	3
89	90	3
89	96	3
90	91	3
90	96	3
91	92	3
91	96	3
91	97	3
91	98	3
92	100	3
92	98	3
92	99	3
93	94	3
94	95	3
95	96	3
96	97	3
97	98	3
98	99	3
99	100	3

3. Format of input file SGKP12A.INP

LSW3	LSW4	LSW5	LSW6
3	2	2	1

H(K): INITIAL WATER TABLE ELEVATIONS JUNE 1976

199.2	202.5	203.6	206	206	203.5	203	201.5	202	204
199.7	195	195	201	204.2	205	203	201.2	201	200.5
199	201.8	199	195.5	192.5	192	195	202	201.5	200
199	198.1	197	201	200	197.8	199	195.5	197	192
193.8	200.5	198.5	197.8	196.5	195	196	194	186	194.4
197.5	198.7	197	196.5	196	193.5	194	192	192.6	192
184	191.5	195	194.1	193.2	190	189.9	190.2	187.8	182
188.2	192.7	192.4	191	189.5	189	188.2	185	182	186.5
190.1	189.7	189	188.3	187.5	186	184.5	183.4	189	188
186.5	184	187.8	189.5	188.6	188.3	186.8	185	183	180.5

H(K): WATER TABLE ELEVATIONS ON OCTOBER 1976

200.1	203.5	204.5	208	208	205.5	204.1	202.5	202.2	205
199.8	196	196	202	206.1	207	205	203	202.4	200.5
199.2	201.9	200	196	193.5	193.9	195.9	204	203.9	202.2
200.7	199	197.9	201.2	200.4	199	196.6	196.5	198.2	193
195	203	200.5	199.3	197.8	196.2	197.2	195	187.5	195.9
199.6	201	197.8	197.3	197.2	194.5	195.1	193	193.9	192.2
186	192.2	197	195.6	195	191	191	191.6	189	183
190.6	194.5	194.4	193	190	189.7	189.6	186	183	189
192.5	191.2	190.2	189	188.7	187.3	185.5	183.5	190.8	189
188	185	190.2	191.7	191	189.9	188.3	186	183.8	181.2

DELTA: MONSOON 1976

4

H(K): WATER TABLE ELEVATIONS JUNE 1977

200	203	204	207	206	203.1	202.1	201	201.2	203
198.8	195	195.1	201	204.2	205	203	200.7	200	199.5
198.5	201.6	198.6	194.9	191.5	192	194.5	202	201.5	200
198.7	198	197	201	200.1	198	198.6	195	197.1	191.2
193.2	200.5	198.2	197.5	196	195	195.8	194	186	194.5
197.2	198.2	196.6	196.1	195.5	193	193.5	191.5	192	191.7
184	191.5	195	194	194	189.9	189.4	189.5	187	182
188.5	192.5	192.3	191	189	188	187.6	184.1	182	187
190	189.7	188.7	188	186.8	185.5	184.2	182.1	189	187.3
186.1	183.8	188	189.2	189	187.5	186.5	184.3	182.4	180

DELTA: NONMONSOON 1976 - 77

8

H(K): WATER TABLE ELEVATIONS OCTOBER 1977

200.5	204	204.8	208.5	207	204.6	203.8	202.2	202.5	205
202	198.5	197	202	205.6	206.3	204.8	203	202.2	200.6
199.5	203.5	200	197	194	193.6	195.8	203.2	203.6	203.1
200.8	199	198.2	203	202	199.8	200.5	197.2	199	193
195	202	200.5	199.2	197.8	196.6	198	195.1	187	195.9
198.7	200.5	198.6	197.8	197.3	195	195.5	193.5	194	193.2
186	193.2	197	196.1	195.8	191.2	190.8	191.2	188.7	184
190.7	194.7	194.6	193.5	190.6	189.5	189	186.1	184	189.3
192.7	191.8	190.5	189.5	188.2	187.2	185.7	182.8	191.3	189.6
188.2	185.5	190.6	192.3	192.1	190.6	189.2	186.3	184	181.3

Contd.

DELTA: KONSOON 1977

4

H(K): WATER TABLE ELEVATIONS JUNE 1978										
200	203	204	207	206.5	203.5	202.5	201	200.5	204	200
199	195.5	195.5	201	204.2	205	203	201	200.5	197.5	192
199	202	200	196	193	192	197	202	202	200.5	192
199	198	197	201.4	200.5	198.2	200	196	197.5	194.5	192
194.5	200.5	199	198	196	195.5	196.5	194	189	194.5	192
197	198.6	196.5	197	196.5	195	195	194.4	194	192	182
184	192	195	194	194	193	191	191	188	188	182
189	193	192.2	191.5	191.6	191	191.2	185.5	182	188	188
191	190.5	189.6	189.6	188	186	185	182.5	189	188.5	180.5
185.5	183.5	188.3	189.6	188.3	187.2	185.2	183.4	182.1	180.5	180.5

DELTA: NONKONSOON 1977 - 78

8

4. Format of input file SGMP12B.INP

QNET(K): NET RECHARGE OF KONSOON 1976

0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0

QNET(K): NET RECHARGE OF NONKONSOON 1976 - 77

0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0

QNET(K): NET RECHARGE OF KONSOON 1977

0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0	0

Contd.

QNET(E): NET RECHARGE OF NONMONSOON 1977 - 78

0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0
0	0	0	0	0	0	0	0	0	0

2. Format of input file SGMP13.INP

LSW7	LSW8	LSW9
2	2	2

APPENDIX F

1. Format of output file for inverse run

2. Format of output file SGMP13.OUT

GROUNDWATER MODEL FOR NUMERICAL GROUNDWATER MODELLING OF BULANDSHEHAR
WITH MONTHLY DATA FROM JUNE 1976 TO OCT. 1977

PRINTING OF CALCULATED WATER LEVELS AND BALANCES

TOTAL NUMBER OF NODES IS 100

UNIT LENGTH : 1 METRE
UNIT AREA : 1,000,000 SQ. METRES
UNIT VOLUME : 1,000,000 CU. METRES

UNIT OF TIME FOR DELTA AND BOUNDARY CONDITIONS IS MONTH

VALUES OF EXTERNAL SWITCHES

LSW7 = 2
LSW8 = 2
LSW9 = 2

INITIAL WATER LEVEL ELEVATIONS

T = .0000

NODE	HINIT	NODE	HINIT	NODE	HINIT
1	199.2000	35	200.0000	69	187.8000
2	202.5000	36	197.8000	70	182.0000
3	203.6000	37	199.0000	71	188.2000
4	206.0000	38	195.5000	72	192.7000
5	206.0000	39	197.0000	73	192.4000
6	203.5000	40	192.0000	74	191.0000
7	203.0000	41	193.8000	75	189.5000
8	201.5000	42	200.5000	76	189.0000
9	202.0000	43	198.5000	77	188.2000
10	204.0000	44	197.8000	78	185.0000
11	199.7000	45	196.5000	79	182.0000
12	195.0000	46	195.0000	80	186.5000
13	195.0000	47	196.0000	81	190.1000
14	201.0000	48	194.0000	82	189.7000
15	204.2000	49	186.0000	83	189.0000
16	205.0000	50	194.4000	84	188.3000
17	203.0000	51	197.5000	85	187.5000
18	201.2000	52	198.7000	86	186.0000
19	201.0000	53	197.0000	87	184.5000
20	200.5000	54	196.5000	88	183.4000
21	199.0000	55	196.0000	89	189.0000
22	201.8000	56	193.5000	90	188.0000
23	199.0000	57	194.0000	91	186.5000
24	195.5000	58	192.0000	92	184.0000

Contd.

25	192.5000	59	192.6000	93	187.8000
26	192.0000	60	192.0000	94	189.5000
27	195.0000	61	184.0000	95	188.6000
28	202.0000	62	191.5000	96	188.3000
29	201.5000	63	195.0000	97	186.8000
30	200.0000	64	194.1000	98	185.0000
31	199.0000	65	193.2000	99	183.0000
32	198.1000	66	190.0000	100	180.5000
33	197.0000	67	189.9000		
34	201.0000	68	190.2000		

T = 4.0000 NO. OF ITERATIONS = 20 NO. OF SUBITERATIONS = 125

NODE	H	NODE	H	NODE	H
1	200.1000	35	200.4188	69	188.9657
2	203.5000	36	199.0144	70	183.0000
3	204.5000	37	196.5959	71	190.6000
4	208.0000	38	196.4644	72	194.4946
5	208.0000	39	198.2126	73	194.3653
6	205.5000	40	193.0000	74	192.9648
7	204.1000	41	195.0000	75	189.9678
8	202.5000	42	202.9820	76	189.7014
9	202.2000	43	200.4600	77	189.6171
10	205.0000	44	199.3046	78	185.9563
11	199.8000	45	197.8464	79	183.0000
12	196.0000	46	196.2209	80	189.0000
13	196.0000	47	197.1509	81	192.4977
14	201.9591	48	194.9788	82	191.1854
15	206.0526	49	187.5000	83	190.2400
16	206.9987	50	195.9000	84	189.0082
17	204.9970	51	199.5638	85	188.7013
18	202.9670	52	200.9597	86	187.3242
19	202.3399	53	197.7638	87	185.5079
20	200.5112	54	197.2611	88	183.5000
21	199.1605	55	197.1861	89	190.7544
22	201.8890	56	194.5264	90	188.9695
23	199.9857	57	195.1441	91	187.9658
24	196.0281	58	193.0320	92	184.9970
25	193.5000	59	193.9110	93	190.2000
26	193.3000	60	192.1755	94	191.7000
27	195.8503	61	186.0000	95	191.0000
28	203.9815	62	192.2000	96	189.9000
29	203.9050	63	196.9662	97	188.3000
30	202.1944	64	195.5648	98	186.0000
31	200.6858	65	194.9678	99	183.8000
32	199.0476	66	190.9709	100	181.2000
33	197.8559	67	190.9980		
34	201.1606	68	191.6449		

WATER BALANCE COMPONENTS

NODE NET RECHARGE CHANGE IN STORAGE SUBSURFACE FLOW ARTIFICIAL FLOW NODE
(ALL COMPONENTS ARE CALCULATED FOR THE LAST 1 TIME STEPS)

14	.0000	-8.3733	-.0274	8.4000	14
15	.0000	-14.4262	-.1795	14.6000	15

Contd.

16	.0000	-7.7280	-.0725	7.8000	16
17	.0000	-7.3780	-.0224	7.4000	17
18	.0000	-4.8075	.0073	4.8000	18
19	.0000	-4.7864	-.0138	4.8000	19
20	.0000	-.0414	.0417	.0000	20
21	.0000	-.8604	.0608	.8000	21
22	.0000	-.4003	.0000	.4000	22
23	.0000	-2.9285	-.0717	3.0000	23
24	.0000	-1.2564	.0565	1.2000	24
27	.0000	-6.4350	.2345	6.2000	27
28	.0000	-12.7149	-.0821	12.8000	28
29	.0000	-11.5490	-.0496	11.6000	29
30	.0000	-8.4083	.0075	8.4000	30
31	.0000	-5.4029	.0027	5.4000	31
32	.0000	-4.2464	.0467	4.2000	32
33	.0000	-5.6218	.0223	5.6000	33
34	.0000	-.5422	-.0579	.6000	34
35	.0000	-1.6972	-.1025	1.8000	35
36	.0000	-3.9345	-.0657	4.0000	36
37	.0000	5.6881	.1125	-5.8000	37
38	.0000	-2.9683	-.0319	3.0000	38
39	.0000	-5.5318	-.0689	5.6000	39
42	.0000	-16.8760	-.0738	17.0000	42
43	.0000	-12.9685	-.0302	13.0000	43
44	.0000	-5.2235	.0232	5.2000	44
45	.0000	-6.0450	.0453	6.0000	45
46	.0000	-6.6300	.0304	6.6000	46
47	.0000	-8.1695	-.0310	8.2000	47
48	.0000	-6.6671	-.1325	6.8000	48
51	.0000	-18.7946	-.0666	18.8000	51
52	.0000	-16.5311	-.0538	16.6000	52
53	.0000	-6.1960	-.0045	6.2000	53
54	.0000	-5.5421	-.0574	5.6000	54
55	.0000	-6.6066	.0070	6.6000	55
56	.0000	-5.4094	.0098	5.4000	56
57	.0000	-4.9936	-.0061	5.0000	57
58	.0000	-4.4285	.0289	4.4000	58
59	.0000	-7.3762	-.0234	7.4000	59
60	.0000	-1.0262	.0266	1.0000	60
63	.0000	-17.5973	-.0307	17.6000	63
64	.0000	-9.7963	-.0033	9.8000	64
65	.0000	-13.9439	-.0309	14.0000	65
66	.0000	-7.9194	.1207	7.8000	66
67	.0000	-5.2115	.0118	5.2000	67
68	.0000	-6.7664	-.0333	6.8000	68
69	.0000	-7.7739	-.0256	7.8000	69
72	.0000	-10.1615	-.0379	10.2000	72
73	.0000	-13.5681	-.0273	13.6000	73
74	.0000	-16.2207	.0095	16.2000	74
75	.0000	-3.4401	.0405	3.4000	75
76	.0000	-3.6226	.0229	3.6000	76
77	.0000	-7.8000	.0004	7.8000	77
78	.0000	-7.0135	.0141	7.0000	78
81	.0000	-16.7168	-.0368	16.8000	81
82	.0000	-10.2023	.0032	10.2000	82
83	.0000	-5.7980	-.0017	5.8000	83
84	.0000	-3.4150	.0154	3.4000	84
85	.0000	-7.0008	.0011	7.0000	85

Contd.

86	.0000	-6.3963	-.0034	6.4000	86
87	.0000	-3.6101	.0101	3.6000	87
88	.0000	-14.6011	-.0045	14.6000	88
89	.0000	-8.0038	.0047	8.0000	89
90	.0000	-11.9812	-.0201	12.0000	90
91	.0000	-6.4046	.0050	6.4000	91
TOTAL	.0000	-480.7997	-.5571	481.3997	

T = 12.0000 NO. OF ITERATIONS = 31 NO. OF SUBITERATIONS = 188

NODE	H	NODE	H	NODE	H
1	200.0000	35	200.0833	69	186.9842
2	203.0000	36	198.0009	70	182.0000
3	204.0000	37	198.5784	71	188.5000
4	207.0000	38	194.9758	72	192.5084
5	206.0000	39	197.1277	73	192.3302
6	203.1000	40	191.2000	74	190.9856
7	202.1000	41	193.2000	75	188.9952
8	201.0000	42	200.5145	76	188.0095
9	201.2000	43	198.1594	77	187.5843
10	203.0000	44	197.4674	78	184.1073
11	198.8000	45	196.0043	79	182.0000
12	195.0000	46	195.0476	80	187.0000
13	195.1000	47	195.7918	81	189.9683
14	200.9907	48	193.9996	82	189.7274
15	204.2149	49	186.0000	83	188.7015
16	204.9980	50	194.5000	84	188.0127
17	203.0262	51	197.1734	85	186.8499
18	200.7398	52	198.2208	86	185.5023
19	199.9636	53	196.5718	87	184.1696
20	199.4506	54	196.0879	88	182.1000
21	198.5078	55	195.5383	89	188.9747
22	201.5963	56	193.0142	90	187.2742
23	198.6373	57	193.4970	91	186.1026
24	194.9240	58	191.4595	92	183.8134
25	191.5000	59	191.9934	93	188.0000
26	192.0000	60	191.6867	94	189.2000
27	194.4779	61	184.0000	95	189.0000
28	202.0221	62	191.5000	96	187.5000
29	201.4757	63	194.9527	97	186.5000
30	200.0071	64	194.0071	98	184.3000
31	198.6925	65	193.9904	99	182.4000
32	197.9851	66	189.8692	100	180.0000
33	197.0080	67	189.3985		
34	200.9959	68	189.5054		

WATER BALANCE COMPONENTS

NODE NET RECHARGE CHANGE IN STORAGE SUBSURFACE FLOW ARTIFICIAL FLOW NODE
(ALL COMPONENTS ARE CALCULATED FOR THE LAST 1 TIME STEPS)

14	.0000	8.4547	-.0540	-8.4000	14
15	.0000	14.3099	-.3090	-14.0000	15
16	.0000	7.7359	-.1356	-7.6000	16
17	.0000	7.2810	-.0807	-7.2000	17

Contd.

18	.0000	6.0596	.0168	-6.0000	18
19	.0000	8.3300	.0706	-8.4000	19
20	.0000	3.9372	.0594	-4.0000	20
21	.0000	3.4985	.1011	-3.6000	21
22	.0000	1.3164	-.1167	-1.2000	22
23	.0000	4.0061	-.0064	-4.0000	23
24	.0000	2.6269	.1713	-2.8000	24
27	.0000	10.3863	.4143	-10.8000	27
28	.0000	12.5733	-.1739	-12.4000	28
29	.0000	11.6655	-.0666	-11.6000	29
30	.0000	8.3810	.0198	-8.4000	30
31	.0000	6.3885	.0124	-6.4000	31
32	.0000	4.7615	.0382	-4.8000	32
33	.0000	5.5691	.0314	-5.6000	33
34	.0000	.5560	-.1563	-.4000	34
35	.0000	1.3596	-.1604	-1.2000	35
36	.0000	3.2835	-.0841	-3.2000	36
37	.0000	-4.6907	-.1985	4.8000	37
38	.0000	4.5817	.1076	-4.8000	38
39	.0000	4.9491	-.1495	-4.8000	39
42	.0000	16.7776	-.1098	-16.8000	42
43	.0000	15.2221	-.0216	-15.2000	43
44	.0000	6.3782	.0216	-6.4000	44
45	.0000	8.2704	.1299	-8.4000	45
46	.0000	6.3714	.0289	-6.4000	46
47	.0000	9.6473	-.0469	-9.6000	47
48	.0000	6.6695	-.2690	-6.4000	48
51	.0000	21.7691	-.0831	-21.6000	51
52	.0000	20.0367	-.0281	-20.0000	52
53	.0000	9.6692	-.0687	-9.6000	53
54	.0000	8.5430	-.1425	-8.4000	54
55	.0000	9.1783	.0221	-9.2000	55
56	.0000	7.9700	.0297	-8.0000	56
57	.0000	7.1893	.0104	-7.2000	57
58	.0000	6.7481	.0516	-6.8000	58
59	.0000	10.7889	.0106	-10.8000	59
60	.0000	2.8581	-.0585	-2.8000	60
63	.0000	18.0210	-.0244	-18.0000	63
64	.0000	10.4173	-.0169	-10.4000	64
65	.0000	7.7092	-.1086	-7.6000	65
66	.0000	8.9804	.2131	-9.2000	66
67	.0000	7.5917	.0080	-7.6000	67
68	.0000	10.0188	-.0185	-10.0000	68
69	.0000	13.2146	-.0141	-13.2000	69
72	.0000	11.2165	-.0162	-11.2000	72
73	.0000	14.0502	-.0497	-14.0000	73
74	.0000	16.3395	.0651	-16.4000	74
75	.0000	7.1518	.0485	-7.2000	75
76	.0000	8.7385	.0612	-8.8000	76
77	.0000	11.1890	.0106	-11.2000	77
78	.0000	13.5605	.0399	-13.6000	78
81	.0000	17.6350	-.0338	-17.6000	81
82	.0000	10.0140	-.0135	-10.0000	82
83	.0000	7.1936	.0064	-7.2000	83
84	.0000	4.8004	-.0007	-4.8000	84
85	.0000	10.7894	.0111	-10.8000	85
86	.0000	8.8005	-.0001	-8.8000	86
87	.0000	4.7939	.0059	-4.8000	87

Contd.

89	.0000	14.8114	-.0120	-14.8000	89
90	.0000	13.9958	.0049	-14.0000	90
91	.0000	15.2293	-.0287	-15.2000	91
92	.0000	7.6034	-.0030	-7.6000	92
TOTAL	.0000	585.3098	-1.0677	-584.3998	

T = 16.0000 NO. OF ITERATIONS = 23 NO. OF SUBITERATIONS = 171

NODE	H	NODE	H	NODE	H
1	200.5000	35	202.0330	69	188.6925
2	204.0000	36	199.7822	70	184.0000
3	204.8000	37	200.4818	71	190.7000
4	208.5000	38	197.1985	72	194.7283
5	207.0000	39	199.0375	73	194.5863
6	204.6000	40	193.0000	74	193.4647
7	203.8000	41	195.0000	75	190.5769
8	202.2000	42	201.9789	76	189.5252
9	202.5000	43	200.4810	77	189.0037
10	205.0000	44	199.2045	78	186.0717
11	202.0000	45	197.8419	79	184.0000
12	198.5000	46	196.5990	80	189.3000
13	197.0000	47	197.9820	81	192.6764
14	201.9521	48	195.0993	82	191.7655
15	205.5542	49	187.0000	83	190.5421
16	206.3244	50	195.9000	84	189.5070
17	204.8011	51	198.6640	85	188.2219
18	203.0118	52	200.4798	86	187.2417
19	202.1645	53	198.5643	87	185.7343
20	200.5925	54	197.7549	88	182.8000
21	199.5293	55	197.3003	89	191.2599
22	203.4936	56	194.9877	90	189.5764
23	200.0139	57	195.5130	91	188.1566
24	196.9926	58	193.5134	92	185.4630
25	194.0000	59	194.0189	93	190.6000
26	193.6000	60	193.1870	94	192.3000
27	195.7747	61	186.0000	95	192.1000
28	203.1656	62	193.2000	96	190.6000
29	203.6283	63	196.9640	97	189.2000
30	202.0973	64	196.0693	98	186.3000
31	200.7501	65	195.7594	99	184.0000
32	199.0239	66	191.1608	100	181.3000
33	198.2014	67	190.8347		
34	202.9797	68	191.2106		

WATER BALANCE COMPONENTS

NODE NET RECHARGE CHANGE IN STORAGE SUBSURFACE FLOW ARTIFICIAL FLOW NODE
 (ALL COMPONENTS ARE CALCULATED FOR THE LAST 1 TIME STEPS)

14	.0000	-8.3940	-.0062	8.4000	14
15	.0000	-10.4291	-.1714	10.6000	15
16	.0000	-5.1287	-.0716	5.2000	16
17	.0000	-6.5572	-.0430	6.6000	17
18	.0000	-6.1814	-.0184	6.2000	18
19	.0000	-7.5939	-.0064	7.6000	19
20	.0000	-4.2392	.0390	4.2000	20

Contd.

21	.0000	-5.4752	.0756	5.4000	21
22	.0000	-8.5345	-.0652	8.6000	22
23	.0000	-4.0898	.0900	4.0000	23
24	.0000	-4.9213	.1214	4.8000	24
27	.0000	-9.8141	.2245	9.6000	27
28	.0000	-7.3374	-.0621	7.4000	28
29	.0000	-10.3369	-.0628	10.4000	29
30	.0000	-8.0090	.0088	8.0000	30
31	.0000	-6.5946	-.0057	6.6000	31
32	.0000	-4.6553	.0550	4.6000	32
33	.0000	-7.8388	.0392	7.8000	33
34	.0000	-6.6962	-.1040	6.8000	34
35	.0000	-7.9004	-.0999	8.0000	35
36	.0000	-5.7712	-.0288	5.8000	36
37	.0000	-4.5034	-.0968	4.6000	37
38	.0000	-6.8411	.0409	6.8000	38
39	.0000	-8.7123	-.0874	8.8000	39
42	.0000	-9.9571	-.0425	10.0000	42
43	.0000	-15.3616	-.0348	15.4000	43
44	.0000	-6.0306	.0309	6.0000	44
45	.0000	-8.2502	.0505	8.2000	45
46	.0000	-8.4247	.0251	8.4000	46
47	.0000	-15.5470	-.0519	15.6000	47
48	.0000	-7.4903	-.1092	7.6000	48
51	.0000	-13.5754	-.0302	13.6000	51
52	.0000	-16.5258	-.0352	16.6000	52
53	.0000	-16.1631	-.0376	16.2000	53
54	.0000	-12.1389	-.0615	12.2000	54
55	.0000	-9.8148	.0152	9.8000	55
56	.0000	-10.4015	.0019	10.4000	56
57	.0000	-8.7998	-.0005	8.8000	57
58	.0000	-8.8136	.0133	8.8000	58
59	.0000	-11.3959	-.0037	11.4000	59
60	.0000	-8.7722	-.0274	8.8000	60
63	.0000	-18.0005	-.0222	18.0000	63
64	.0000	-13.7914	-.0082	13.8000	64
65	.0000	-13.9534	-.0471	14.0000	65
66	.0000	-10.5357	.1352	10.4000	66
67	.0000	-6.8165	.0167	6.8000	67
68	.0000	-7.9847	-.0157	8.0000	68
69	.0000	-11.3928	-.0067	11.4000	69
72	.0000	-12.5697	-.0299	12.6000	72
73	.0000	-15.5759	-.0194	15.6000	73
74	.0000	-20.4661	.0135	20.4000	74
75	.0000	-11.6303	.0306	11.6000	75
76	.0000	-7.8285	.0289	7.8000	76
77	.0000	-7.8124	.0128	7.8000	77
78	.0000	-14.4062	.0107	14.4000	78
81	.0000	-18.8809	-.0339	19.0000	81
82	.0000	-13.9988	-.0009	14.0000	82
83	.0000	-8.6063	.0062	8.6000	83
84	.0000	-7.2055	.0055	7.2000	84
85	.0000	-8.0128	.0132	8.0000	85
86	.0000	-8.4018	.0019	8.4000	86
87	.0000	-5.6050	.0050	5.6000	87
89	.0000	-19.0189	.0023	19.0000	89
90	.0000	-19.0060	-.0070	19.0000	90
91	.0000	-16.7888	-.0163	16.8000	91

Contd.

92	,0000	-10.5965	-,0032	10.6000	92
TOTAL	,0000	-662.9028	-,4576	663.3997	

T = 24.0000 NO. OF ITERATIONS = 22 NO. OF SUBITERATIONS = 184

NODE	H	NODE	H	NODE	H
1	200.0000	35	200.5159	69	188.0303
2	203.0000	36	198.1710	70	182.0000
3	204.0000	37	200.0190	71	189.0000
4	207.0000	38	196.0434	72	193.0218
5	206.5000	39	197.5135	73	192.2095
6	203.5000	40	192.0000	74	191.4662
7	202.5000	41	194.5000	75	191.6083
8	201.0000	42	200.4977	76	190.9951
9	200.5000	43	199.0213	77	191.2288
10	204.0000	44	197.9602	78	185.4755
11	199.0000	45	196.0106	79	182.0000
12	195.5000	46	195.5056	80	188.0000
13	195.5000	47	196.5096	81	190.9996
14	200.9922	48	194.0162	82	190.5410
15	204.1891	49	189.0000	83	189.6050
16	204.9544	50	194.5000	84	189.5911
17	202.9536	51	196.9888	85	188.0300
18	200.9686	52	198.6128	86	186.0006
19	200.5467	53	196.4876	87	184.9516
20	200.0464	54	197.0207	88	182.5000
21	199.0178	55	196.5120	89	188.9912
22	201.9563	56	194.9910	90	188.5056
23	199.9688	57	194.9678	91	185.4639
24	196.0460	58	194.4290	92	183.5341
25	193.0000	59	194.0107	93	188.3000
26	192.0000	60	192.0221	94	189.6000
27	196.9977	61	184.0000	95	188.3000
28	202.0258	62	192.0000	96	187.2000
29	202.0207	63	194.9535	97	185.2000
30	200.5317	64	194.0333	98	183.4000
31	199.0061	65	193.9835	99	182.1000
32	197.9720	66	192.9843	100	180.5000
33	196.9959	67	191.0280		
34	201.4044	68	190.9594		

WATER BALANCE COMPONENTS

NODE NET RECHARGE CHANGE IN STORAGE SUBSURFACE FLOW ARTIFICIAL FLOW NODE
 (ALL COMPONENTS ARE CALCULATED FOR THE LAST 1 TIME STEPS)

14	.0000	8.3810	.0196	-8.4000	14
15	.0000	10.6305	-.2300	-10.4000	15
16	.0000	5.2972	-.0972	-5.2000	16
17	.0000	6.8254	-.0257	-6.8000	17
18	.0000	5.5588	.0417	-5.6000	18
19	.0000	5.6001	.0266	-5.6000	19
20	.0000	2.0274	-.0271	-2.0000	20
21	.0350	2.7418	.0580	-2.8000	21
22	.0000	6.9153	-.1156	-6.8000	22

Contd.

23	.0000	.1339	-.1335	.0000	23
24	.0000	2.2519	.1593	-2.4000	24
27	.0000	-9.2560	.0565	9.2000	27
28	.0000	7.3132	-.1128	-7.2000	28
29	.0000	7.7201	-.1202	-7.6000	29
30	.0000	5.9989	.0006	-6.0000	30
31	.0000	5.5895	.0102	-5.6000	31
32	.0000	4.7137	.0866	-4.8000	32
33	.0000	7.9185	.0811	-8.0000	33
34	.0000	5.3172	-.1170	-5.2000	34
35	.0000	6.1475	-.1478	-6.0000	35
36	.0000	5.2203	-.0200	-5.2000	36
37	.0000	1.0951	-.2932	-.8000	37
38	.0000	3.5551	.1142	-3.6000	38
39	.0000	6.9522	-.1515	-6.8000	39
42	.0000	10.0714	-.0709	-10.0000	42
43	.0000	9.6587	-.0584	-9.6000	43
44	.0000	4.3196	.0230	-4.4000	44
45	.0000	8.2221	.1770	-8.4000	45
46	.0000	5.9379	.0617	-6.0000	46
47	.0000	10.4516	-.0502	-10.4000	47
48	.0000	7.3775	-.1770	-7.2000	48
51	.0000	15.2563	-.0405	-15.2000	51
52	.0000	13.6582	-.0516	-13.6000	52
53	.0000	16.8463	-.0161	-16.8000	53
54	.0000	5.3465	-.1460	-5.2000	54
55	.0000	4.3910	.0085	-4.4000	55
56	.0000	-.0172	.0168	.0000	56
57	.0000	2.3798	.0199	-2.4000	57
58	.0000	-3.9288	-.0715	4.0000	58
59	.0000	.0460	-.0463	.0000	59
60	.0000	6.8110	-.0106	-6.8000	60
63	.0000	17.9930	-.0098	-18.0000	63
64	.0000	13.6164	-.0150	-13.6000	64
65	.0000	14.0081	-.0059	-14.0000	65
66	.0000	-14.8732	.0733	14.8000	66
67	.0000	-.9172	.1175	.8000	67
68	.0000	1.1764	.0233	-1.2000	68
69	.0000	4.4163	-.0159	-4.4000	69
72	.0000	9.6627	-.0631	-9.6000	72
73	.0000	16.4091	-.0024	-16.4000	73
74	.0000	16.4987	.0618	-16.4000	74
75	.0000	-7.5843	-.0160	7.6000	75
76	.0000	-7.5918	-.0081	7.6000	76
77	.0000	-12.2470	-.1530	12.4000	77
78	.0000	4.3722	.0281	-4.4000	78
81	.0000	11.6905	-.0810	-11.6000	81
82	.0000	8.4103	-.0098	-8.4000	82
83	.0000	4.3816	.0186	-4.4000	83
84	.0000	-.4054	.0057	.4000	84
85	.0000	1.1354	.0646	-1.2000	85
86	.0000	5.9947	.0051	-6.0000	86
87	.0000	2.8039	-.0121	-2.8000	87
89	.0000	18.8819	.0009	-18.8000	89
90	.0000	8.8401	-.0323	-8.8000	90
91	.0000	22.0094	.0099	-22.0000	91
92	.0000	12.3902	.0102	-12.4000	92
TOTAL	.0000	382.5483	-1.3747	-380.7998	